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CHECK SHEET
FOR
RECURRING SPECIAL PROVISIONS

Adopted January 1, 2015

The following RECURRING SPECIAL PROVISIONS indicated by an "X" are applicable to this contract and are included by reference:

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BDE SPECIAL PROVISIONS
For the January 15 and March 4, 2016 Lettings

The following special provisions indicated by an "x" are applicable to this contract and will be included by the Project Development and Implementation Section of the BD&E. An * indicates a new or revised special provision for the letting.

<u>File Name</u>	<u>#</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
80240	1	Above Grade Inlet Protection	July 1, 2009	Jan. 1, 2012
80099	2	Accessible Pedestrian Signals (APS)	April 1, 2003	Jan. 1, 2014
* 80274	3	Aggregate Subgrade Improvement	April 1, 2012	Jan. 1, 2016
80192	4	Automated Flagger Assistance Device	Jan. 1, 2008	
80173	5	Bituminous Materials Cost Adjustments	Nov. 2, 2006	July 1, 2015
80241	6	Bridge Demolition Debris	July 1, 2009	
5026I	7	Building Removal-Case I (Non-Friable and Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5048I	8	Building Removal-Case II (Non-Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5049I	9	Building Removal-Case III (Friable Asbestos)	Sept. 1, 1990	April 1, 2010
5053I	10	Building Removal-Case IV (No Asbestos)	Sept. 1, 1990	April 1, 2010
80360	11	Coarse Aggregate Quality	July 1, 2015	
80310	12	Coated Galvanized Steel Conduit	Jan. 1, 2013	Jan. 1, 2015
80341	13	Coilable Nonmetallic Conduit	Aug. 1, 2014	Jan. 1, 2015
80198	14	Completion Date (via calendar days)	April 1, 2008	
80199	15	Completion Date (via calendar days) Plus Working Days	April 1, 2008	
80293	16	Concrete Box Culverts with Skews > 30 Degrees and Design Fills ≤ 5 Feet	April 1, 2012	April 1, 2015
80294	17	Concrete Box Culverts with Skews ≤ 30 Degrees Regardless of Design Fill and Skews > 30 Degrees with Design Fills > 5 Feet	April 1, 2012	April 1, 2014
80311	18	Concrete End Sections for Pipe Culverts	Jan. 1, 2013	
80334	19	Concrete Gutter, Curb, Median, and Paved Ditch	April 1, 2014	Aug. 1, 2014
80277	20	Concrete Mix Design – Department Provided	Jan. 1, 2012	Jan. 1, 2014
80261	21	Construction Air Quality – Diesel Retrofit	June 1, 2010	Nov. 1, 2014
80335	22	Contract Claims	April 1, 2014	
80029	23	Disadvantaged Business Enterprise Participation	Sept. 1, 2000	Jan. 2, 2015
80358	24	Equal Employment Opportunity	April 1, 2015	
80265	25	Friction Aggregate	Jan. 1, 2011	Nov. 1, 2014
80229	26	Fuel Cost Adjustment	April 1, 2009	July 1, 2015
80329	27	Glare Screen	Jan. 1, 2014	
80304	28	Grooving for Recessed Pavement Markings	Nov. 1, 2012	Aug. 1, 2014
80246	29	Hot-Mix Asphalt – Density Testing of Longitudinal Joints	Jan. 1, 2010	April 1, 2012
80322	30	Hot-Mix Asphalt – Mixture Design Composition and Volumetric Requirements	Nov. 1, 2013	Nov. 1, 2014
80323	31	Hot-Mix Asphalt – Mixture Design Verification and Production	Nov. 1, 2013	Nov. 1, 2014
80347	32	Hot-Mix Asphalt – Pay for Performance Using Percent Within Limits – Jobsite Sampling	Nov. 1, 2014	July 1, 2015
80348	33	Hot-Mix Asphalt – Prime Coat	Nov. 1, 2014	
80315	34	Insertion Lining of Culverts	Jan. 1, 2013	Nov. 1, 2013
80351	35	Light Tower	Jan. 1, 2015	
80336	36	Longitudinal Joint and Crack Patching	April 1, 2014	
80324	37	LRFD Pipe Culvert Burial Tables	Nov. 1, 2013	April 1, 2015
80325	38	LRFD Storm Sewer Burial Tables	Nov. 1, 2013	April 1, 2015
80045	39	Material Transfer Device	June 15, 1999	Aug. 1, 2014
80342	40	Mechanical Side Tie Bar Insertor	Aug. 1, 2014	Jan. 1, 2015
80165	41	Moisture Cured Urethane Paint System	Nov. 1, 2006	Jan. 1, 2010
80361	42	Overhead Sign Structures Certification of Metal Fabricator	Nov. 1, 2015	
80337	43	Paved Shoulder Removal	April 1, 2014	

<u>File Name</u>	<u>#</u>	<u>Special Provision Title</u>	<u>Effective</u>	<u>Revised</u>
80349	44	Pavement Marking Blackout Tape	Nov. 1, 2014	
80298	45	Pavement Marking Tape Type IV	April 1, 2012	
80254	46	Pavement Patching	Jan. 1, 2010	
80352	47	Pavement Striping - Symbols	Jan. 1, 2015	
80359	48	Portland Cement Concrete Bridge Deck Curing	April 1, 2015	
80353	49	Portland Cement Concrete Inlay or Overlay	Jan. 1, 2015	April 1, 2015
80338	50	Portland Cement Concrete Partial Depth Hot-Mix Asphalt Patching	April 1, 2014	
80343	51	Precast Concrete Handhole	Aug. 1, 2014	
80300	52	Preformed Plastic Pavement Marking Type D - Inlaid	April 1, 2012	
80328	53	Progress Payments	Nov. 2, 2013	
3426I	54	Railroad Protective Liability Insurance	Dec. 1, 1986	Jan. 1, 2006
80157	55	Railroad Protective Liability Insurance (5 and 10)	Jan. 1, 2006	
80306	56	Reclaimed Asphalt Pavement (RAP) and Reclaimed Asphalt Shingles (RAS)	Nov. 1, 2012	April 1, 2014
80350	57	Retroreflective Sheeting for Highway Signs	Nov. 1, 2014	
80327	58	Reinforcement Bars	Nov. 1, 2013	
80344	59	Rigid Metal Conduit	Aug. 1, 2014	
80354	60	Sidewalk, Corner, or Crosswalk Closure	Jan. 1, 2015	April 1, 2015
80340	61	Speed Display Trailer	April 2, 2014	
80127	62	Steel Cost Adjustment	April 2, 2004	July 1, 2015
* 80362	63	Steel Slag in Trench Backfill	Jan. 1, 2016	
80317	64	Surface Testing of Hot-Mix Asphalt Overlays	Jan. 1, 2013	
80355	65	Temporary Concrete Barrier	Jan. 1, 2015	July 1, 2015
80301	66	Tracking the Use of Pesticides	Aug. 1, 2012	
80356	67	Traffic Barrier Terminals Type 6 or 6B	Jan. 1, 2015	
20338	68	Training Special Provisions	Oct. 15, 1975	
80318	69	Traversable Pipe Grate	Jan. 1, 2013	April 1, 2014
80345	70	Underpass Luminaire	Aug. 1, 2014	April 1, 2015
80357	71	Urban Half Road Closure with Mountable Median	Jan. 1, 2015	July 1, 2015
80346	72	Waterway Obstruction Warning Luminaire	Aug. 1, 2014	April 1, 2015
80288	73	Warm Mix Asphalt	Jan. 1, 2012	Nov. 1, 2014
80302	74	Weekly DBE Trucking Reports	June 2, 2012	April 2, 2015
80289	75	Wet Reflective Thermoplastic Pavement Marking	Jan. 1, 2012	
80071	76	Working Days	Jan. 1, 2002	

The following special provisions are in the 2015 Supplemental Specifications and Recurring Special Provisions:

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location</u>	<u>Effective</u>	<u>Revised</u>
80292	Coarse Aggregate in Bridge Approach Slabs/Footings	Articles 1004.01(b) and 1004.02(f)	April 1, 2012	April 1, 2013
80303	Granular Materials	Articles 1003.04, 1003.04(c), and 1004.05(c)	Nov. 1, 2012	
80330	Pavement Marking for Bike Symbol	Article 780.14	Jan. 1, 2014	
80331	Payrolls and Payroll Records	Recurring CS #1 and #5	Jan. 1, 2014	
80332	Portland Cement Concrete – Curing of Abutments and Piers	Article 1020.13	Jan. 1, 2014	
80326	Portland Cement Concrete Equipment	Article 1103.03(a)(5)	Nov. 1, 2013	
80281	Quality Control/Quality Assurance of Concrete Mixtures	Recurring CS #31	Jan. 1, 2012	Jan. 1, 2014
80283	Removal and Disposal of Regulated Substances	Articles 669.01, 669.08, 669.09, 669.14, and 669.16	Jan. 1, 2012	Nov. 2, 2012
80319	Removal and Disposal of Surplus Materials	Article 202.03	Nov. 2, 2012	
80307	Seeding	Article 250.07	Nov. 1, 2012	
80339	Stabilized Subbase	Article 312.06	April 1, 2014	

<u>File Name</u>	<u>Special Provision Title</u>	<u>New Location</u>	<u>Effective</u>	<u>Revised</u>
80333	Traffic Control Setup and Removal Freeway/Expressway	Articles 701.18(l) and 701.19(a)	Jan. 1, 2014	

The following special provisions require additional information from the designer. The additional information needs to be included in a separate document attached to this check sheet. The Project Development and Implementation section will then include the information in the applicable special provision. The Special Provisions are:

- Bridge Demolition Debris
- Building Removal-Case I
- Building Removal-Case II
- Building Removal-Case III
- Building Removal-Case IV
- Completion Date
- Completion Date Plus Working Days
- DBE Participation
- Material Transfer Device
- Railroad Protective Liability Insurance
- Training Special Provisions
- Working Days

GUIDE BRIDGE SPECIAL PROVISION INDEX/CHECK SHEET

Effective as of the: July 31, 2015 Letting

√	File Name	Title	Effective	Revised
	GBSP4	Polymer Modified Portland Cement Mortar	June 7, 1994	July 26, 2013
	GBSP12	Drainage System	June 10, 1994	Jun 24, 2015
	GBSP13	High-Load Multi-Rotational Bearings	Oct 13, 1988	Oct 30, 2012
	GBSP14	Jack and Remove Existing Bearings	April 20, 1994	Jan 1, 2007
	GBSP15	Three Sided Precast Concrete Structure	July 12, 1994	Dec 29, 2014
	GBSP16	Jacking Existing Superstructure	Jan 11, 1993	Jan 1, 2007
	GBSP17	Bonded Preformed Joint Seal	July 12, 1994	Jan 1, 2007
	GBSP18	Modular Expansion Joint	May 19, 1994	Dec 29, 2014
	GBSP21	Cleaning and Painting Contact Surface Areas of Existing Steel Structures	June 30, 2003	May 18, 2011
	GBSP25	Cleaning and Painting Existing Steel Structures	Oct 2, 2001	April 19, 2012
	GBSP26	Containment and Disposal of Lead Paint Cleaning Residues	Oct 2, 2001	April 30, 2010
	GBSP28	Deck Slab Repair	May 15, 1995	Oct 15, 2011
	GBSP29	Bridge Deck Microsilica Concrete Overlay	May 15, 1995	Jun 24, 2015
	GBSP30	Bridge Deck Latex Concrete Overlay	May 15, 1995	Jun 24, 2015
	GBSP31	Bridge Deck High-Reactivity Metakaolin (HRM) Conc Overlay	Jan 21, 2000	Jun 24, 2015
	GBSP32	Temporary Sheet Piling	Sept 2, 1994	Jan 31, 2012
	GBSP33	Pedestrian Truss Superstructure	Jan 13, 1998	Dec 29, 2014
	GBSP34	Concrete Wearing Surface	June 23, 1994	Feb 6, 2013
	GBSP35	Silicone Bridge Joint Sealer	Aug 1, 1995	Oct 15, 2011
	GBSP38	Mechanically Stabilized Earth Retaining Walls	Feb 3, 1999	Dec 29, 2014
	GBSP42	Drilled Soldier Pile Retaining Wall	Sept 20, 2001	Jan 3, 2014
	GBSP43	Driven Soldier Pile Retaining Wall	Nov 13, 2002	Jan 3, 2014
	GBSP44	Temporary Soil Retention system	Dec 30, 2002	May 11, 2009
	GBSP45	Bridge Deck Thin Polymer Overlay	May 7, 1997	Feb 6, 2013
	GBSP46	Geotextile Retaining walls	Sept 19, 2003	July 26, 2013
	GBSP51	Pipe Underdrain for Structures	May 17, 2000	Jan 22, 2010
	GBSP53	Structural Repair of Concrete	Mar 15, 2006	Aug 29, 2014
	GBSP55	Erection of Curved Steel Structures	June 1, 2007	
	GBSP56	Setting Piles in Rock	Nov 14, 1996	April 19, 2012
	GBSP57	Temporary Mechanically Stabilized Earth Retaining Walls	Jan 6, 2003	Dec 29, 2014
	GBSP59	Diamond Grinding and Surface Testing Bridge Sections	Dec 6, 2004	Jan 3, 2014
	GBSP60	Containment and Disposal of Non-Lead Paint Cleaning Residues	Nov 25, 2004	Mar 6, 2009
	GBSP61	Slipform Parapet	June 1, 2007	Dec 29, 2014
	GBSP62	Concrete Deck Beams	June 13, 2008	Oct 9, 2009
	GBSP64	Segmental Concrete Block Wall	Jan 7, 1999	Oct 30, 2012
	GBSP65	Precast Modular Retaining Wall	Mar 19, 2001	Dec 29, 2014
	GBSP67	Structural Assessment Reports for Contractor's Means and Methods	Mar 6, 2009	
	GBSP70	Braced Excavation	Aug 9, 1995	May 18, 2011
	GBSP71	Aggregate Column Ground Improvement	Jan 15, 2009	Oct 15, 2011

	GBSP 72	Bridge Deck Fly Ash or GGBF Slag Concrete Overlay	Jan 18, 2011	Jun 24, 2015
	GBSP 73	Cofferdams	Oct 15, 2011	
	GBSP 74	Permanent Steel Sheet Piling (LRFD)	Jan 31, 2012	Aug 17, 2012
	GBSP 75	Bond Breaker for Prestressed Concrete Bulb-T Beams	April 19, 2012	
	GBSP 76	Granular Backfill for Structures	April 19, 2012	Oct 30, 2012
	GBSP 77	Weep Hole Drains for Abutments, Wingwalls, Retaining Walls and Culverts	April 19, 2012	Oct 22, 2013
	GBSP 78	Bridge Deck Construction	Oct 22, 2013	April 18, 2014
	GBSP 79	Bridge Deck Grooving (Longitudinal)	Dec 29, 2014	
	GBSP 80	Fabric Reinforced Elastomeric	Aug 29, 2014	

LIST ADDITIONAL SPECIAL PROVISIONS BELOW

The following Guide Bridge Special Provisions have been incorporated into the 2012 Standard Specifications:

File Name	Title	Std Spec Location
GBSP22	Cleaning and Painting New Metal Structures	506
GBSP36	Surface Preparation and Painting Req. for Weathering Steel	506
GBSP50	Removal of Existing Non-composite Bridge Decks	501
GBSP58	Mechanical Splicers	508
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GBSP68	Piling	512
GBSP69	Freeze-Thaw Aggregates for Concrete Superstructures Poured on Grade	1004

The following Guide Bridge Special Provisions have been discontinued or have been superseded:

File Name	Title	Disposition:
GBSP37	Underwater Structure Excavation Protection	Replaced by GBSP73
GBSP11	Permanent Steel Sheet Piling	Replaced by GBSP74
GBSP47	High Performance Concrete Structures	Discontinued
GBSP 52	Porous Granular Embankment (Special)	Replaced by GBSP76
GBSP66	Wave equation Analysis of Piles	Discontinued

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LR107-4 SPECIAL PROVISION FOR INSURANCE

BDE SPECIAL PROVISIONS

GUIDE BRIDGE SPECIAL PROVISIONS

SWPPP

NOTICE OF INTENT

US ARMY CORPS OF ENGINEERS INDIVIDUAL PERMIT AUTHORIZATION

IEPA 401 CLEAN WATER CERTIFICATION STATEMENT

ROADWAY GEOTECHNICAL REPORT – SOIL BORING LOGS

PRELIMINARY SITE INVESTIGATION ANALYTICAL RESULTS

CCDD FORM 663

BLANDING’S TURTLE CONSERVATION PLAN

STATE OF ILLINOIS

SPECIAL PROVISIONS

The following Special Provisions supplement the “Standard Specifications for Road and Bridge Construction”, adopted January 1, 2012, the latest edition of the “Manual on Uniform Traffic Control Devices for Streets and Highways”, and the “Manual of Test Procedures of Materials” in effect on the date of invitation of bids, and the “Supplemental Specifications and Recurring Special Provisions” indicated on the Check Sheet included herein which apply to and govern the construction of the Longmeadow Parkway, Randall Road, and Sleepy Hollow Road roadway improvements, in the Village of Algonquin and unincorporated Dundee Township, Illinois in Kane County, and in case of conflict with any part or parts of said specifications, the said Special Provisions shall take precedence and shall govern.

LOCATION OF PROJECT

The project is located in Kane County, Illinois, in the Village of Algonquin and unincorporated Dundee Township. The project extends through the South Half of Section 5, North Half of Section 8, South Half of Section 4, North Half of Section 9, West Half of Section 3, and Northwest Quarter of Section 10, all in Township 42 North, Range 8 East of the third principal meridian. The proposed corridor improvement begins at the intersection of Randall Road and Longmeadow Parkway and ends approximately 0.20 miles east of White Chapel Lane and is under the jurisdiction of the Kane County Division of Transportation. Relative to major arterials, the project is located approximately 2.5 miles north of Illinois Route 72 and 2.0 miles southwest of Illinois Route 62. The total gross and net length of the project is 13,172.78 feet (2.50 mi.).

DESCRIPTION OF PROJECT

The work consists of existing roadway reconstruction and new lane construction on a new alignment. Work consists of pavement and subgrade construction; storm sewer installation; two traffic signals and associated system interconnect; various retaining wall and multi-cell box culvert installations; roadway resurfacing along Randall Road; beacon lighting improvements; soil erosion and sediment control measures; landscaping and tree removal & replacement; and pavement marking and signing.

COORDINATION WITH ADJACENT AND/OR OVERLAPPING CONTRACTS

The Longmeadow Parkway Corridor Project encompasses various construction contracts which may be performed concurrently. Contracts may abut and/or overlap others; therefore, each contract includes work items that require close coordination between contractors regarding the sequence and timing for execution of work items.

General Coordination. The contractor is directed and shall comply with Section 105.08 of the Standard Specifications and as herein described.

MAINTENANCE OF ROADWAYS (KDOT)

Effective: September 30, 1985

Revised: November 24, 2014

Beginning on the date that work begins on this project, the Contractor shall assume responsibility for normal maintenance of all existing roadways within the limits of the improvement. This normal maintenance shall

include all repair work deemed necessary by the Engineer, but shall not include snow removal operations. Traffic control and protection for maintenance of roadways will be provided by the Contractor as required by the Engineer.

The Contractor shall commence work on all maintenance of roadways items within 2 hours of notification by the Engineer and complete such items in an expedient and timely manner. Failure to do so may result in a deficiency of \$2000 per calendar day.

If items of work have not been provided in the contract or otherwise specified for payment such items, including the accompanying traffic control and protection required by the Engineer, will be paid for in accordance with Article 109.04 of the Standard Specifications.

STATUS OF UTILITIES TO BE ADJUSTED

Effective: January 30, 1987

Revised: January 24, 2013

Utilities companies involved in this project have provided the following estimated durations:

NAME OF UTILITY	TYPE	LOCATION	Estimated Duration of Time for the Completion of Relocation or Adjustments
Village of Algonquin Robert Mitchard, II 110 Meyer Dr. Algonquin, IL 60102 (847) 658-2754	Watermain, 8"	3 Locations	As part of contract.
AT&T Hector Garcia AT&T Civic Eng. Project 1000 Commerce Dr. Oak Brook, IL 60523 (847) 888-6799 (O) (630) 639-8372			
Comcast Pat Goheen 688 Industrial Dr. Elmhurst, IL 60126 (847) 789-0976			

ComEd Nora Fernandez West Region Public Relations 123 Energy Av. Rockford, IL 61109 nora.fernandez@comed.com			
Nicor Constance Lane 1844 Ferry Rd. Naperville, IL 60563 (630) 388-3830			

In accordance with 605 ILCS 5/9-113 of the Illinois Compiled Statutes, utility companies have 90 days to complete the relocation of their facilities after receipt of written notice from the Department. The 90-day written notice will be sent to the utility companies after the following occurs:

- 1) Proposed right of way is clear for contract award.
- 2) Final plans have been sent to and received by the utility company.
- 3) Utility permit is received by the Department and the Department is ready to issue said permit.
- 4) If a permit has not been submitted, a 15 day letter is sent to the utility company notifying them they have 15 days to provide their permit application. After allowing 15 days for submission of the permit the 90 day notice is sent to the utility company.
- 5) Any time within the 90 day relocation period the utility company may request a waiver for additional time to complete their relocation. The Department has 10 days to review and respond to a waiver request.

The above represents the best information available to the Department and is included for the convenience of the bidder. The applicable portions of Articles 105.07 and 107.31 of the Standard Specifications shall apply.

RESTRICTION ON WORKING DAYS AFTER A COMPLETION DATE

Effective: January 21, 2003

Revised: January 1, 2007

All temporary lane closures during the period governed by working days after a completion date will not be permitted during the hours of 6:00 a.m. to 9:00 a.m. and 3:00 p.m. to 6:00 p.m. Monday through Friday.

All lane closure signs shall not be erected any earlier than one-half (1/2) hour before the starting hours listed above. Also, these signs should be taken down within one-half (1/2) hour after the closure is removed.

Failure to Open Traffic Lanes to Traffic: Should the Contractor fail to completely open and keep open all the traffic lanes to traffic in accordance with the limitations specified above, the Contractor shall be liable and

shall pay to the Department the amount of \$250 per lane blocked, not as a penalty but as liquidated and ascertained damages, for each and every 15 minute interval or a portion thereof that a lane is blocked outside the allowable time limitations. The Department may deduct such damages from any monies due the Contractor. These damages shall apply during the period governed by working days after a completion date and any extensions of that contract time.

COMPLETION DATE PLUS GUARANTEED WORKING DAYS (KDOT)

Effective: 09/30/1985

Revised: 11/10/2014

Revise Article 108.05 (c) of the Standard Specifications as follows:

"When a completion date plus guaranteed working days is specified, the Contractor shall complete all contract items and safely open all roadways (including shoulders) to traffic by 11:59 PM on October 7, 2017.

The Contractor will be allowed to complete all clean-up work, punch list items, and landscaping within 10 guaranteed working days after the completion date for opening the roadway to traffic. Under extenuating circumstances the Engineer may direct that certain items of work, not affecting the safe opening of the roadway to traffic, may be completed within the guaranteed working days allowed for cleanup work and punch list items."

Article 108.09 of the Recurring Specifications or the Special Provision for Failure to Complete the Work on Time shall apply to the completion date and the number of working days.

MODIFIED IDOT STANDARD PAY ITEM
SPECIAL PROVISIONS

20200100 EARTH EXCAVATION

Description: This work shall consist of the excavation and transportation of suitable excavated material to embankment locations throughout the limits of the project. This work shall also consist of the excavation, transportation and disposal of excess and unsuitable materials.

General: This work shall conform to the requirements of Section 202 of the “Standard Specifications” and the following:

For this Project the Earth Excavation shall consist of:

1. ☒ *Excavation to the subgrade elevation.*
2. ☒ *Excavation for topsoil placement.*
3. ☐ *The removal of bituminous material not included in any other pay item.*
4. ☒ *Undercutting, as determined by the Engineer to include:*
 - a. ☒ *Removal of existing topsoil under proposed embankment.*
 - b. ☒ *Removal of unsuitable material in wet areas.*
5. ☒ *Undercutting, based on the recommendations of the soil survey and report.*
 - a. *An estimated quantity of excavation for undercutting has been included in the quantity of Earth Excavation and is shown on the plans.*
 - b. *Undercutting may be employed only at the discretion of the Engineer after it has been determined that the provisions of Section 301 of the “Standard Specifications” will not yield sufficient results to allow the timely progress of the project.*

Removal and disposal of unstable, unsuitable and/or excess material will be paid for separately as Removal and Disposal of Unsuitable Material or Topsoil Excavation and Placement, Special. All unstable, unsuitable and/or excess material shall be disposed of outside the right-of-way according to Article 202.03 of the “Standard Specifications”.

Earth moved more than once due to construction staging and/or procedures selected by the Contractor, will not be paid for separately, but shall be considered included in the unit cost of Earth Excavation.

A Soil Survey and Report:

☒ Was performed.

☐ Was not performed.

Project Roadway Geotechnical Report (RGR) is included with the project specifications.

Method of Measurement: Earth Excavation will be measured in its original position and the volume in cubic yards computed by the method of average end areas.

Basis of Payment: This work will be paid for at the contract unit price per cubic yard for EARTH EXCAVATION. *The unit price shall include all equipment and labor required to excavate, transport and distribute earth.*

28000305 TEMPORARY DITCH CHECKS

Description: This work shall consist of constructing, maintaining, and removing temporary ditch checks.

General: The work shall be performed according to Section 280 of the “Standard Specifications”, the details shown in the plans, and the following:

The temporary ditch check shall be triangular shaped, urethane foam covered with a geotextile fabric. The temporary ditch check shall be installed on a geotextile fabric apron. The temporary ditch check shall have a triangle base 16” – 20” wide and a minimum triangle height of 8” – 10”. The temporary ditch checks shall be installed at the locations specified on the Erosion Control Plan, and/or as directed by the Engineer. The temporary ditch check installation shall be according to the detail shown on the plans and the manufacturer’s recommendations.

The geotextile fabric shall conform to Article 1080.05 of the “Standard Specifications”, for Geotechnical Fabric for French Drains.

The temporary ditch checks shall remain in place until just before placing the final landscaping in the ditch area. The Contractor shall not remove the temporary ditch checks if it is raining and/or rain is in the immediate forecast.

The ditch checks shall become the property of the Contractor upon their removal.

Method of Measurement: Temporary Ditch Checks will be measured in place and the length calculated in feet for each ditch check section actually installed.

Basis of Payment: This work will be paid for at the contract unit price per foot for TEMPORARY DITCH CHECKS. The unit price shall include all labor, equipment and materials necessary for their installation and removal. The maintenance of this item shall be included with and paid for as part of the contract total price for MAINTENANCE OF EROSION CONTROL SYSTEM.

42400800 DETECTABLE WARNINGS

Description: This work shall consist of furnishing and installing detectable warnings in accessibility ramps.

Materials: The detectable warnings shall be cast iron panels of the sizes shown on the plans and shall meet the following material specification:

The detectable warning plate shall be constructed of gray iron meeting the requirements of Article 1006.14 of the “Standard Specifications” and ASTM A48, CLASS 35B; or cast ductile iron meeting the requirements of Article 1006.15 of the “Standard Specifications”.

The coating system shall consist of a rust inhibiting epoxy primer and a finish coat.

The epoxy primer shall have the following properties:

Property	Test Method	Performance
Humidity	ASTM D1735	1000 Hours Minimum
Water Immersion	ASTM D870	250 Hours Minimum
Corrosion Resistance (Salt Spray)	ASTM B117	1000 Hours Minimum

Cold Rolled Steel Lab Panels

The finish coat shall be a powder coat and shall have the following properties:

Property	Test Method	Performance
Color	---	Federal Yellow
Corrosion Resistance (Salt Spray)	ASTM B117	1000 Hours Minimum

Cold Rolled Steel Lab Panels

General: The installation of detectable warnings shall meet the requirements of Article 424.09 of the “Standard Specifications”. Grey iron plates shall be installed in concrete accessibility ramps only. Ductile iron plates may be installed in either concrete or hot-mix asphalt (HMA) accessibility ramps.

Method of Measurement: This work will be measured for payment in place installed, in square feet. The concrete area under the detectable warnings will be measured for payment as PORTLAND CEMENT CONCRETE SIDEWALK of the thickness specified, with no deductions made for the detectable warnings panels located within the ramp.

Basis of Payment: This work will be paid for at the contract unit price per square foot of DETECTABLE WARNINGS. The unit price shall include all equipment, materials and labor required to install the panels.

SECTION 602 RESTRICTED DEPTH DRAINAGE STRUCTURES

Effective: June 9, 2011

Revised: August 1, 2011

Description: This work shall consist of constructing restricted depth manholes, catch basins and inlets with a specified frame and grate/lid at locations identified on the plans.

Materials: The materials shall meet the requirements of Article 602.02 of the “Standard Specifications”.

General: The work shall be performed according to Section 602 of the “Standard Specifications”; the applicable IDOT Highway Standard(s) for the drainage structure type (manhole, catch basin or inlet); the IDOT Highway Standard Drawing 602601 [flat slab top] and the following:

- *The reinforced concrete slab shall be used in lieu of the cone section.*
- *A 24” sump shall be provided in a Catch Basin.*

- *For structures having Type 8 grates, a 24" inside diameter by 4" (minimum) high riser shall be installed on the flat slab to provide earth cover over the slab for vegetation.*

Method of Measurement: This work will be measured per each of the type drainage structure installed. Drainage structures of like type, size and frame and grate/lid will be counted under the same pay item regardless of whether a cone section (regular) or flat slab (restricted depth) top is used.

Basis of Payment: This work will be paid for at the contract unit price per each for MANHOLES, CATCH BASINS or INLETS, of the type and diameter specified, and with the frame and grate or frame and lid specified. The unit price shall include all equipment, labor and materials to install the drainage structure. No additional compensation will be made for drainage structures constructed as restricted depth.

7800XXXX MODIFIED URETHANE PAVEMENT MARKING (TYPE SPECIFIED)

This work shall consist of modifying the standard specifications for modified urethane pavement marking as follows:

Description: This work shall consist of furnishing and applying a reflectorized modified urethane, plural component, durable liquid pavement marking lines, sizes and colors as shown on the plans.

Materials: All materials shall meet the following specifications:

- (a) **Modified Urethane Marking:** The modified urethane pavement marking material shall consist of a homogeneous blend of modified urethane resins and pigments designed to provide a simple volumetric mixing ratio of two components (must be two volumes of Part A to one volume of Part B). No volatile solvent or fillers will be allowed.
- (b) **Pigmentation:** The pigment content by weight of Component A shall be determined by low temperature ashing according to ASTM D 3723. The pigment content shall not vary more than \pm two percent from the pigment content of the original qualified paint.

White Pigment shall be Titanium Dioxide meeting ASTM D 476 Type II, Rutile.

Yellow Pigment shall be Organic Yellow and contain no heavy metals.

- (c) **Environmental:** Upon heating to application temperature, the material shall not exude fumes, which are toxic or injurious to persons or property when handled according to manufacturer specifications. The modified urethane pavement marking material compositions shall not contain free isocyanate functionality.
- (d) **Daylight Reflectance:** The daylight directional reflectance of the cured modified urethane material (without reflective media) shall be a minimum of 80 percent (white) and 50 percent (yellow) relative to magnesium oxide when tested using a color spectrophotometer with a 45 degree circumferential / zero

degrees geometry, illuminant C, and two degrees observer angle. The color instrument shall measure the visible spectrum from 380 to 720 nm with a wavelength measurement interval and spectral bandpass of 10 nm. In addition, the color of the yellow modified urethane shall visually match Color Number 33538 of Federal Standard 595a with chromaticity limits as follows:

x	0.490	0.475	0.485	0.539
y	0.470	0.438	0.425	0.456

- (e) Weathering Resistance: The modified urethane, when mixed in the proper ratio and applied at 0.35 to 0.41 mm (14 to 16 mils) wet film thickness to an aluminum alloy panel (Federal Test Std. No. 141, Method 2013) and allowed to cure for 72 hours at room temperature, shall be subjected to accelerated weathering for 75 hours. The accelerated weathering shall be completed by using the light and water exposure apparatus (fluorescent UV – condensation type) and tested according to ASTM G 53.

The cycle shall consist of four hours UV exposure at 50 °C (122 °F) and four hours of condensation at 40 °C (104 °F). UVB 313 bulbs shall be used. At the end of the exposure period, the material shall show no substantial change in color or gloss.

- (f) Drying Time: The modified urethane material, when mixed in the proper ratio and applied at 0.35 to 0.41 mm (14 to 16 mils) wet film thickness and with the proper saturation of glass spheres, shall exhibit a no-tracking time of three minutes or less when tested according to ASTM D 711.
- (g) Adhesion: The catalyzed modified urethane pavement marking materials when applied to a 100 x 100 x 50 mm (4 x 4 x 2 in) concrete block shall have a degree of adhesion which results in a 100 percent concrete failure in the performance of this test.

The concrete block shall be brushed on one side and have a minimum strength of 24,100 kPa (3,500 psi). A 50 mm (2 in) square film of the mixed modified urethane shall be applied to the brushed surface and allowed to cure for 72 hours at room temperature. A 50 mm (2 in) square cube shall be affixed to the surface of the modified urethane by means of an epoxy glue. After the glue has cured for 24 hours, the modified urethane specimen shall be placed on a dynamic testing machine in such a fashion so that the specimen block is in a fixed position and the 50 mm (2 in) cube (glued to the modified urethane surface) is attached to the dynamometer head. Direct upward pressure shall be slowly applied until the modified urethane system fails. The location of the break and the amount of concrete failure shall be recorded.

- (h) Hardness: The modified urethane marking materials, when tested according to ASTM D-2240, shall have a Shore D Hardness greater than 75. Films shall be cast on a rigid substrate at 0.35 to 0.41 mm (14 to 16 mils) in thickness and allowed to cure at room temperature for 72 hours before testing.
- (i) Abrasion: The abrasion resistance shall be evaluated on a Taber Abrader with a 1,000 gram load and CS-17 wheels. The duration of test shall be 1,000 cycles. The wear index shall be calculated based on ASTM test method D-4060 and the wear index for the catalyzed material shall not be more than 80. The tests shall be run on cured samples of modified urethane material which have been applied at a film thickness of 0.35 to 0.41 (14 to 16 mils) to code S-16 stainless steel plates. The films shall be allowed to cure at room temperature for at least 72 hours and not more than 96 hours before testing.
- (j) Tensile: When tested according to ASTM D-638, the modified urethane pavement marking materials shall have an average tensile strength of not less than 6,000 pounds per square inch. The Type IV Specimens shall be pulled at a rate of 1/4" per minute by a suitable dynamic testing machine. The samples shall be allowed to cure at 75 °F± 2°F for a minimum of 24 hours and a maximum of 72 hours prior to performing the indicated tests.
- (k) Compressive Strength: When tested according to ASTM D-695, the catalyzed modified urethane pavement marking materials shall have a compressive strength of not less than 12,000 pounds per square inch. The cast sample shall be conditioned at 75°F± 2°F for a minimum of 72 hours before performing the indicated tests. The rate of compression of these samples shall be no more than 1/4" per minute.
- (l) Glass Spheres: The glass spheres shall meet the requirements of Article 1095.04(m) and Article 1095.07 of the Standard Specifications for first drop and second drop glass beads.
- (m) The material shall be shipped to the job site in substantial containers and shall be plainly marked with the manufacturer's name and address, the name and color of the material, date of manufacture and batch number.
- (n) Prior to approval and use of the modified urethane pavement marking materials, the manufacturer shall submit a notarized certification of an independent laboratory, together with the results of all tests, stating these materials meet the requirements as set forth herein. The certification test report shall state the lot tested, manufacturer's name, brand name of modified urethane and date of manufacture. The certification shall be accompanied by one half-liter (one-pint) samples each of Part A and Part B. Samples shall be sent in the appropriate volumes for complete mixing of Part A and Part B.

After approval by the Department, certification by the modified urethane manufacturer shall be submitted for each batch used. New independent laboratory certified test results and samples for testing by the Department shall be submitted any time the manufacturing process or paint formulation is changed. All costs of testing (other than tests conducted by the Department) shall be borne by the manufacturer.

- (o) Acceptance samples shall consist of one half-liter (one-pint) samples of Part A and Part B, of each lot of paint. Samples shall be sent in the appropriate volumes for complete mixing of Part A and Part B. The samples shall be submitted to the Department for testing, together with a manufacturer's certification. The certification shall state the formulation for the lot represented is essentially identical to that used for qualification testing. All, acceptance samples shall be taken by a representative of the Illinois Department of Transportation. The modified urethane pavement marking materials shall not be used until tests are completed and they have met the requirements as set forth herein.
- (p) The manufacturer shall retain the test sample for a minimum of 18 months.

Application Equipment: The modified urethane pavement marking compounds shall be applied through equipment specifically designed to precisely meter the two components in the ratio of 2:1 and approved by the manufacturer of the material. This equipment shall produce the required amount of heat at the mixing head and gun tip and maintain those temperatures within the tolerances specified. This equipment shall also have as an integral part of the gun carriage, a high pressure air spray capable of cleaning the pavement immediately prior to the marking application.

The equipment shall be capable of spraying both yellow and white urethane, according to the manufacturer's recommended proportions and be mounted on a truck of sufficient size and stability with an adequate power source to produce lines of uniform dimensions and prevent application failure. The truck shall have at least two urethane tanks each of 415 L (110 gal) minimum capacity and shall be equipped with hydraulic systems. It shall be capable of placing stripes on the left and right sides and placing two lines on a three-line system simultaneously with either line in a solid or intermittent pattern, in yellow or white, and applying glass beads by the double drop pressurized bead system. The system shall apply both the first drop glass beads and the second drop glass beads at a rate of 1.2 kg per L (10 lb./gal). The equipment shall be equipped with pressure gauges for each proportioning pump. All guns shall be in full view of operators at all times. The equipment shall have a metering device to register the accumulated installed quantities for each gun, each day. Each vehicle shall include at least one operator who shall be a technical expert in equipment operations and urethane application techniques. Certification of equipment shall be provided at the preconstruction conference.

Application: The pavement shall be cleaned by a method approved by the Engineer to remove all dirt, grease, glaze or any other material that would reduce the adhesion of the markings with minimum or no damage to the pavement. New PCC pavements shall be blast-cleaned to remove all curing compounds.

Markings shall be applied to the cleaned surfaces on the same calendar day. If this cannot be accomplished, the surface shall be re-cleaned prior to applying the markings. Existing pavement markings shall be at least 90 percent removed. No markings shall be applied until the Engineer approves the cleaning.

Widths, lengths and shapes of the cleaned surface shall be prepared wider than the modified urethane pavement marking material to be applied, such that a prepared area is on all sides of the urethane pavement marking material after application.

New asphalt concrete and seal coated surfaces shall be in place a minimum of two weeks prior to marking applications.

The cleaning operation shall be a continuous moving operation process with minimum interruption to traffic.

The pavement markings shall be applied to the cleaned road surface, during conditions of dry weather and subsequently dry pavement surfaces at a minimum uniform wet thickness of 20 mils in accordance with the manufacturer's installation instructions and at the widths and patterns shown on the contract plans. The application and combination of reflective media (glass beads and/or reflective elements) shall be applied at a rate specified by the manufacturer. At the time of installation the pavement surface temperature shall be 40 ° F and rising and the ambient temperature shall be 35° F and rising. The pavement surface temperature and the ambient temperatures shall be determined and documented before the start of each of marking operation. The pavement markings shall not be applied if the pavement shows any visible signs of moisture or it is anticipated that damage causing moisture, such as rain showers, may occur during the installation and curing periods. The Engineer shall determine the atmospheric conditions and pavement surface conditions that produce satisfactory results.

Unless directed by the Engineer, lines shall not be laid directly over a longitudinal crack or joint. The edge of the center line or lane line shall be offset a minimum distance of 50 mm (2 inches) from a longitudinal crack or joint. Edge lines shall be approximately 50 mm (2 inches) from the edge of pavement. The finished center and lane lines shall be straight, with the lateral deviation of any 3 meter (10-foot) line not to exceed 25 mm (1 inch).

Notification: The Contractor shall notify the Engineer 72 hours prior to the placement of the markings in order that an inspector can be present during the operation. At the time of this notification, the Contractor shall indicate the manufacturer and lot numbers of urethane and reflective media that he intends to use. The Engineer will ensure that the approved lot numbers appear on the material package. Failure to comply with this provision may be cause for rejection.

The Contractor shall provide an accurate temperature-measuring device(s) that shall be capable of measuring the pavement temperature prior to application of the material, the material temperature at the gun tip and the material temperature prior to mixing.

The Contractor shall be required to maintain a minimum initial retroreflectivity for all epoxy pavement marking that he/she applies, as follows:

Material	Color	Retro reflectivity (millicandelas/m ² /lux)
Urethane	White	300
Urethane	Yellow	250

The Engineer will measure the retro reflectivity a minimum of **twelve (12) hours** after and within **fourteen (14) days** of the application. The Engineer will take a minimum of ten (10) readings per color line, evenly spaced, on a 1,000 meter (0.6 mile) roadway section on all roadways specified in the schedule of quantities for epoxy pavement marking or as determined by the Engineer. The Engineer will average all of the readings for each color line within the 1,000 meter section of roadway to determine the retro reflectivity. The Contractor shall be required to replace all epoxy pavement not meeting the minimum retro reflectivity requirements at no additional expense to this contract.

Inspection: The urethane pavement markings will be inspected following installation, but no later than December 15, and inspected following a winter performance period that extends 180 days from December 15 in accordance with the provisions of Article 780.10 of the Standard Specification for Road and Bridge Construction.

Method of Measurement: The lines will be measured for payment in feet of urethane pavement marking lines applied and accepted, measured in place. Double yellow lines will be measured as two separate lines. Words and symbols shall conform to the size and dimensions specified in the Manual on Uniform Traffic Control Devices and Standard 780001 and will be measured based on total areas indicated in table 1 or as specified in the plans.

Basis of Payment: This work will be paid for at the contract unit prices per FOOT of applied line for MODIFIED URETHANE PAVEMENT MARKING - LINE 4, 5, 6, 8, 12, 24 inches or per SQUARE FOOT for MODIFIED URETHANE PAVEMENT MARKING – LETTERS AND SYMBOLS measured as specified herein.

PROJECT SPECIAL PROVISIONS

25200200 SUPPLEMENTAL WATERING

Description: This work will include watering turf, trees, shrubs, vines and perennial plants at the rates specified and as directed by the Engineer.

Schedule: Watering will only begin after the successful completion of all period of establishment requirements and will continue through the construction year growing season as directed by the Engineer.

Watering must be completed in a timely manner. When the Engineer directs the Contractor to do supplemental watering, the Contractor must begin the watering operation within 24 hours of notice. A minimum of 10 units of water per day must be applied until the work is complete.

Damage to plant material that is a result of the Contractor's failure to water in a timely way must be repaired or replaced at the Contractor's expense.

Source of Water: The Contractor shall notify the Engineer of the source of water used and provide written certification that the water does not contain chemicals harmful to plant growth.

Rate of Application: The normal rates of application for watering are as follows. The Engineer will adjust these rates as needed depending upon weather conditions.

Trees:	35 gallons per tree
Class 2A Seed:	10 gallons per square yard
All Other Seeded Areas:	3 gallons per square yard

Method of Application: A spray nozzle that does not damage small plants must be used when watering perennial plants or turf. Water shall be applied at the base of the plant to keep as much water as possible off plant leaves. An open hose may be used to water trees, shrubs, and vines if mulch and soil are not displaced by watering. Water shall trickle slowly into soil and completely soak the root zone. The Contractor must supply metering equipment as needed to assure the specified application rate of water.

Method of Measurement: Supplemental watering will be measured in units of 1000 gallons (3,785 liters) of water applied as directed.

Basis of Payment: This work will be paid for at the contract unit price per unit of SUPPLEMENTAL WATERING, measured as specified. Payment will include the cost of all water, equipment and labor needed to complete the work specified herein and to the satisfaction of the Engineer.

253XXXXX PLANTING WOODY PLANTS

Add the following to Construction Requirements:

Delete the third sentence of Article 253.07 and substitute the following:

The Contractor shall be responsible for all plant layout. The layout must be performed by qualified personnel. The planting locations must be laid out as shown in the landscape plan. This will require the use of an engineer's scale to determine some dimensions. Tree locations within each planting area shall be marked with a different color stake/flag and labeled to denote the different tree species. Shrub beds limits must be painted. The Engineer will contact the

Roadside Development Unit at (847) 705-4171 to approve the layout prior to installation. Allow a minimum of seven working (7) days prior to installation for approval.

Delete the first paragraph of Article 253.15 Plant Care and substitute the following:

The Contractor is responsible for plant care until receipt of the "Final Acceptance of Landscape Work" memorandum from the Bureau of Maintenance. The Contractor shall properly care for all plants including weeding, watering, adjusting of braces, repair of water saucers, or other work which is necessary to maintain the health, vigor, and satisfactory appearance of the plantings. This may require pruning, cultivating, tightening and repairing supports, repair of wrapping, and furnishing and applying sprays as necessary to keep the plants free of insects and disease. The Contractor shall provide plant care a minimum of every two weeks, or within 3 days following notification by the Engineer. All requirements for plant care shall be considered as included in the cost of the contract.

Delete the first paragraph of Article 253.15 Plant Care (a) and substitute the following:

During plant care watering shall be performed at least every two weeks beginning in May until receipt of the "Final Acceptance of Landscape Work" memorandum from the Bureau of Maintenance. The contractor shall apply a minimum of 35 gallons of water per tree, 25 gallons per large shrub, and 15 gallons per small shrub. The Engineer may direct the Contractor to adjust the watering rate and frequency depending upon weather conditions.

Revise Basis of Payment as follows:

Basis of Payment: This work will be paid for at the contract unit price per each for TREES and SHRUBS of the species, root type, and plant size specified. Payment will be made according to the following schedule.

(a) Initial Payment. Upon completion of planting, mulch covering, wrapping, and bracing, 90 percent of the pay item(s) will be paid.

(b) Final Payment. Upon inspection and acceptance of the plant material, or upon execution of a third party bond, the remaining ten percent of the pay item(s) will be paid.

56106400 & 56106600 ADJUSTING WATER MAIN 8", 12"

This work shall follow Article 562 of the Standard Specifications and applicable portions of Section 41 of the Standard Specifications for Water and Sewer Construction in Illinois. Work shall consist of adjusting (lowering) an existing water main in the location identified on the plans to no longer conflict with the proposed storm sewer improvements. This pay item shall include all time, labor, and materials to complete the adjustment operation.

If adjustment is not feasible and the water main must be cut and new pipe laid, the cost of time, materials, and labor shall be considered included in the unit cost of this pay item and no additional compensation shall be provided.

The Village of Algonquin Public Works Department shall be notified if the main adjustment requires a shut off and must be approved by the Village prior to performing the work.

Method of Measurement & Basis of Payment: This work shall be paid for at the contract unit price per FOOT for ADJUSTING WATER MAIN 8" or ADJUSTING WATER MAIN 12".

56201600 CORPORATION STOPS 1 1/2"

Description: This work shall consist of all labor, equipment, and materials required to provide and install a new corporation stop of the specified size at locations shown on the plans or as directed by the Engineer.

Material Requirements: Corporation stops shall be fabricated of brass and provided with an outlet suitable for copper connection in accordance with the requirements of AWWA Standard C-800.

Construction Requirements: Corporation stops shall be installed in accordance with the applicable requirements of Section 41-2.12 of the Water and Sewer Specifications.

Basis of Payment: This work will be paid for at the contract unit price per each for CORPORATION STOPS 1 1/2".

56400300 FIRE HYDRANTS TO BE ADJUSTED

Description: This item shall consist of the proper adjustment of the hydrant where indicated on the plans in accordance with the Standard Specifications for Water and Sewer Construction in Illinois, 7th Edition.

Method of Measurement & Basis of Payment: This work will be measured for in payment as EACH. Work shall be paid for as FIRE HYDRANTS TO BE ADJUSTED, EACH.

60610900 COMBINATION CONCRETE CURB AND GUTTER, TYPE M-6.24 (VARIABLE WIDTH GUTTER FLAG)

Description. This work shall consist of constructing Combination Concrete Curb and Gutter, Type M-6.24 (Variable Width Gutter Flag) at locations shown and dimensions detailed in the plans.

This work shall be in accordance with the applicable portions of IDOT Standard Drawing 606001 Concrete Curb, Type B, and Combination Concrete Curb and Gutter and Section 606 of the Standard Specifications

Protective coat, when required, shall be constructed according to Article 420.18 of the Standard Specifications.

Method of Measurement. This work will be measured for payment as defined in Article 606.14 of the Standard Specifications.

Basis of Payment. This work will be paid for at the contract unit price per foot for COMBINATION CONCRETE CURB AND GUTTER, TYPE M-6.24 (VARIABLE WIDTH GUTTER FLAG).

Protective coat will be paid for at the contract unit price per square yard for PROTECTIVE COAT.

K0013030 PERENNIAL PLANTS, WETLAND TYPE, 2" DIAMETER BY 4" DEEP PLUG

Description: This work shall consist of providing perennial plants, as noted on the plans, and generally follow the specifications as noted in Section 254 Planting Perennial Plants, in the "Standard Specifications." The work shall consist of delivery and placement of perennial plants as noted and detailed in the plans with the following specific requirements:

1. Plants included are to be herbaceous plants and native grass as noted on the plan.
2. All plants supplied are to be in 2" x 4" rooted plugs.
3. All plants are to follow a layout of spacing of one and one-half feet (1.5 feet) in rows as noted on the plans.

A 3" mulch layer shall be placed between plug plantings. This mulch layer shall not be paid for separately but included in the cost of PERENNIAL PLANTS, WETLAND TYPE, 2" DIAMETER BY 4" DEEP PLUG.

Plug planting material shall be as follows:

<u>Common Name</u>	<u>Botanical Name</u>	<u>% of Plan Unit Quantity</u>
Little Bluestem	Schizachyrium Scoparium	60.0%
Butterfly Milkweed	Aesclepias Tuberosa	5.0%
Purple Coneflower	Echinacea Purpurea	5.0%
Sky Blue Aster	Aster Azureus	5.0%
Prairie Coreopsis	Coreopsis Palmata	5.0%
Prairie Blazing Star	Liatris Pycnostachya	5.0%
Wild Monarda	Monarda Fistulosa	5.0%
Black-Eyed Susan	Rudbeckia Hirta	5.0%
Golden Alexander	Zizia Aurea	5.0%

Method of Measurement: Perennial plants will be measured for payment per UNIT planted. One hundred (100) perennial plants are equal to one (1) UNIT.

Basis of Payment: This work shall be paid for at the contract unit price per UNIT for PERENNIAL PLANTS, WETLAND TYPE, 2" DIAMETER BY 4" DEEP PLUG and shall also include the 3" mulch layer as described. Payment is incumbent on the health and vigor of the plants after the establishment period, and correction/replacement must be made by the Contractor of those plants not living before full payment is allowed.

K0026830 SHRUB REMOVAL

Description: This work shall consist of removing existing shrubs in locations as identified in the plans. This work shall follow Article 201.08 of the Standard Specifications.

Method of Measurement & Basis of Payment: This work shall be measured and paid for per EACH for SHRUB REMOVAL.

K1005421 SEEDING (SPECIAL)

Description. This work shall consist of placing seed and other materials in accordance with Section 250 of the Standard Specifications and the following or as directed by the Engineer.

Construction Requirements. For areas shown on the plans to be seeded with the item Seeding (Special), the areas shall be seeded in accordance with Article 250.07 of the Standard Specifications using both Seeding Class 4 (MODIFIED) and Seeding Class 4B (MODIFIED) as specified in the special provisions included herein.

Method of Measurement. This work will be measured for payment, complete in place and accepted, in acres of surface seeded.

Basis of Payment. This work will be paid for at the contract unit price per acre for SEEDING (SPECIAL) installed. Price shall include but not be limited to seeds and all other labor, materials, and equipment needed to perform the work as specified herein.

K1005481 SHREDDED BARK MULCH 3"

Description: This work shall consist of furnishing, transporting, and placing mulch in accordance with Section 253 of the Standard Specifications except as specified herein, as shown on the plans, and as directed by the Owner's Representative. The work shall include all mulching for woody plants (PLANTING WOODY PLANTS) and perennials (PLANTING PERENNIAL PLANTS), and all other work described.

Materials: Mulch shall be six-month old, well-rotted, shredded, native hardwood bark mulch, not larger than 4 inches in length and ½ inches in width, free of wood chips and sawdust.

Construction: This work shall be performed in accordance with Article 253 of the Standard Specifications.

Verify subgrade of the landscape bed, so that finish grade, including proper topsoil and mulch thickness, is flush or below the adjacent material as depicted in the plans. Mulch is to be contained by the edge and shall not spill out of landscape beds onto adjacent pavement or turf.

Place mulch around trees, shrubs, and groundcovers and perennials in thickness specified. Make sure to work mulch in under branches and foliage.

Measurement: This item will be measured in square yards around the planting to the depth specified and no deduct will be made for the tree trunk diameter.

Basis of Payment: This work shall be paid for at the contract unit price per square yard for SHREDDED BARK MULCH of the thickness specified and shall include furnishing, transporting, and placing mulch around tree and shrub plantings as shown on the plans, including all materials, labor, or equipment required to complete this work.

X0322871 MAINTENANCE OF EROSION CONTROL SYSTEM

Description: This work shall consist of maintaining the temporary erosion control systems installed by the Contractor on the project. The maintenance shall be performed as directed by the Engineer, to control siltation at all times during the duration of the project.

General: The work shall be performed according to Section 280 of the “Standard Specifications” and the following:

The Maintenance of Erosion Control System work item shall include:

- Any repairs to the various temporary erosion control systems.
- The removal of entrapped sediment.
- Cleaning of any silt filter fabric.
- Other items as specified in project special provisions relating to erosion control.

When a temporary erosion control system is in need of maintenance, the Engineer will give the Contractor written notice. If the Contractor fails to maintain the temporary erosion control systems within 48 hours of receiving the written notice, the Engineer may proceed to maintain the systems as deemed necessary. The cost of this maintenance will be deducted from any compensation due, or which may become due the Contractor under this contract.

Sediment basin(s) shall be cleaned out (accumulated silt removed) any time the basin(s) become 75% filled. Any additional materials and work required by the Engineer will be measured and paid for as specified.

Removed sediment and other materials shall be disposed of according to Article 202.03 of the “Standard Specifications”.

Method of Measurement: Work performed under this pay item shall be submitted by the Contractor to the Engineer on a lump sum basis. The Engineer may use any, all or none of this pay item.

The quantity for this item is established based on the Engineer’s Estimate and the following formula.

<u>Contract Pay Item</u>	<u>Percent of Engineer’s Estimate for Pay Item</u>
<i>Temporary Ditch Checks/Rock Check Dams</i>	20%
<i>Perimeter Erosion Barrier, Special</i>	20%
<i>Inlet & Pipe Protection</i>	20%

<i>Erosion Control Blanket (All Types)</i>	<i>5%</i>
<i>Seeding</i>	<i>5%</i>

** if more than one of these items is included in the pay items then the sum is used. Temporary erosion control seeding, blanket, or Class 7 seeding are not included in the maintenance calculation.*

*The plan-measured quantity for MAINTENANCE OF EROSION CONTROL SYSTEM for this contract is **31,724**.*

Basis of Payment: The unit price for MAINTENANCE OF EROSION CONTROL SYSTEM will be LUMP SUM according to the formula as established above by plan measured quantities. Therefore the LUMP SUM will equal \$31,724.00 of work performed according to Article 109.04 (b) of the “Standard Specifications”.

X0322936 REMOVE EXISTING FLARED END SECTION

Description: This work shall consist of the removal and disposal of flared end section(s) as shown on the Plans in accordance with Section 551.

Basis of Payment: This work shall be measured and paid for at the contract unit price per EACH for REMOVE EXISTING FLARED END SECTION and shall include all labor, excavation, backfill, material, and equipment to complete this item as specified.

X0324045 SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE REMOVAL

Description: This work shall consist of the removal of a stabilized construction entrance and all items necessary for removal of the stabilized construction entrance. This includes (but not limited to) excess aggregate for mountable berms, aggregate radii abutting temporary, permanent, or existing pavement; cellular confinement grids; all unnecessary aggregate within 20 feet within the original lines and dimensions in which the original entrance was constructed. All methods of removal shall be approved by the engineer. Material shall be disposed of according to Article 202.03 or as directed by the Engineer.

Basis of Payment: This work shall be measured and paid for at the contract unit price per EACH for SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE REMOVAL and shall include all labor, excavation, and disposal of material used for the stabilized construction entrance.

X0324079 EXISTING FIELD TILE REMOVAL

Description: This work shall consist of the removal and disposal or salvaging of existing field tiles of the various sizes and types, excavation, backfilling (if required) as indicated in locations on the plans or as directed by the Engineer.

This work shall be in accordance with the applicable portions of Section 501 of the Standard Specifications which apply to Pipe Culvert Removal. Trenches resulting from the removal of existing field tile which will be allowed below the final grade shall be backfilled to the applicable requirements of Article 550.07. Any damage made by construction activities to portions of the existing field tile which is to remain shall be repaired with materials matching "in like kind" to the satisfaction of the Engineer at no additional cost to the contract.

With the Engineer's approval, any drain tile deemed not to be in conflict with the proposed improvements can be crushed or broken at 10-foot intervals and may remain in place without removal.

Basis of Payment: This work shall be paid for at the contract unit price per FOOT for EXISTING FIELD TILE REMOVAL regardless of size or type. Backfilling (if required) for the removal of the existing field tile will not be paid for separately but will be considered included in the unit price bid for EXISTING FIELD TILE REMOVAL.

X0324775 SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE MAINTENANCE

Description: This work shall consist of maintaining stabilized construction entrances that have become ineffective as a result of standard operations and natural forces. This work will include the removal of proper disposal of excess materials and the delivery and placing of aggregate in the manner described in SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE.

This pay item shall not be paid for each time maintenance is required but shall consist of maintenance over the life of the project for which the entrance is needed.

Basis of Payment: This work shall be measured for payment to the outside dimensions of the material being removed and the area calculated in SQUARE YARDS. All excavation and grading necessary to remove and replace the sediment fill aggregate shall not be paid for separately but included in the cost of SEDIMENT CONTROL, STABILIZED ENTRANCE MAINTENANCE.

X0325476 RADAR VEHICLE DETECTION SYSTEM

Description.

This work shall consist of furnishing and installing a radar/microwave vehicle detection system as specified and/or as shown on the plan. This pay item shall include all necessary work and equipment required to have a fully operational system including but not limited to the detector unit/s, the interface unit and all the necessary hardware, cable and accessories required to complete the installation in accordance with the manufacturer's specifications.

The radar/microwave vehicle detection system shall work under all weather conditions, including rain, freezing rain, snow, wind, dust, fog, and changes in temperature and light. It shall work in an ambient temperature range of -34 to 74 degrees Celsius. It shall have a max power output of 75 watts or less. The detection system shall be capable of detecting stopped vehicles, this is often referred to as Frequency-Modulated Continuous Wave (FMCW) or true-presence detection. Each detector system shall be capable of

detecting volume, occupancy, classification, and speed while gathering this data for each detection field/travel or turn lane configured in the device.

The radar/microwave vehicle detection system shall be compatible with the District's approved traffic controller assemblies utilizing NEMA TS 1 or NEMA TS 2 controllers and cabinet components for full time operation. The radar/microwave vehicle detection system shall provide a minimum of one interface unit that has Ethernet connectivity, surge protection and shall be capable of supporting a minimum of 4 detector units. In cases where vender utilizes separate detector units between uptight and advance detection, Ethernet connectivity and surge protection shall be capable of connecting to all detector units using one IP address unless otherwise approved by the engineer.

The far back radar/microwave detection shall have a detection range of 400 feet or better.

A representative from the supplier of the radar/microwave vehicle detection system shall supervise the installation and testing of the radar/microwave vehicle detection system and shall be present at the traffic signal turn-on inspection. Once the radar/microwave vehicle detection system is configured, it shall not need reconfiguration to maintain performance, unless the roadway configuration or the application requirements change.

The mounting location/s of the detector unit/s shall be per the manufacturer's recommendations. If an extension mounting assembly is needed, it shall be included in this item. All holes drilled into signal poles, mast arms, or posts shall require rubber grommets to prevent chafing of wires.

The radar/microwave vehicle detection system shall be warrantied, free from material and workmanship defects for a period of two years from final inspection. Warranty shall be furnished and provided to the District at the time of traffic signal turn on and shall include appropriate contact information (Email and direct telephone) to which all warranty concerns may be directed.

Basis of Payment.

This work shall be paid for at the contract unit price each for RADAR VEHICLE DETECTION SYSTEM, SINGLE APPROACH, STOP BAR; RADAR VEHICLE DETECTION SYSTEM, SINGLE APPROACH, FAR BACK; RADAR VEHICLE DETECTION SYSTEM, SINGLE APPROACH, STOP BAR AND FAR BACK, the price of which shall include the cost for all of the work and material described herein and includes furnishing, installing, delivery, handling, testing, set-up and all appurtenances and mounting hardware necessary for a fully operational radar vehicle detection system.

X0326694 PLUG EXISTING STORM SEWERS

Description: This work shall consist of plugging and abandoning existing storm sewers where shown on the plans in existing manholes, at right-of-way lines or as directed by the engineer. The pipe end shall be capped with bricks and mortar to seal the pipe invert with 24" of lateral length along each pipe to be filled with a slurry seal or other method approved by the engineer. This work shall be in conformance with Article 550 of the Standard Specifications.

Method of Measurement: This work shall be measured for payment by CUBIC YARD of brick and mortar and slurry seal for each storm sewer to be plugged.

Basis of Payment: This work shall be paid for at the contract unit price for PLUG EXISTING STORM SEWERS per CUBIC YARD. This item includes all material, excavation, and labor to complete the operation as described.

X0327036 BIKE PATH REMOVAL

Description: This work shall consist of the removal of the existing bike path/multi-use trail at the location(s) identified in the plans. This work shall conform to Articles 440.03 and 440.06 of the Standard Specifications.

Method of Measurement & Basis of Payment: Removal of the existing hot-mix asphalt path will be measured and paid for per SQUARE YARD removed. This work will be paid for at the contract unit price per SQUARE YARD for BIKE PATH REMOVAL.

X0327297 REMOVE BARRICADES

Description. This work shall consist of removing existing barricades at the Huntley-Boyer-Longmeadow intersection and at the west limit of Longmeadow on this project and delivering them to the Kane County Division of Transportation facility at 41W011 Burlington Road, St. Charles, IL or as directed by the Engineer.

Construction Requirements. These barricades shall be removed once the work at the intersection of Longmeadow Parkway and Randall Road has been completed, including the traffic signal and pavement markings. The barricades shall not be removed until directed by the Engineer.

Basis of Payment. This work will be paid for at the contract unit price per each for REMOVE BARRICADES.

X0426200 DEWATERING

This work shall be done in accordance with the applicable portions of Section 281 of the Standard Specifications and shall include the furnishing, installing, maintaining and removing of a temporary dewatering ditch in locations where dewatering is necessary.

MATERIALS:

1. SEDIMENT CONTAINER FILTER BAG - Sediment Control Filter Bags must be ACF Environmental ERO-TEX dewatering filter bag, US Fabrics filter bag, or JMD Enviro-Protection filter bag of the size required to adequately filter pumped water per the manufacturers' specifications.
2. TEMPORARY DEWATERING SUMP - Materials for Dewatering Sump must be 2 inch Coarse Aggregate and a filter fabric, with a ¼ to ½ inch hardware cloth wire placed around the standpipe prior to attaching the filter fabric, as shown on the detail Drawings.

3. FLOCCULATION POWDER - The polymer must be a water-soluble anionic polyacrylamide (PAM) used to minimize soil erosion, bind soil particles, remove suspended particles, and act as a construction aide. All site-specific soils must be tested by a qualified person each time a PAM is used. The polymer must be used in accordance with manufacturer's guidelines and as approved by the Engineer.
 - a. Anionic PAM mixture must have $\leq 0.05\%$ free acrylamide monomer by weight as established by the Food and Drug Administration (FDA) and the Environmental Protection Agency (EPA).
 - b. The PAM mixture must be accompanied by manufacturers written instructions to ensure proper (1) Product and Site Preparation, (2) Application, (3) Maintenance/Re-application, (4) Storage, and (5) Safety, in accordance with Occupational Health and Safety Administration (OSHA) material safety data sheet (MSDS) requirements and other applicable guidelines including manufacturer's recommendations for specified use.
 - c. Anionic PAM application must comply with all federal, state, and local laws, rules or regulations governing anionic PAM. The Contractor will be responsible for securing required permits.
 - d. In addition to soil testing, a Qualified Person must design the installation plan for the polymers based on mix time and point of entry.
 - e. The materials used must be harmless to plant and aquatic life.
 - f. Different types of polymers may be required for each soil type or combination of soils. The manufacturer or supplier will provide general written application methods, based on site conditions, such as slope and soil type.
4. VISQUENE/PLASTIC LINER – A plastic liner passing ASTM E1745 Class A, B, and C shall be used to line the ditch.
5. JUTE NETTING –
 - a. Jute netting must be of a uniform, open, plain weave, undyed and unbleached single jut yarn. The yarn must be of loosely twisted construction and must not vary in thickness by more than one-half its normal diameter.
 - b. Minimum width must be 48 inches, + or – 1 inch from manufacturer's rated width.
 - c. Seventy-eight warp ends per 4 feet of width.
 - d. Forty one weft ends per yard.
 - e. Weight must average 1.22 lbs per linear yard with a tolerance of + or – 5%.
 - f. Jute netting must be used in conjunction with polymer (PAM) as per Engineer direction.
6. Other Items - All other materials such as straw bales, lathe, etc shall be in compliance with Section 280 of the Standard Specifications and must meet commercial grade standards and must be approved by the Engineer before being incorporated into the Project.

CONSTRUCTION METHODS:

Temporary Dewatering Ditch – This work shall include the furnishing, installing, maintaining and removing of the Temporary Dewatering Ditch as directed by the Engineer to provide a drainage path to convey the flow of water pumped from the various work zones in a positive direction toward the creek. The bottom of the ditch is to be constructed with VISQUENE/PLASTIC LINER with an outside edge of hay bales, staked into the ground and a layer of JUTE NETTING with FLOCCULATION POWDER. Also, AGGREGATE DITCH CHECKS must be provided according to the plans to provide an energy dissipater with a layer of JUTE NETTING over each AGGREGATE DITCH CHECK. Behind each AGGREGATE DITCH CHECK

the contractor must place a FLOCCULATION LOG as directed by the engineer. The VISQUENE/PLASTIC LINER, Riprap and JUTE NETTING must be inspected by the Engineer, before the TEMPORARY DEWATERING DITCH may convey flow.

1. Dewatering:

- a. TEMPORARY DEWATERING SUMP. Pumping water from open trenches or other areas must be performed in a manner to minimize the turbidity of the pumped water in accordance with Illinois Urban Manual IL-650. This pumping shall utilize an inlet hose with filter fabric over the intake and positioned in an aggregate-filled hole near the bottom of the trench. The intake hose shall be supported on a floatation or similar device so as not to pump directly from the bottom of the basin, trench, etc. Water must be pumped directly into a Sediment Containment Filter Bag. All materials, labor and excavation required to prepare the TEMPORARY DEWATERING SUMP are included in the cost of DEWATERING.
- b. SEDIMENT CONTAINER FILTER BAG. Water must be pumped directly to a SEDIMENT CONTAINER FILTER BAG. A SEDIMENT CONTAINER FILTER BAG must be used according to the manufacturer's instructions, as modified by the Contract Drawings and Specifications. The SEDIMENT CONTAINER FILTER BAG must be placed on 4" of compacted CA-6 bedding.

2. FLOCCULATION POWDER & FLOCCULATION LOG:

- a. All vendors and suppliers of polyacrylamide (PAM), PAM mix or blends must present or supply a written toxicity report which verifies that the PAM, PAM mix or blend exhibits acceptable toxicity parameters which meet or exceed the requirements for the state and federal water quality standards. No Cationic formulations of PAM, PAM blends, polymers of Chitosan are allowed for use under this Specification.

The manufacturer or supplier must provide a product expiration date for anionic PAM mixtures based on product expiration date of PAM in pure form.

The application method must provide uniform coverage to the target area and avoid drift to non-target areas. The applicator of anionic PAM must document, at the time of application, the following:

- i. Name of applicator
- ii. Application rate per acre
- iii. Date applied
- iv. Product type
- v. Weather conditions during application
- vi. Method of application

Copies of this documentation must be entered into the Contractor's monitoring log or project diary and made available upon request.

Unused liquid anionic PAM mixtures must be minimized. Excess material will not be applied at a rate greater than the maximum application rate. Disposal must not occur in Waters of the U.S. (W.U.S.) and storm water conveyance systems (i.e. Storm sewer manholes, storm sewer inlets, ditches, and culverts).

Anionic PAM mixtures must achieve P 80% reduction in soil loss as measured by a 1 hour storm duration 2"/hour rainfall simulator test performed in accordance with methods used by Bubenzer and Patterson (1982) as pre-qualification for field testing.

Performance of anionic PAM mixtures must be verified and field tested. The manufacturer must provide a toxicological report for the Polymer Binder performed by a third-party, EPA approved laboratory.

Anionic PAM use must conform to all federal, state, and local laws, rules, and regulations regarding use, discharge, and disposal of chemical materials.

- b. FLOCCULATION LOG Application: A FLOCCULATION LOG is a semi-hydrated polyacrylamide block that when placed within storm water or construction site discharge, will remove fine particles and reduce NTU values. Placement of the flocculation log should be as close to the source of particle suspension as possible. Ideal performance of the flocculation logs results when used in conjunction with other best management practices. Each flocculation log is formulated for the soil and water chemistry at the site. Soil and water samples shall be taken by the Contractor and tested to determine which formula flocculation log is needed along with proper placement.
- c. The POLYMER and FLOCCULATION LOG will be paid for at the contract unit price for each respective pay item.

MEASUREMENT/PAYMENT:

1. DEWATERING. This work will be measured for payment as LUMP SUM and shall include furnishing, excavation, installation, maintenance and removal of TEMPORARY DEWATERING SUMP, JUTE NETTING, and SEDIMENT CONTAINER FILTER BAG and items required for the construction of the TEMPORARY DEWATERING DITCH as detailed on the plans such as excavation, VISQUENE/PLASTIC LINER, filter fabrics, straw bales, lathe and any other related work necessary to provide a work zone free of excessive water levels. Maintenance efforts such as excavation of sediment from TEMPORARY DEWATERING DITCH and/or replacement of the SEDIMENT CONTAINER FILTER BAG will not be paid for separately but will be considered included in the cost for DEWATERING.

Pumping: Means and methods of pumping water from excavated trenches, footing excavations, or other areas will be considered included in the work item to which it pertains and will not be paid for separately.

2. FLOCCULATION POWDER and FLOCCULATION LOG: These items of work will not be measured and paid for separately per the contract unit pay item as specified in these Special Provisions.
3. AGGREGATE DITCH CHECKS – The AGGREGATE DITCH CHECKS identified to be in the Temporary Dewatering Ditch on the downstream side of the sediment containment filter bag will be paid for separately at the contract unit price per TON for AGGREGATE DITCH CHECKS.

X1400081 FULL-ACTUATED CONTROLLER AND TYPE SUPER P CABINET (SPECIAL)

Description. This work shall consist of furnishing and installing a traffic actuated solid state digital controller in the controller cabinet of the type specified, meeting the requirements of Section 857 of the Standard Specifications, as modified herein, including malfunction management unit, load switches and flasher relays, with all necessary connections for proper operation.

If the intersection is part of an existing system and/or when specified in the plans, this work shall consist of furnishing and installing a properly configured traffic actuated solid state controller from the following list of approved models:

Firmware type	KDOT ATMS Firmware Supported	Hardware	Command protocol
Eagle SEPAC (M52)	4.08	NEMA	NTCIP
Econolite ASC/3	2.61	NEMA	NTCIP
Econolite ASC/3 (Cobalt, 2070-1C)	32.62	ATC	NTCIP
Intelight MaxTime	1.8	NEMA	NTCIP

Materials. Add the following to Article 857.02 of the Standard Specifications:

For installation as a stand-alone traffic signal, connected to a closed loop system or integrated into an advance traffic management system (ATMS), controllers shall be Econolite ASC/3S-1000 or Eagle/Siemens M52 unless specified otherwise on the plans or elsewhere on these specifications. Only controllers supplied by one of the District One approved closed loop equipment suppliers will be allowed. Unless specified otherwise on the plans or these specifications, the controller shall be of the most recent model and software version supplied by the equipment supplier at the time of the traffic signal TURN-ON. A removable controller data key shall also be provided. Individual load switches shall be provided for each vehicle, pedestrian, and right turn over lap phase. The controller shall prevent phases from being skipped during program changes and after all preemption events and shall inhibit simultaneous display of circular yellow and yellow arrow indications.

For integration into an ATMS such as Centrac, Tactics, or TransSuite, the controller shall have the latest version of NTCIP software installed. For operation prior to integration into an ATMS, the controller shall maintain existing close loop management communications.

Add the following to Article 1074.03 of the Standard Specifications:

- (a) (6) Cabinets shall be designed for NEMA TS2 Type 1 operation. All cabinets shall be pre-wired for a minimum of eight (8) phases of vehicular, four (4) phases of pedestrian and four (4) phases of overlap operation.
- (b) (1) Revise “conflict monitor” to read “Malfunction Management Unit”

- (b) (5) Cabinets – Provide 1/8" (3.2 mm) thick unpainted aluminum alloy 5052-H32. The surface shall be smooth, free of marks and scratches. All external hardware shall be stainless steel.
- (b) (6) Controller Harness – Provide a TS2 Type 2 “A” wired harness in addition to the TS2 Type 1 harness.
- (b) (7) Surge Protection – Shall be a 120VAC Single phase Modular filter Plug-in type, supplied from an approved vendor.
- (b) (8) BIU – shall be secured by mechanical means.
- (b) (9) Transfer Relays – Solid state or mechanical flash relays are acceptable.
- (b) (10) Switch Guards – All switches shall be guarded.
- (b) (11) Heating – One (1) 200 watt, thermostatically-controlled, electric heater.
- (b) (12) Lighting – One (1) LED Panel shall be placed inside the cabinet top panel and one (1) LED Panel shall be placed on each side of the pull-out drawer/shelf assembly located beneath the controller support shelf. The LED Panels shall be controlled by a door switch. The LED Panels shall be provided from an approved vendor.
- (b) (13) The cabinet shall be equipped with a pull-out drawer/shelf assembly. A 1 ½ inch (38mm) deep drawer shall be provided in the cabinet, mounted directly beneath the controller support shelf. The drawer shall have a hinged top cover and shall be capable of accommodating one (1) complete set of cabinet prints and manuals. This drawer shall support 50 lbs. (23 kg) in weight when fully extended. The drawer shall open and close smoothly. Drawer dimensions shall make maximum use of available depth offered by the controller shelf and be a minimum of 18 inches (610mm) wide.
- (b) (14) Plan & Wiring Diagrams – 12” x 15” (305mm x 406mm) moisture sealed container attached to door.
- (b) (15) Detector Racks – Fully wired and labeled for four (4) channels of emergency vehicle pre-emption and sixteen channels (16) of vehicular operation.
- (b) (16) Field Wiring Labels – All field wiring shall be labeled.
- (b) (17) Field Wiring Termination – Approved channel lugs required.
- (b) (18) Power Panel – Provide a nonconductive shield.
- (b) (19) Circuit Breaker – The circuit breaker shall be sized for the proposed load but shall not be rated less than 30 amps.
- (b) (20) Police Door – Provide wiring and termination for plug in manual phase advance switch.

Basis of Payment. This work will be paid for at the contract unit price each for FULL-ACTUATED CONTROLLER AND TYPE SUPER R CABINET (SPECIAL).

X2501800 SEEDING, CLASS 4 (MODIFIED)

Description. This work shall consist of placing seed and other materials in accordance with Section 250 of the Standard Specifications and the plans and/or as directed by the Engineer.

Construction Requirements. The seeding mixture for Class 4 shall be modified to the following:

MODIFIED- IDOT Class 4 Native Grass Mixture

Spring Oats	60 lbs. / acre
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Native Grasses (below)	18 lbs. / acre
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Native Grasses:		
Common Name	Scientific Name	Lbs/Acre
Big Bluestem	<i>Andropogon gerardi</i>	4
Little Bluestem	<i>Andropogon scoparius</i>	5
Side-Oats Grama	<i>Bouteloua curtipendula</i>	4
Canada Wild Rye	<i>Elymus canadensis</i>	1
Switchgrass	<i>Panicum virgatum</i>	2
Indian Grass	<i>Sorghastrum nutans</i>	2
	Total	18

Method of Measurement. This work will be measured for payment, complete in place and accepted, in acres of surface seeded.

Basis of Payment. This work will be paid for at the contract unit price per acre for SEEDING, CLASS 4 (MODIFIED) installed. Price shall include but not be limited to seeds and all other labor, materials, and equipment needed to perform the work as specified herein.

X2502024 SEEDING, CLASS 4B (MODIFIED)

Description. This work shall consist of placing seed and other materials in accordance with Section 250 of the Standard Specifications and the plans and/or as directed by the Engineer.

Construction Requirements. The seeding mixture for Class 4B shall be modified to the following:

MODIFIED- IDOT Class 4B Wetland Grass & Sedge Mixture

Spring Oats	60 lbs. / acre
Wetland Grasses (below)	12.4 lbs. / acre

Wetland Grasses:		
Common Name	Scientific Name	Lbs/Acre
Blue Joint Grass	<i>Calamagrostis canadensis</i>	1.44
Lake-Bank Sedge	<i>Carex lacustris</i>	0.36
Awl-Fruited Sedge	<i>Carex tribuloides</i>	0.36
Tussock Sedge	<i>Carex stricta</i>	0.72

Fox Sedge	<i>Carex stipata</i>	0.72
Needle Spike Rush	<i>Eleocharis acicularis</i>	0.36
Blunt Spike Rush	<i>Eleocharis ovata</i>	0.36
Fowl Manna Grass	<i>Glyceria striata</i>	1.68
Common Rush	<i>Juncus effusus</i>	0.72
Slender Rush	<i>Juncus tenuis</i>	0.36
Torrey's Rush	<i>Juncus torreyi</i>	0.36
Rice Cut Grass	<i>Leersia oryzoides</i>	0.2
Switchgrass	<i>Panicum virgatum</i>	2
Hard-Stemmed Bulrush	<i>Scirpus acutus</i>	0.36
Dark Green Rush	<i>Scirpus atrovirens</i>	0.36
River Bulrush	<i>Scirpus fluviatilis</i>	1.2
Softstem Bulrush	<i>Schoenoplectus tabernaemontani</i>	0.36
Cord Grass	<i>Spartina pectinata</i>	0.48
Total		12.4

Method of Measurement. This work will be measured for payment, complete in place and accepted, in acres of surface seeded.

Basis of Payment. This work will be paid for at the contract unit price per acre for SEEDING, CLASS 4B (MODIFIED) installed. Price shall include but not be limited to seeds and all other labor, materials, and equipment needed to perform the work as specified herein.

X2511630 EROSION CONTROL BLANKET (SPECIAL)

Description: This work shall consist of furnishing and placing erosion control blanket over seeded areas on slopes 3:1 or flatter in locations as indicated on the plans. The work shall be performed according to Article 251.04 of the “Standard Specifications”.

Materials: The erosion control blanket shall meet the requirements of Article 1081.10 of the “Standard Specifications”, except that:

The blanket material shall be 100% biodegradable leno-woven agricultural straw.

List of Vendors & Product Name:

- Tensar/North American Green – S75BN
- ADS Geosynthetics – 00S2AN
- Western Excelsior Corporation – Excel SR-1AN (All-Natural)
- American Excelsior Company – Premier Single Straw

- East Coast Erosion Control – ECS-1B
- Erosion Control Blanket.com – S31 BD “Big Daddy”

Each blanket will be secured with a 12” degradable stake. Securing devices are not paid for separately but included in the cost of the pay item.

Method of Measurement: This work will be measured for payment in place in square yards of actual area covered.

Basis of Payment: This work will be paid for at the contract unit price per square yard for EROSION CONTROL BLANKET (SPECIAL). *The unit price shall include all equipment, materials and labor required to furnish and place the erosion control blanket*

X2510635 HEAVY DUTY EROSION CONTROL BLANKET, SPECIAL

Description: This work shall consist of furnishing and placing heavy-duty erosion control blanket (turf reinforcement mat) over in anticipated heavy drainage flow ditch locations as indicated on the plans. The work shall be performed according to Article 251.04 of the “Standard Specifications”.

Materials: The erosion control blanket shall meet the requirements of Article 1081.10 of the “Standard Specifications”, except that:

The blanket material shall consist of polyolefin fibers positioned between two high-strength, biaxially oriented nets and mechanically bound together by parallel stitching with polyolefin thread. The matrix shall possess strength and elongation properties to limit stretching and shall be maintained in high-flow conditions.

List of Vendors & Product Name:

- Tensar/North American Green – C350 TRM
- ADS Geosynthetics – PP5-10 TRM
- Western Excelsior Corporation – PP5-10 TRM
- Propex Landlok TRM 1051/1060
- East Coast Erosion Control – ECC-3 Coconut TRM

Each blanket will be secured with a 12” degradable stake. Securing devices are not paid for separately but included in the cost of the pay item.

Method of Measurement: This work will be measured for payment in place in square yards of actual area covered.

Basis of Payment: This work will be paid for at the contract unit price per square yard for HEAVY DUTY EROSION CONTROL BLANKET, SPECIAL. *The unit price shall include all equipment, materials and labor required to furnish and place the erosion control blanket*

X2511640 EROSION CONTROL BLANKET (MODIFIED)

Description: This work shall consist of furnishing and placing erosion control blanket over seeded areas on slopes 3:1 or flatter , anticipated in low-to-median flow in locations as indicated on the plans. The work shall be performed according to Article 251.04 of the “Standard Specifications”.

Materials: The erosion control blanket shall meet the requirements of Article 1081.10 of the “Standard Specifications”, except that:

Blanket shall consist of double net structure an integration of leno-woven coconut (coir) fiber in matrix of 70% agricultural straw / 30% coconut fiber.

List of Vendors & Product Name:

- Tensar/North American Green – SC150BN
- ADS Geosynthetics – 0CS2TT
- Western Excelsior Corporation – Excel CS-3
- American Excelsior Company – Premier Straw/Coconut
- East Coast Erosion Control – ECSC-2
- Erosion Control Blanket.com – SC32
- Propex – Landlok ECB-CS2

Each blanket will be secured with a 12” degradable stake. Securing devices are not paid for separately but included in the cost of the pay item.

Method of Measurement: This work will be measured for payment in place in square yards of actual area covered.

Basis of Payment: This work will be paid for at the contract unit price per square yard for EROSION CONTROL BLANKET (MODIFIED). *The unit price shall include all equipment, materials and labor required to furnish and place the erosion control blanket*

XXXXXXXX EROSION CONTROL BLANKET (SPECIAL 2)

Description: This work shall consist of furnishing and placing erosion control blanket over plug plantings and promotion of wetland-type plantings. The work shall be performed according to Article 251.04 of the “Standard Specifications”.

Materials: The erosion control blanket shall meet the requirements of Article 1081.10 of the “Standard Specifications”, except that:

The blanket material shall be 100% agricultural straw with degradable thread.

List of Vendors & Product Name:

- Tensar/North American Green – DS75
- Western Excelsior Corporation – Excel SR-1RG (Rapid-Go)

- Erosion Control Blanket.com – S31 UVD
- Propex – Landlok S2-RD

Each blanket will be secured with a 12” degradable stake. Securing devices are not paid for separately but included in the cost of the pay item.

Method of Measurement: This work will be measured for payment in place in square yards of actual area covered.

Basis of Payment: This work will be paid for at the contract unit price per square yard for EROSION CONTROL BLANKET (SPECIAL 2). *The unit price shall include all equipment, materials and labor required to furnish and place the erosion control blanket*

X2800400 PERIMETER EROSION BARRIER, SPECIAL

Description: This work shall consist of constructing, removing, and disposing of perimeter erosion barrier, special as part of the project’s temporary erosion control system. Perimeter erosion control barrier, special shall be utilized adjacent to existing wetlands.

General: The work shall be performed according to Section 280 of the “Standard Specifications,” special provision 28000400 “PERIMETER EROSION BARRIER” and the following:

Materials:

Geotextile Requirements: The geotextile used for the temporary silt fence shall be classified as supported (with a wire of polymeric mesh backing) or unsupported (no backing). The temporary silt fence geotextile shall meet the requirements of the Table included below. All numeric values except Apparent Opening Size (AOS) represent Minimum Average Roll Values (MARV as defined in ASTM D4439). The values for AOS are the Maximum Average Roll Values.

Table – Temporary Silt Fence Requirements

Requirements	Test Methods	Wire Backed Supported Silt Fence^a
Maximum Post Spacing		4 feet
Grab Strength	ASTM D4632	
Machine Direction		90 lbs
X-Machine Direction		90 lbs
Permittivity ^b	ASTM D4491	0.05 sec ⁻¹
Apparent Opening Size	ASTM D4751	0.024in maximum average roll value
Ultraviolet Stability	ASTM D4355	70% after 500 hours of exposure

Notes:

1. Silt fence support shall consist of 14-gauge steel wire with a mesh backing of 6"x6" or prefabricated polymeric mesh of equivalent strength.
2. These default filtration property values are based on empirical evidence with a variety of sediments. For environmentally sensitive areas, a review of previous experience and/or site or regionally specific geotextile tests should be performed to confirm the suitability of these requirements.

The wire support fence shall:

- 1) Be a minimum of 14-gauge
- 2) Have a minimum of six horizontal wires
- 3) The maximum vertical wire spacing shall be 6"

Method of Measurement: This work will be measured for payment in place by FOOT.

Basis of Payment: This work will be paid for at the contract unit price per FOOT for PERIMETER EROSION BARRIER, SPECIAL. The unit price shall include all work and materials necessary to properly install the barrier and to remove and dispose of the used materials at the completion of the project. Maintenance requirements shall be included and paid for according to Section 280 of the "Standard Specifications."

X5510100 STORM SEWER REMOVAL

Description: This work shall consist of removing existing elliptical storm sewer pipe, 36"x24" equivalent, in locations identified on the plans. This work shall follow applicable portions of Article 551 of the special provisions.

Method of Measurement: This work shall be measured for payment by FOOT of elliptical storm sewer to be removed.

Basis of Payment: This work shall be paid for at the contract unit price per FOOT for STORM SEWER REMOVAL. This item includes all material, excavation, and labor to complete the operation as described. Backfill of the existing trench shall be paid for separately with TRENCH BACKFILL.

X6020095 MANHOLES, TYPE A, 4' DIAMETER, TYPE 1 FRAME, CLOSED LID, RESTRICTOR PLATE

Description: This work shall consist of constructing a Type A manhole of the diameter specified with restrictor in accordance with Sections 602 and 1006 of the Standard Specifications and the plans and/or as directed by the Engineer.

Construction Requirements: Construction shall conform to the details shown in the plans, all applicable Standard Drawings, and all applicable portions of Sections 602 and 1006 of the Standard Specifications.

Method of Measurement: This work will be measured for payment, complete in place and accepted, in units of EACH.

Basis of Payment: This work will be paid for at the contract unit price per EACH for MANHOLES, TYPE A, 4' DIAMETER, TYPE 1 FRAME, CLOSED LID, RESTRICTOR installed. Price shall include but not be limited to all frames, grates, lids, sand cushion, steps, 6" concrete wall, flat slab tops, all excavation and backfilling, and all other labor, materials, and equipment needed to perform the work as specified herein.

X6020094 MANHOLES, TYPE A, 6' DIAMETER, TYPE 1 FRAME, CLOSED LID, RESTRICTOR PLATE

Description: This work shall consist of constructing a Type A manhole of the diameter specified with restrictor in accordance with Sections 602 and 1006 of the Standard Specifications and the plans and/or as directed by the Engineer.

Construction Requirements: Construction shall conform to the details shown in the plans, all applicable Standard Drawings, and all applicable portions of Sections 602 and 1006 of the Standard Specifications.

Method of Measurement: This work will be measured for payment, complete in place and accepted, in units of EACH.

Basis of Payment: This work will be paid for at the contract unit price per EACH for MANHOLES, TYPE A, 6' DIAMETER, TYPE 1 FRAME, CLOSED LID, RESTRICTOR installed. Price shall include but not be limited to all frames, grates, lids, sand cushion, steps, 6" concrete wall, flat slab tops, all excavation and backfilling, and all other labor, materials, and equipment needed to perform the work as specified herein.

X6026050 SANITARY MANHOLES TO BE ADJUSTED

Description: This work shall be done in accordance with Section 602 of the Standard Specifications and shall consist of the adjustment of sanitary manholes. Non-hardening butyl rubber mastic sealant; minimum thickness ¼" shall be used between adjusting rings in place of mortar. In locations where external frame seals exist, it shall be removed and replaced and disposed of and an internal/external frame seal shall be installed. In locations where an internal frame seal exists, it shall be removed and disposed of and an internal/external seal shall be installed. In locations where there are no existing frame seals, an internal/external frame seal shall be installed. The installation of the internal/external frame seal will not be paid for separately and should rather be included in this pay item.

The Internal/External Frame seal shall consist of the following:

- a. Provide frame seals consisting of a flexible internal rubber sleeve, rubber ring, and external rubber sleeve and extension, and stainless steel compression bands.
- b. Rubber sleeve, ring, butyl tape, and extension:
 - a. Provide rubber sleeve and extension complying with ASTM D412 and ASTM D2240.
 - b. Provide rubber ring complying with ASTM D-2000.
 - c. Provide butyl tape: Comply with 1000% minimum webbing @ 77 degrees F, 500% minimum elongation @ 32 degrees F, and maximum 75 psi compressibility @ 7 degrees F.
 - d. Provide sleeve with a minimum thickness of 0.062" and unexpanded external vertical heights of 10 to 12 inches.
 - e. Provide an extension having a minimum thickness of 0.062"

- f. Comply with a minimum 1500 psi tensile strength, maximum of 18 percent compression set and a hardness (durameter) of 48+/-5.
- c. Compression band:
 - a. Provide compression band to compress the sleeve to the manhole.
 - b. Use 16-gauge stainless steel conforming to ASTM A240 Type 304 with no welded attachments and having a minimum width of ½”.
 - c. Make a watertight seal having a minimum adjustment range of 2 diameter inches.
 - d. Provide stainless steel screws, bolts, and nuts conforming to ASTM F593 and 594, Type 304.
- d. Acceptable Products:
 - a. Adaptor Inc. Internal/External Adaptor Seal
 - b. Or Equal.

The Internal/External Frame Seal shall be installed as follows:

- e. Install internal/external rubber gasket on the manhole chimney. Provide watertight gasket to eliminate leakage between the internal/external frame seal and the adjusting ring and between each adjusting ring down to and including cone section.
- f. Clean surface and prepare the lower 2 inches of the manhole frame and exterior of all adjusting rings and cone section/corbel surfaces.
- g. Install internal rubber gasket in accordance with manufacturer’s recommendation.
 - a. Field-verify for suitable dimensions and layout prior to installation.
 - b. Realign frame as required.
- h. Repair and apply mortar grout to the adjusting rings as required to provide a smooth circular surface for the rubber gasket.
- i. Install external rubber gasket in accordance with manufacturer’s recommendations.
 - a. Field-verify for suitable dimensions and layout before installation
 - b. Utilize sealing caulk where required
 - c. Provide chimney seal extensions as required.
- j. Test installation by flooding area around the manhole with water before backfilling and surface restoration. Gaskets are required to provide watertight seal at openings between the frame and adjusting rings and between adjacent adjusting rings down to the cone/corbel section.
- k. Reinstall and retest failing gaskets at no additional cost to the contract.

Basis of Payment: This work shall be paid for at the contract unit price for each of SANITARY MANHOLES TO BE ADJUSTED, which price shall include all of the above.

X6061100 CONCRETE MEDIAN, TYPE SB (SPECIAL)

Description. This work shall consist of constructing Concrete Median, Type SB in accordance with Section 606 of the Standard Specifications, Highway Standard 606301, and the detail on the plans.

Construction Requirements. The optional keyed construction joint or joints shown on Standard 606301 may be required in order to conform to the Construction Staging and Maintenance of Traffic shown on the plans. No extra compensation will be provided for construction either one or two keyed construction joints.

Method of Measurement. This work will be measured for payment in place and the area computed in square feet. Concrete curb and gutter around solid concrete median will not be measured separately for payment.

Basis of Payment. This work will be paid for at the contract unit price per square foot for CONCRETE MEDIAN, TYPE SB SPECIAL).

X8360095 EXISTING LIGHT POLE FOUNDATION ADJUSTMENT

Description: This work shall consist of adjusting an existing subdivision entrance light pole foundation to finished grade as a result of the proposed improvements.

If the change in grade results in a foundation that cannot be adjusted, this pay item shall include the cost of pouring a new foundation adjacent to the existing foundation, de-energizing and relocating the existing light to the new foundation, and reconnecting/reenergizing the service. Any additional wiring, trenching or splicing required to reconnect the light pole service in its new location shall be considered included in this pay item.

Basis of Measurement & Payment - Work shall be paid for and measured by EACH for EXISTING LIGHT POLE FOUNDATION ADJUSTMENT required.

X8440116 RELOCATE EXISTING LIGHTING UNIT, SPECIAL

Description: This work shall consist of relocating subdivision entrance light poles or street lights in conflict with the proposed improvements on a new foundation in a nearby location as indicated on the plans or as directed by the Engineer such that it does not conflict with existing underground or aboveground utilities or features.

New light pole foundation shall be identical to the existing foundation as observed in the field.

Work shall also include disconnecting existing light pole from electrical service and reconnection of light pole on aforementioned new foundation. Any additional wiring, trenching or splicing required to reconnect the light pole service in its new location shall be considered included in this pay item.

Work shall include removal of existing light pole, storing and protecting the removed light pole from damage or vandalism while not in service, and attaching light pole on new foundation. Existing foundation removal and backfilling with suitable material as designated by the Engineer shall also be considered included.

Basis of Measurement & Payment: Work shall be paid for and measured by EACH for RELOCATE EXISTING LIGHTING UNIT, SPECIAL required.

X8710031 FIBER OPTIC CABLE – SINGLE MODE

This work shall be in accordance with Sections 871 of the Standard Specifications except as modified herein and shall include furnishing and installing fiber optic cable in conduit and within innerduct as noted on the plans and as herein described.

Add the following to Article 871.01 of the Standard Specifications:
The Fiber Optic cable shall be installed in conduit or as specified on the plans.

The fibers shall have a 9 micron core and a 125 micron cladding, thus having a type of 9µm/125µm optical fiber.

Add the following to Article 871.04 of the Standard Specifications:
Single-mode fibers shall be terminated with approved optical connectors. Twelve (12) fibers shall be terminated with ST mechanical connectors and fusion splices shall be used for the remaining 24 fibers at each termination point.

A minimum of 13.0 feet (4m) of extra cable length shall be provided for controller cabinets. The controller cabinet extra cable length shall be coiled and stored as approved by the Engineer.

Include in paragraph (b) of Article 1076.02:
Single mode fiber shall satisfy the criteria of ITU Recommendation. G.652.

FIBER OPTIC TERMINATIONS

The Contractor will splice and terminate optical fibers from different cable sheaths at the locations shown on the Plans. Fibers assigned to a cabinet or location will be terminated on ST-connectors in a termination housing or termination panel; fibers not assigned to the location shall be spliced “through” to the next cabinet/location.

Two splices are identified based on the number of potential terminations: 12 terminations and 48 terminations.

Materials: Three types of terminations will be provided as summarized in the following table.

Panel Type	Connector Type	Fiber Count	Connector Count	Splices
6-Fiber	ST	24	Up to 12	Up to 30
36-Fiber	ST	36	Up to 48	Up to 36
48-Fiber	ST	48	Up to 48	Up to 48

Fiber optic terminations will consist of three components: the termination panel and housing, a fiber optic pigtail with one fiber for each connector, fusion splices, and a splice closure. Fiber optic interconnect cables will be provided to connect the termination panels to the network equipment or to crosspatch fibers from different cable sheaths.

6-fiber Termination

The 6-fiber termination is typically used to connect a field cabinet to the backbone cable. It consists of a pre-terminated ITS drop cable equipped with a 6-fiber pigtail and cable splice.

36-fiber Termination

The 36-fiber termination is typically used to terminate all fibers in a 36-fiber cable at an end point or network node. It consists of a termination box with bulkhead adapters/connectors, a pre-connectorized pigtail, and cable splice.

ITS Drop Cable

The ITS Drop Cable is a cable assembly consisting of a hermetically-sealed Fiber Termination Box equipped with six ST-type, female optical connectors. These connectors terminate a 6-fiber pigtail, with the same optical and physical characteristics as the cable it is terminating. These drop cables should be functionally equivalent to a GATOR patch™.

Fiber Termination Box

The Fiber Termination Box shall either rack-mounted or wall-mounted. It will consist of two chambers, one normally used for splicing pigtails to the entrance cable and one used for patch cords used for connecting equipment and cross-patching fibers. The splicing chamber is not required when pre-connectorized, pigtailed entrance cables are used.

Pre-connectorized Pigtail

The pre-connectorized cable connects the adapters in the termination panel to the splice in the cable vault/double handhole. ST-connectors are factory-installed on one end of a cable pigtail. The other end of the cable is spliced to appropriate fibers in the mainline cable. The cable shall be optically and mechanically equivalent to the fiber optic mainline cable specified for this project. These cables shall contain either 36 fibers for the 36-fiber termination or 48 fibers for the 48-fiber termination. The pigtails shall be factory-tested and shall have loss not exceeding 0.5 dB per connector.

Fusion Splice

The Contractor shall splice the fibers in the pigtail cable to the mainline cable as indicated in the plans. Additional protection shall be installed on the spliced fibers. The maximum splice loss for the fusion splices shall not exceed 0.1 dB. This splice loss will be measured as part of the fiber optic testing required under the fiber optic cable installation.

Splice Closure

The splice closure shall be designed for underground applications. It shall be waterproof and re-enterable using common hand tools. It will provide a chamber tray to house the fiber optic splices. It shall also provide storage space for buffer tubes in the mainline cable that are not accessed at the specific location. The nominal dimensions of the splice closure shall be 6.5" diameter and 17" length.

All tapes and hardware required for the proper installation of the splice closure shall be incidental to this pay item.

All mounting hardware and labeling materials are included. Also included are jumper cables with ST connectors on one end and SC (or LC) connectors on the other to match the connectors on the equipment. These jumpers connect the terminated fibers to the ports on the Ethernet switches or other field devices. Each 6-fiber panel shall include five (5) jumpers and each 48-fiber panel shall include ten (10) jumpers. Each jumper will be 72 inches long. Jumpers not used for this project will be stored in plastic pouches as maintenance spares and placed in the controller cabinets. If pigtails are used to attach connectors to the mainline cables, excess pigtails shall be similarly stored in plastic bags and placed in the controller cabinet.

The cables shall be terminated according to the manufacturer's recommended guidelines. The Contractor shall prepare the cables and fibers in accordance with the termination panel and cable manufacturers' installation practices. A copy of these practices shall be provided to the Engineer 21 days prior to splicing operations.

Using a fusion splicer, the Contractor shall optimize the alignment of the fibers and fuse them together. The Contractor shall recoat the fused fibers and install mechanical protection over them.

Upon completing all splicing operations for a cable span, the Contractor shall measure the mean bi-directional loss at each splice using an Optical Time Domain Reflectometer. This loss shall not exceed 0.1 dB.

The Contractor shall measure the end-to-end attenuation of each fiber, from connector to connector, using an optical power meter and source. This loss shall be measured at from both directions and shall not exceed 0.5 dB per installed kilometer of single mode cable. For cables less than 1.6 km (1 mile), the measured loss should not exceed 2 dB. Measurements shall be made at both 1300 and 1550 nm for single mode cable.

The splice closure shall be installed using the manufacturer's instructions. It shall be flash tested to 6 psi minimum. The closure should be secured to the wall of the splice vault.

As directed by the Engineer, the Contractor at no additional cost to the Department shall replace any cable splice not satisfying the required objectives.

Measurement and Payment. This work will be paid for at the contract unit price per foot for FIBER OPTIC CABLE, SINGLE MODE of the number of fibers specified, which shall include all work to furnish and install the fiber optic cable as herein specified. The cost for providing Fiber Optic terminations as described are included in this pay item.

XZ127900 RETAINING WALL REMOVAL

Description: This work shall consist of existing retaining wall removal according to the applicable portions of Section 501 of the Standard Specifications. The existing wall is a metal sheet pile wall, likely driven in place at the time of installation.

Basis of Measurement & Payment: The retaining wall removal will be measured for payment per FOOT along the top of the existing wall. This work shall be paid for at the contract unit price per FOOT for RETAINING WALL REMOVAL.

XX006722 TEMPORARY AGGREGATE BERM – COARSE AGGREGATE & XX006723 TEMPORARY AGGREGATE BERM - RIPRAP

Description: This work shall consist of installing aggregate berms in locations as identified as "Rock Check Dams" on the plans for purposes of energy dissipation and water quality in proposed ditches. Both items are utilized together, along with filter fabric, in a single rock check dam. Installation shall follow IUM/NRCS Detail IL-605R as included in the plans.

Coarse Aggregate shall conform to CA-3 gradation. Riprap shall meet IDOT gradation RR-3 and meet Quality Designation A. Coarse aggregate and riprap shall be placed according to Article 281.04.

Filter fabric shall meet requirements of Article 1080.03 and shall not be paid for separately but considered included in the cost of TEMPORARY AGGREGATE BERM – COARSE AGGREGATE & TEMPORARY AGGREGATE BERM - RIPRAP.

Basis of Measurement & Payment: TEMPORARY AGGREGATE BERM – COARSE AGGREGATE & TEMPORARY AGGREGATE BERM – RIPRAP shall be measured for payment in place per TON of each placed.

XX006821 CONCRETE TRUCK WASHOUT

Description: The CONCRETE TRUCK WASHOUT as identified on the plans is used to contain concrete liquids when the chutes of concrete trucks are rinsed out after the delivery of concrete to the construction site. These washout facilities function to consolidate soils for disposal and prevent runoff liquids associated with concrete. Details of the construction of the non-portable facilities are included within the plans as “temporary concrete washout facilities.” Failure to comply with appropriate washout location requirements will result in monetary deficiency deduction against the Contractor.

General Requirements:

- The Contractor must submit a plan of his/her proposed temporary concrete washout facility to the Engineer for his/her approval at least 10 days prior to the first concrete pour.
- Temporary concrete washout facilities are to be in place prior to any delivery of concrete to the construction site.
- Temporary concrete washout facilities are to be located at least 50 feet from storm drain inlets, open drainage facilities, or water bodies. Each facility is to be located away from construction traffic or access areas to prevent disturbance or tracking.
- A sign is to be installed adjacent to each temporary concrete washout facility to inform concrete equipment operations of the designated washout facility.

Design:

Two types of concrete washout facilities are available for use on this project:

Prefabricated portable facilities (as approved by the Engineer)

Non-portable facilities:

- **Above Grade:** Constructed using barrier wall & polyethylene sheeting. Barrier walls are constructed to create a berm with a single sheet of 10-mil polyethylene sheeting which is free of holes, tears, or other defects which may compromise the impermeability of the material. Sandbags are used to hold the sheeting in place on top of the berm. Sheeting must extend over the entire basin and berm to prevent escape of discharge.
- **Below Grade:** Constructed via excavation and the use of polyethylene sheeting and sandbags. A pit is first excavated in a designated location with a single sheet of 10-mil polyethylene sheeting which is free of holes, tears, or other defects, which may compromise the impermeability of the material. Sandbags are then used to hold the sheeting in place.

Size of Washouts:

Number and size of washout facility is to be determined by the Contractor. It is his/her responsibility to provide enough storage for the excess concrete and water produced on the target. Non-portable facilities are to have a minimum length and width of 10'.

Inspection/Maintenance/Removal:

- Temporary concrete washout facilities are to be inspected by the Engineer during his/her weekly erosion and sediment control inspection per the requirements of the SWPPP. The inspector is to ensure there are no leaks, spills, and the capacity of the facility has not yet been compromised.
- Any overflowing of the washout facility onto the ground must be cleaned up and removed within 24 hours of discovery.
- If a rain or snow event is forecasted, a non-collapsing, non-water collecting cover shall be placed over the washout facility and secured to prevent accumulation and overflow of precipitation.
- Contents of each facility are not to exceed 75% of design capacity. If contents reach 75% capacity, discontinue pouring concrete into the facility until it has been cleaned out.
- Allow slurry to evaporate or remove the site in a safe manner (i.e. vacuum truck). All hardened material can then be removed or disposed of properly.
- If a lined basin is used, immediately replace the liner if it becomes damaged.
- Remove temporary concrete washout facilities when they are no longer required and restore the disturbed areas to their original condition.
- Note locations of these facilities and any changes to these locations on the SWPPP.

Basis of Payment: This work shall be paid for at the contract unit price LUMP SUM for CONCRETE TRUCK WASHOUT, which price shall be payment in full for all material, labor, excavation, and maintenance of all temporary concrete washout facilities on the project throughout the life of the project.

XX007251 INTERSECTION VIDEO TRAFFIC MONITORING SYSTEM WITH PTZ CAMERA

Description: The Contractor shall furnish and install a video surveillance camera system consisting of a special video camera in a dome, a dome mount to the video monitoring pole, all mounting hardware, brackets, outdoor rated network cable (to be paid for separately) supplied to the required length by the video system manufacturer with fast disconnect at the camera mount, video camera controller and special electronics/cabling for video transmission and pan/tilt/zoom controls, video controller unit to link all electronic components between the controller unit and the camera dome to include heater, fan, PTZ camera, video coax, video decoders with video encoding and decoding software.

Materials. The camera shall be designed and optimized for roadway video monitoring. The items shall have a minimum Object Distance: 300mm (wide end), 800mm (tele end) and have a minimum mechanical zoom of thirty (30x) plus a minimum digital zoom of twelve (12x). The camera, joystick controller (required for field adjustments and video verification), camera controller and auxiliary devices necessary for a complete and functional video operation shall utilize the Diamond control protocol for pan/tilt/zoom controls. The camera shall be digital with IP port(s) and a built-in encoder for connection to the central office. A separate encoder shall not be required. The camera shall provide for 360-degree rotation on the horizontal plane and 180-degree rotation within the lower hemisphere of the dome. Video resolution of video feed shall have a minimum image quality of HDTV 720p.

The Contractor shall install an auxiliary cabinet, DT-ST Series, when the distance between the camera and traffic controller cabinet exceeds 300 feet. The use of a DT-ST cabinet shall be considered incidental to the

cost of the video traffic monitoring system and no additional compensation shall be provided for the cabinet, cables, additional fiber optic cable, jumpers, etc.

The Contractor shall furnish and install the video software for decoding and encoding.

This item includes furnishing and installing the video monitoring camera, power injector (if required), and an auxiliary DT-ST cabinet as shown on the intersection wiring diagrams, box prints and fiber optic wiring diagram. This item also includes furnishing, installing and testing all auxiliary cabling, connectors, couplers, in-building hardware and software, jacks, splitters, conversion adapters, equipment racks, power supplies, power strips, surge suppressors, etc., necessary for a complete and fully functional system. The cable to be used for connecting the video monitoring camera to the local Ethernet switch shall be paid for separately under the pay item "Outdoor Rated Network Cable."

Cameras shall be mounted onto existing and proposed infrastructure. Proposed poles will be paid separately.

All mounting platforms, connecting hardware and auxiliary devices to test and operate this system to the satisfaction of the Engineer shall be incidental to this pay item and no additional compensation will be allowed.

The contractor shall contact the Kane County Department of Transportation (KDOT), Traffic Division prior to installing the PTZ camera and associated wiring, to receive final approval on the camera location.

Basis of Payment: This item will be paid for at the contract unit price each for INTERSECTION VIDEO TRAFFIC MONITORING SYSTEM WITH PTZ CAMERA, which price shall be payment in full for furnishing all associated equipment required, installing the system complete and in place, and placing the system in operation to the satisfaction of KDOT Traffic.

XX007953 NETWORK CONFIGURATION

Description: This work shall consist of installing, configuring and provisioning a fully operational Ethernet Local Area Network (LAN), which provides communication with remote traffic control field devices from the Kane County Division of Transportation (KDOT) traffic office. Field devices include traffic signal controllers, loop detectors, Malfunction Management Units (MMU), Uninterruptable Power Supply (UPS) units, video detection systems and CCTV (PTZ) cameras or other specified Intelligent Transportation System (ITS) field device as shown on the plans. The system shall have the capacity to add additional devices in the future.

Construction: Contractor shall include configuring Ethernet switches, terminal servers and media converters, assigning IP addresses to field devices based on KDOT Traffic staff input/standards, troubleshooting and submitting documentation to KDOT Traffic staff. A new, contractor-provided 36-strand single mode KDOT fiber optic cable shall serve as the Local Area Network (LAN) communication backhaul for this project to the traffic cabinet located at Randal Road and through the existing KDOT network to the KDOT Traffic operation Center. This work shall also require coordination with each manufacturer of field end devices, converters, and networking equipment to ensure successful digital video transmissions, serial-over-copper, serial-over-fiber, and serial-over-Ethernet communications between the WAN and field devices. The contractor shall also coordinate final connection to the existing system network with the KDOT network contractor of record. The contractor shall setup a meeting between the contractor, KDOT IT staff,

KDOT stakeholders, and the Engineer to coordinate programming requirements for the final network programming prior to final turnover.

The Contractor shall develop a written test plan and submit it to the Engineer and KDOT Traffic for approval. The test plan shall be revised to the satisfaction of the Engineer and KDOT Traffic for approval. The testing plan shall include systematic procedures with anticipated results that demonstrate that the communication network and all of its subsystems are fully operational. Approved testing procedures will be performed in the presence of KDOT and Contractor representatives. The testing plan shall include forms listing itemized functional checks of the system with signature placeholders for KDOT and Contractor representatives.

The test plan will verify the network performance over the extent of this project. The Contractor shall emulate traffic operations over the network by interfacing a laptop computer with the Type 2 Ethernet Switch at Randall Road and Longmeadow Parkway. From this location, the Contractor will control and exchange data with all ITS and traffic controllers, CCTV cameras, DMS signs and the RWIS. The computer shall also monitor the UPS components and all other alarms.

After satisfactory completion of this work, the existing master controllers shall be returned to KDOT as directed by the Engineer.

Basis of Payment: The work shall be paid for at the contract unit price per lump sum for NETWORK CONFIGURATION, which price shall be payment in full for all communication network configuration and coordination necessary to deliver an Ethernet network that provides successful communications between all field devices and the communication backhaul to the KDOT Traffic Office.

XX008392 OUTDOOR RATED NETWORK CABLE

Description: This work shall consist of furnishing an outdoor-rated 24 AWG, 4-pair data cable. Each cable link that is routed to an external device outside of the area serving ITS cabinet shall be protected by a lightning protection device on the switch side of the link cable for equipment protection. Contractor shall also provide an outdoor rated Ethernet extender to connect to Radar speed signs and power and connect to PTZ CCTV cameras located throughout the project.

Materials:

Shielded polyolefin cable with four 24 AWG twisted pair conductors.

Jacket Material: PE

Conductor Material: Bare Copper

Drain Wire Material: Tinned Copper

Insulation Material: Polyolefin

Separator Material: Polyolefin

Shield (Tape) Material: Aluminum/Poly

Cable shall meet the following electrical criteria:

ANSI/TIA Category: 6

Maximum dc Resistance Unbalance: 5 percent

Maximum dc Resistance: 9.38 ohms/100 m

Mutual Capacitance: 6.0 nF/100 m @ 1 kHz

Nominal Velocity of Propagation (NVP): 62 percent

Maximum Operating Frequency: 250 MHz

Transmission Standards: ANSI/TIA-568-C.2, CENELEC EN 50288-6-1, ISO/IEC 11801 Class E

Cable shall have an operating temperature from -40 degrees Celsius to 70 degrees Celsius, with an insulation temperature from 0 degrees Celsius to 60 degrees Celsius.

Cable shall be type F/UTP (unshielded) with 4 pairs.

Conductor gauge shall be 24 AWG and of solid type. 8 conductors shall be provided.

Maximum pull tension of cable shall not be less than 25 pounds (11 kg).

Nominal cable diameter over jacket shall be no greater than 8.255 millimeters.

The cable shall satisfy the requirements of OUTDOOR RATED NETWORK CABLE.

RJ-45 grounded lightning protection device shall be a DITEK DTK-MRJPOE or approved equal

RJ-45 External Ethernet and POE extender with 60W pass thru shall be a VERACITY OUTREACH MAX XT or approved equal.

Basis of Payment: This work will be paid for at the contract unit price per FOOT for OUTDOOR RATED NETWORK CABLE which price shall include all equipment, labor, and materials necessary to complete this work as specified including mounting hardware and terminating connectors.

XX08453 ETHERNET SWITCH, TYPE 1

Description: This work shall include all materials and work necessary to install an Ethernet Switch, Type 1 in a traffic signal cabinet. The Ethernet Managed Switch, Type 1 will connect the equipment in the field cabinet to the Kane County ITS data-comm fiber optic network.

Materials: The Ethernet Managed Switch, Type 1 is a managed edge switch configured with a minimum of the following ports:

- 8 RJ-45 10/100 Communication ports; a minimum of four (4) ports shall be equipped to provide power over Ethernet (PoE)
- 2 Single-mode 100 base Fiber optic communication ports
- 2 Single-Mode 1000 base fiber optic communication ports

The Ethernet Managed Switch, Type 1 shall satisfy the following:

- Dimensions: 6.85" H x 7.5" W x 2.0" D
- Power: 88-150 VAC, 47-63 Hz
- Power Consumption: 20 W (maximum)
- Temperature Range -40 to +140 degrees F; cooling shall use convection and heat sinking; no fans
- Weight: 3 lbs (nominal)

Performance:

- Filtering / Forwarding Rate: Ethernet (10Mb): 14,880 pps
- Fast Ethernet (100Mb): 148,800 pps

Gigabit Ethernet (1000Mb):	1,488,000 pps
Switching Processing:	Store and Forward with IEEE 802.3x full duplex flow control, non-blocking
Data Rate:	10Mbps, 100Mbps and 1000Mbps
Address Table Capacity:	4K node, self-learning with address aging
Packet buffer size:	240KB for 10/100 and 120KB for 1000Mb
Latency:	5 μ s + packet time (100 to 100Mbps) 15 μ s + packet time (10 to 10 Mbps, and 10 to 100Mbps)
Throughput with	max. - 4.17M pps (Transmit) (8 10/100 links and 2 Glinks)
Back plane-	2.66Gb/s per slot

Network Standards and Compliance, hardware:

Ethernet V1.0/V2.0 IEEE 802.3: 10BASE-T,
 IEEE 802.3u: 100Base-TX, 100BASE-FX
 IEEE 802.3z: 1000BASE-X Ethernet (Auto-negotiation)
 IEEE 802.3ab: 1000BASE-X Ethernet
 IEEE 802.1p: Priority protocol
 IEEE 802.1d: Spanning tree protocol
 IEEE 802.1w: Rapid Spanning tree protocol
 IEEE 802.1q: VLAN Tagging
 IEEE 802.3x: Flow Control
 IEEE 802.3ad: Link Aggregation (Trunking)
 IEEE 802.1x: Port based Network access control
 IEEE 802.3af: Power over Ethernet

Compatibility: The switch must be form, fit, and function interchangeable with the legacy Garrettcom 6KQE Ethernet switch. If requested by the Engineer, the Contractor shall provide an off-the-shelf factory model of the proposed switch and demonstrate that the proposed switch will operate transparently and with full functionality in the existing ITS data-comm network. The demonstration will take place prior to ordering any data-comm equipment.

Construction: The Contractor shall locate shelf space or other suitable mounting location in the traffic signal cabinets or as identified on the plans. The Contractor shall secure the Ethernet Switch as appropriate and approved by the engineer.

The Contractor shall install all necessary patch cords, optical transceivers, connectors, power supplies, communication transformers, or auxiliary equipment necessary to complete the communication circuits at full functional potential. The Contractor shall connect the switch to the field devices as indicated on the plans.

When requested by the Contractor, the Engineer will provide the necessary IP address assignments and port assignments, including the necessary port provisioning. The contractor shall be responsible for all network programming of the network switches and communicating elements within the traffic signal cabinet.

The Contractor will demonstrate that the switches are correctly installed and configured as specified in other special provisions for this project.

Basis of Payment: This work shall be paid for at the contract unit price each for ETHERNET SWITCH, TYPE 1, which price shall be payment in full for furnishing and installing an Ethernet Switch as specified.

XX08454 ETHERNET SWITCH, TYPE 2

Description. This work shall include all materials and work necessary to install an Ethernet Switch, Type 2 in a traffic signal cabinet. The Ethernet Switch, Type 2 connects field elements to the Kane County ITS data-comm network; in addition, it acts as an aggregation node and Gigabit Ethernet router.

Materials. The Ethernet Switch, Type 2 is a managed edge switch configured with a minimum of the following ports:

- 12 RJ-45 10/100 Communication ports; a minimum of four (4) ports shall be equipped to provide power over Ethernet (PoE)
- 2 Single-mode 100 base Fiber optic communication ports
- 4 Single-Mode 1000 base fiber optic communication ports

The Ethernet Managed Switch, Type 2 shall satisfy the following:

Dimensions:	6.85" H x 7.5" W x 2.0" D
Power:	88-150 VAC, 47-63 Hz
Power Consumption:	20 W (maximum)
Temperature Range	-40 to +140 degrees F; cooling shall use convection and heat sinking; no fans
Weight:	3 lbs (nominal)
Performance	
Filtering / Forwarding Rate:	Ethernet (10Mb): 14,880 pps
Fast Ethernet (100Mb):	148,800 pps
Gigabit Ethernet (1000Mb):	1,488,000 pps
Switching Processing:	Store and Forward with IEEE 802.3x full-duple flow -control, non-blocking
Data Rate:	10Mbps, 100Mbps and 1000Mbps Address
Table Capacity:	4K node, self-learning with address aging
Packet buffer size :	240KB for 10/100 and 120KB for 1000Mb
Latency:	6 μ s + packet time (100 to 100Mbps)
Throughput with	max.- 8.33M pps (Transmit) (8 10/100linls and 4 Glinks)
Back plane-	2.66Gb/s per slot

Network Standards and Compliance, hardware

Ethernet V1.0/V2.0 IEEE 802.3: 10BASE-T,
 IEEE 802.3u: 100Base-TX, 100BASE-FX
 IEEE 802.3z: 1000BASE-X Ethernet (Auto-negotiation)
 IEEE 802.3ab: 1000BASE-X Ethernet
 IEEE 802.1p: Priority protocol

IEEE 802.1d: Spanning tree protocol
IEEE 802.1w: Rapid Spanning tree protocol
IEEE 802.1q: VLAN Tagging
IEEE 802.3x: Flow Control
IEEE 802.3ad: Link Aggregation (Trunking)
IEEE 802.1x: Port based Network access control
IEEE 802.3af: Power over Ethernet

Compatibility. The switch must be form, fit, and function interchangeable with the legacy Garrettcom 6K32 Ethernet switch. If requested by the Engineer, the Contractor shall provide an off-the-shelf factory model and demonstrate that the proposed switch will operate transparently and with full functionality in the existing ITS data-comm network. The demonstration will take place prior to ordering any data-comm equipment.

Construction. The Contractor shall mount the Ethernet switch on a standard DIN rail or mounting channel in the traffic signal cabinet or as identified on the plans. The Contractor shall secure the Ethernet Switch as appropriate and approved by the engineer. The power supply shall be hard-wired to the cabinet power.

The Contractor shall install all necessary patch cords, optical transceivers, connectors, power supplies, communication transformers, or auxiliary equipment necessary to complete the communication circuits at full functional potential. The Contractor shall connect the switch to the field devices as indicated on the plans.

When requested by the Contractor, the Engineer will provide the necessary IP address assignments and port assignments, including the necessary port provisioning. The contractor shall be responsible for all network programming of the network switches and communicating elements within the traffic signal cabinet.

The Contractor will demonstrate that the switches are correctly installed and configured as specified in other special provisions for this project.

Basis of Payment. This work shall be paid for at the contract unit price each for ETHERNET SWITCH, TYPE 2, which price shall be payment in full for furnishing and installing an Ethernet Switch as specified.

Z0013302 SEGMENTAL CONCRETE BLOCK WALL

This work shall consist of excavation for a segmental block wall in accordance with the applicable portions of Section 502 of the Standard Specifications and furnishing all materials and constructing a segmental block retaining wall at the location shown on the plans in accordance with the applicable portions of IDOT's Guide Bridge Special Provisions for Segmental Concrete Block Wall and as described herein. This work shall include all excavation and disposal of excavated material, construction of a leveling pad and granular backfill behind the wall in accordance with the manufacturer's recommendations. This item shall also include any tiebacks or geotechnical fabric if recommended by the manufacturer for the specific location.

The Contractor shall prepare shop drawings for review by the Engineer and make recommendations as to the wall color, exposed block texture and the needed of any drainage. The Engineer shall decide on the finish and color of the materials and may consult with local jurisdictions or maintenance personnel in reviewing the

shop drawings and preparing recommendations. Connections between existing wall to remain and the new wall shall be in accordance with wall manufacturer's standards and recommendations.

Measurement and Payment: This work will be measured and paid for at the contract unit price per square foot based upon the lineal foot of wall installed and measured horizontally along the base of the wall and the height of the wall. The height of the wall, for purposes of measurement, will include the height of all block courses but not the leveling pad. This work will be paid for at the contract unit price per square foot for SEGMENTAL CONCRETE BLOCK WALL, which price shall be payment in full for all labor, materials and equipment necessary furnish, install the wall as shown on the plans and as described herein.

Z0013796 SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE

Description: This work shall consist of constructing a stabilized construction entrance, including furnishing, installing, maintaining and removing a stabilized pad of aggregate underlain with filter fabric, as shown on the plans or directed by the Engineer.

Materials: The materials used shall meet the requirements of the following:

Aggregate: The aggregate shall be limited to IDOT Coarse Aggregate Gradation CA-1.

Filter Fabric: The filter fabric shall be made of synthetic polymers composed of at least 85 percent by weight polypropylene, polyesters, polyamides, polyethylene, polyolefins, or polyvinylidene-chlorides. The geotextile shall be free of any chemical treatment or coating that significantly reduces its porosity. Fibers shall contain stabilizers and/or inhibitors to enhance resistance to ultraviolet lights.

Construction Requirements: The aggregate shall be at least six inches thick. The aggregate shall not be placed until the entrance area has been inspected and approved by the Engineer.

The aggregate shall be dumped and spread into place in approximately horizontal layers. The layer(s) shall not exceed three feet in thickness. The aggregate shall be placed in such a manner as to produce a reasonably homogeneous stable fill that contains no segregated pockets of larger or smaller fragments or large unfilled space caused by bridging of larger fragments. No compaction shall be required beyond that resulting from the placing and spreading operations.

The construction entrance shall follow the dimensions shown on the plans and/or have a minimum width of 14 feet and a minimum length of 50 feet.

All surface water flowing or diverted toward the construction entrance shall be piped across the entrance. Any pipe used for this will be considered included in the unit price for SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE. The stabilized construction entrance shall have positive drainage away from the roadway.

The entrance shall remain in place and be maintained until the disturbed area is stabilized. Any sediment spilled onto public right-of-way(s) shall be removed immediately. All removed materials shall be disposed of outside the limits of the right-of-way according to Article 202.03 of the "Standard Specifications" and/or as directed by the Engineer.

Method of Measurement: The Stabilized Construction Entrance will be measured in place and the area computed in square yards.

Basis of Payment: The work will be paid for at the contract unit price per square yard for SEDIMENT CONTROL, STABILIZED CONSTRUCTION ENTRANCE. The unit price shall include all material, including filter fabric, labor, equipment and any other items required to complete the construction entrance.

Z0013798 CONSTRUCTION LAYOUT

Description. The Contractor shall furnish and place construction layout stakes for this project. The Engineer will provide adequate reference points to the centerline of survey including all PI's, PCs, PT's, PRCs, and POT's as indicated on the plans as well as bench marks as described in the plans. Any additional control points set by the Department will be identified in the field to the Contractor and all field notes will be kept in the office of the Engineer.

The Contractor shall provide field forces, equipment, and material to set all additional stakes for this project, which are needed to establish offset stakes, reference points, and any other horizontal or vertical controls, including supplementary bench marks, necessary to secure a correct layout for the roadway portion of the work. Stakes for line and grade of pavement and/or curb shall be set at sufficient station intervals (not to exceed 50 ft (15 m)) to assure substantial conformance to plan line and grade. The Contractor will not be required to set additional stakes to locate a utility line which is not included as a pay item in the contract nor to determine property lines between private properties.

The Contractor shall be responsible for having the finished work conform to the lines, grades, elevations, and dimensions called for in the plans. Any inspection or checking of the Contractor's layout by the Engineer and the acceptance of all or any part of it shall not relieve the Contractor of his/her responsibility to secure the proper dimensions, grades, and elevations of the several parts of the work. The Contractor shall exercise care in the preservation of stakes and bench marks and shall have them reset when any are damaged, lost, displaced, removed, or otherwise obliterated.

Responsibility of the Contractor.

The Contractor shall establish from the given survey points and bench marks all the control points necessary to construct the individual project elements. (S)He shall provide the Engineer adequate control in close proximity to each individual element to allow adequate checking of construction operations. This includes, but is not limited to, line and grade stakes, line and grade nails in form work, and/or filed or etched marks in substantially completed construction work. It is the Contractor's responsibility to tie in centerline control points in order to preserve them during construction operations.

At the completion of the grading operations, the Contractor shall set stakes at 100 ft (25 m) station intervals along each profile grade line. These stakes will be used for final cross sectioning by the Engineer.

The Contractor shall locate the right-of-way points for the installation of right-of-way markers. The Contractor shall set all line stakes for the construction of fences by the Contractor.

All work shall be according to normally accepted self-checking surveying practices. Field notes shall be kept in standard survey field notebooks and those books shall become the property of the Department at the completion of the project. All notes shall be neat, orderly, and in accepted form.

Measurement and Payment. This work will be paid for at the contract LUMP SUM price for CONSTRUCTION LAYOUT.

Z0015200 CURB STOPS 1 ½”

Description. This work shall consist of all labor, equipment, and materials required to provide and install a new curb stop and domestic water service box of the specified size at locations shown on the plans or as directed by the Engineer.

Material Requirements. Curb stops shall be fabricated of brass and provided with an outlet suitable for copper connection in accordance with the requirements of AWWA Standard C-800. Curb stops shall be of the round-way type, quarter-turn check and Minneapolis pattern top thread. Curb boxes shall be Minneapolis pattern and 1 ½” diameter. Domestic water service boxes shall be cast iron, screw-type, with the base set over the curb stop and of such construction that it can be extended in length to fit the particular location with a minimum 18” of adjustment left. The cover of the domestic water service box shall be marked “WATER”.

Construction Requirements. Curb stops shall be installed in accordance with the applicable requirements of Section 41-2.12 of the Water and Sewer Specifications.

Basis of Payment. This work will be paid for at the contract unit price per each for CURB STOPS 1 ½”.

Z0019600 DUST CONTROL WATERING

Description: This work shall consist of furnishing and applying water to control dust and air-borne dirt generated by construction activities.

General: This work shall be performed according to Article 107.36 of the “Standard Specifications” and the following:

Revise Article 107.36 of the “Standard Specifications” as follows:

Replace sub-paragraph (d) of under the third paragraph with the following:

(d) Dust shall be controlled by the uniform application of sprinkled water and shall be applied only when directed and in a manner approved by the Engineer. All equipment used for this work shall meet with the Engineer’s approval and shall be equipped with adequate measuring devices for determining the exact amount of water discharged. All water used shall be properly documented by ticket or other approved means.

The Contractor is reminded of the provisions of Article 107.18 of the “Special Provisions” regarding the procurement of water from fire hydrants.

Method of Measurement: This work will be measured in units of gallons of water applied. One unit is equivalent to 1,000 gallons of water applied.

Basis of Payment: This work will be paid for at the contract unit price per unit for DUST CONTROL WATERING. *The unit price shall include all equipment, materials and labor required to control dust.*

Z0022800 FENCE REMOVAL

Description: This work shall consist of the removal and disposal of an existing fence from the project site.

General: The Contractor shall remove all components of the existing fence including any concrete used to anchor fence posts, bracing, guy wires, posts, and/or gates. All removed materials shall be disposed of outside the limits of the right-of-way according to Article 202.03 of the “Standard Specifications” and/or as directed by the Engineer.

Method of Measurement: This work will be measured for payment in feet, along the top of the existing fence, from center to center of end posts, excluding the length occupied by gates.

Basis of Payment: This work will be paid for at the contract unit price per foot for FENCE REMOVAL. The unit price shall include all equipment, materials and labor required to remove and dispose of the fence.

Z0023202 SEDIMENT CONTROL, DRAINAGE STRUCTURE INLET FILTER CLEANING

Description: This work shall consist of cleaning sediment out of a drainage structure inlet filter when directed by the Engineer. The Engineer will be the sole judge of the need for cleaning based on the rate that debris and silt is collected at each inlet filter cleaning.

Cleaning of the inlet filter shall consist of inspecting, cleaning (includes removal and proper disposal of debris and silt that has accumulated) by vactoring, removing and dumping, or any other method that has been approved by the engineer.

For purposes of this contract, it is anticipated that inlet filter cleaning will be performed one time for all inlet filters on the project. Some filters will require no cleaning, others will require multiple cleanings. The Contractor may use some or all quantity for this pay item.

Basis of Payment: This work shall be paid at the contract unit price for EACH for SEDIMENT CONTROL, DRAINAGE STRUCTURE INLET FILTER CLEANING.

Z0005608, Z0005610, Z0056612, Z0056624 STORM SEWER ADJACENT TO OR CROSSING WATER MAIN

Description: This work consists of constructing storm sewer adjacent to or crossing a water main, at the locations shown on the plans. The material and installation requirements shall be according to the latest edition of the “Standard Specifications for Water and Sewer Main Construction in Illinois”, and the applicable portions of Section 550 of the Standard Specifications; which may include concrete collars and encasing pipe with seals if required.

Pipe materials shall meet the requirements of Sections 40 and 41-2.01 of the “Standard Specifications for Water and Sewer Main Construction in Illinois”, except PVC pipe will not be allowed. Ductile-Iron pipe shall meet the minimum requirements for Thickness Class 50.

Encasing of standard type storm sewer, according to the details for “Water and Sewer Separation Requirements (Vertical Separation)” in the “STANDARD DRAWINGS” Division of the “Standard Specifications for Water and Sewer Main Construction in Illinois”, may be used for storm sewers crossing water mains.

Basis of Payment: This work will be paid according to Article 550.10 of the Standard Specifications, except the pay item shall be STORM SEWER, (TYPE SPECIFIED), WATER MAIN QUALITY PIPE of the diameter specified.

Z0062458 TEMPORARY PAVEMENT (VARIABLE DEPTH)

Description: This work shall consist of constructing temporary hot-mix asphalt sections at the locations shown on the plans or as directed by the Engineer. These paved sections will be necessary at various locations throughout the project, including intersections, during the course of construction to temporarily provide a surface along grade differentials due to stage construction (i.e. longitudinal sections of pavement to allow traffic to ride along or transition from new pavement constructed in Stage 1 to adjacent existing pavement remaining in place until a later stage).

The Contractor shall use HMA according to Section 406 of the Standard Specifications, and other applicable HMA special provisions as contained herein. The HMA mixtures to be used shall be specified in the plans. The thickness of the Temporary Pavement will be variable and is shown in the staging temporary pavement details. Article 406.11 of the Standard Specifications shall not apply.

The removal of the Temporary Pavement (Variable Depth), if required, shall conform to Section 440 of the Standard Specifications.

Basis of Payment: This work will be paid for at the contract unit price per ton for TEMPORARY PAVEMENT (VARIABLE DEPTH). Removal of Temporary Pavement (Variable Depth) will not be measured separately for payment, but shall be included within the contract unit price per ton for TEMPORARY PAVEMENT (VARIABLE DEPTH).

Z0066600 & Z0066700 STABILIZED DRIVEWAYS (8”,10”)

Description: This work shall consist of furnishing, placing and compacting hot-mix asphalt driveway pavement at locations shown on the plans and as directed by the Engineer.

This work shall conform to the applicable Sections of Articles 311, 355 and 406.

Indicated driveways to be stabilized shall be constructed to a nominal thickness of 8 inches for a private entrance and 10” for a commercial entrance. Each shall have a minimum 2” thick surface course (HMA Surface Course, Mix “D”, N70) with the balance constructed using hot mix asphalt base course (HMA Base Course, 6” or 8”). Aggregate and bituminous material prime coats shall be applied according to Article 406

and as directed by the Engineer. The driveway shall be constructed on a 4 inch compacted aggregate subbase conforming to the applicable Sections of Article 311 for Subbase Granular Materials Type B.

Method of Measurement: Stabilized driveways will be measured in place and the area computed in SQUARE YARDS. Aggregate subbase and aggregate and bituminous material prime coats will not be measured for payment but shall be considered included in payment for Stabilized Driveways of the thickness specified.

Basis of Payment: The work will be paid at the contract unit price per square yard for STABILIZED DRIVEWAYS, 8 INCH.

Z0075505 TIMBER RETAINING WALL REMOVAL

Description: This work shall consist of removing an existing timber retaining wall at the location shown on the plans in accordance with the applicable portions of Sections 202 and 501 of the Standard Specifications.

The existing timber retaining wall shall be removed and all materials disposed of off the right-of-way as part of this item. The wall appears to have timber tie backs, thus the tie backs shall be removed or cut-off at a sufficient distance to allow a replacement segmental block wall (replacement wall to be paid for separately) to be constructed, at locations as shown on the plans, without interfering with the remaining portion of any tie backs left in place. The existing timber retaining wall is exposed approximately 2' to 3' above the ground along its length.

The Contractor shall carefully cut and remove the portion of the existing wall identified for replacement and that portion of the wall that is to remain in place. A smooth joint shall be prepared so the new wall will fit tight to the portion of the existing wall that is to remain in place. If any portion of the wall that is to remain in place is damaged by the Contractor, it shall be repaired by the Contractor at no additional cost to the contract.

Measurement and Payment. This work will be measured and paid for at the contract unit price per lineal foot for TIMBER RETAINING WALL REMOVAL, which price shall be payment in full for all labor, materials and equipment necessary to remove and dispose of the timber retaining wall as shown on the plans and as described herein.

THREE CELL FABRIC INNERDUCT

Description: This work shall consist of providing and installing a detectable 3-cell fabric innerduct within existing and proposed conduits as shown on the plans.

Materials: Fabric innerduct shall contain three individual cells each capable of housing cables up to 1.3" diameter cables. Fabric innerduct shall be sized to be placed in a 4" or larger conduit. Fabric innerduct shall be constructed of a flexible nylon-6 resin polymer material meeting UL 2024A standards for Optical Fiber Communications raceways. Innerduct material shall be factory lubricated.

Pull Tape: Pull tape shall be constructed of synthetic fiber and shall be pre-installed within each innerduct cell. Pull tape shall have sequential footage marks every 5 feet. Pull tape must be color coated to differentiate between cells. Innerduct shall contain an integrated 14-gauge tracer wire for detecting conduit.

If the product is not available with a 14-gauge tracer wire then the Contractor shall install a separate 14-gauge copper wire alongside the installed innerduct.

Fabric Innerduct shall be installed in accordance with manufactures guidelines. At each end of a conduit run, the fabric innerduct shall be tied off in accordance with the manufacturer's recommendations and the opened conduit end sealed or plugged to prevent infiltration of insects, debris or water.

Basis of Payment: This work will be paid for at the contract unit price per foot for THREE-CELL FABRIC INNERDUCT which price shall include all material, equipment, labor, and tools necessary to complete this work as specified including mounting hardware and terminating the innerduct and plugging the end of the conduits.

IDOT SPECIAL PROVISIONS (Unmodified)

Bureau of Design

AGGREGATE SURFACE COURSE FOR TEMPORARY ACCESS

Effective: April 1, 2001

Revised: January 2, 2007

Revise Article 402.10 of the Standard Specifications to read:

“402.10 For Temporary Access. The contractor shall construct and maintain aggregate surface course for temporary access to private entrances, commercial entrances and roads according to Article 402.07 and as directed by the Engineer.

The aggregate surface course shall be constructed to the dimensions and grades specified below, except as modified by the plans or as directed by the Engineer.

- (a) Private Entrance. The minimum width shall be 12 ft (3.6 m). The minimum compacted thickness shall be 6 in. (150 mm). The maximum grade shall be eight percent, except as required to match the existing grade.
- (b) Commercial Entrance. The minimum width shall be 24 ft (7.2 m). The minimum compacted thickness shall be 9 in. (230 mm). The maximum grade shall be six percent, except as required to match the existing grade.
- (c) Road. The minimum width shall be 24 ft (7.2 m). The minimum compacted thickness shall be 9 in. (230 mm). The grade and elevation shall be the same as the removed pavement, except as required to meet the grade of any new pavement constructed.

Maintaining the temporary access shall include relocating and/or regrading the aggregate surface coarse for any operation that may disturb or remove the temporary access. The same type and gradation of material used to construct the temporary access shall be used to maintain it.

When use of the temporary access is discontinued, the aggregate shall be removed and utilized in the permanent construction or disposed of according to Article 202.03.”

Add the following to Article 402.12 of the Standard Specifications:

“Aggregate surface course for temporary access will be measured for payment as each for every private entrance, commercial entrance or road constructed for the purpose of temporary access. If a residential drive, commercial entrance, or road is to be constructed under multiple stages, the aggregate needed to construct the second or subsequent stages will not be measured for payment but shall be included in the cost per each of the type specified.”

Revise the second paragraph of Article 402.13 of the Standard Specifications to read:

“Aggregate surface course for temporary access will be paid for at the contract unit price per each for TEMPORARY ACCESS (PRIVATE ENTRANCE), TEMPORARY ACCESS (COMMERCIAL ENTRANCE) or TEMPORARY ACCESS (ROAD).

Partial payment of the each amount bid for temporary access, of the type specified, will be paid according to the following schedule:

- (a) Upon construction of the temporary access, sixty percent of the contract unit price per each, of the type constructed, will be paid.
- (b) Subject to the approval of the Engineer for the adequate maintenance and removal of the temporary access, the remaining forty percent of the pay item will be paid upon the permanent removal of the temporary access.”

TRAFFIC CONTROL AND PROTECTION (ARTERIALS)

Effective: February 1, 1996

Revised: March 1, 2011

Specific traffic control plan details and Special Provisions have been prepared for this contract. This work shall include all labor, materials, transportation, handling and incidental work necessary to furnish, install, maintain and remove all traffic control devices required as indicated in the plans and as approved by the Engineer.

When traffic is to be directed over a detour route, the Contractor shall furnish, erect, maintain and remove all applicable traffic control devices along the detour route according to the details shown in the plans.

Method of Measurement: All traffic control (except “Traffic Control and Protection (Expressways)” and temporary pavement markings) indicated on the traffic control plan details and specified in the Special Provisions will be measured for payment on a lump sum basis.

Basis of Payment: All traffic control and protection will be paid for at the contract lump sum price for TRAFFIC CONTROL AND PROTECTION (SPECIAL).

Temporary pavement markings will be paid for separately unless shown on a Standard.

TRAFFIC CONTROL AND PROTECTION (SPECIAL) (KDOT)

Revised on: 2/6/15

Special attention is called to Article 107.09 and Division 700 of the Standard Specifications and the following Highway Standards, Details, Quality Standard for Work Zone Traffic Control Devices, Supplemental Specifications and Recurring Special Provisions, and Special Provisions contained herein, relating to traffic control.

The Contractor shall contact the Engineer at least **72 hours** before beginning work.

The Contractor shall be required to use the latest version of the highway standards listed below as traffic conditions and working conditions warrant.

Highway Standards:

- 635011-02 Reflector Marker and Mounter Details
- 701001-02 Off-Road Operations, 2L, 2W, More than 15' Away
- 701106-02 Off Road Operations, Multilane, More than 15' Away
- 701201-04 Lane Closure, 2L, 2W, Day Only for Speeds \geq 45 MPH
- 701301-04 Lane Closure, 2L, 2W, Short Time Operations
- 701326-04 Lane Closure, 2L, 2W, Pavement Widening, For Speeds \geq 45 MPH
- 701336-06 Lane Closure, 2L, 2W, Work Areas in Series for Speeds \geq 45 MPH
- 701423-08 Lane Closure, Multilane, Intermittent or Moving Operations
- 701501-06 Urban Lane Closure, 2L, 2W, Undivided
- 701502-06 Urban Lane Closure, 2L, 2W, With BiDirectional Turn Lane
- 701701-09 Urban Lane Closure, Multilane Intersection
- 701801-05 Sidewalk, Corner, or Crosswalk Closure
- 701901-04 Traffic Control Devices
- 704001-07 Temporary Concrete Barrier

Special Provisions:

- Maintenance of Roadways
- Restriction on Working Days After A Completion Date
- Public Convenience and Safety (District 1) Traffic Control Plan
- Traffic Control and Protection (Arterials) Permanent Pavement Marking
- Aggregate Surface Course for Temporary Access
- Temporary Pavement Winterized Temporary Access
- Temporary Information Signing
- Wet Reflective Temporary Tape Type III Details

District One Standard Details:

- TC-10 Traffic Control & Protection for Side Roads, Intersections & Driveways
- TC-14 Traffic Control and Protection at Turn Bays (to remain open to traffic)
- TC-16 Pavement Marking Letters and Symbols for Traffic Staging
- TC-22 Arterial Road Information Sign
- TC-26 Driveway Entrance Signing

Recurring Special Provisions:

Night Time Inspection of Roadway Lighting
Pavement Marking Removal

Two weeks before construction begins, the Contractor shall install one sign in each direction for each roadway to be resurfaced. The signs shall be placed approximately 100 feet in

advance of the roadway resurfacing limits or as instructed by the Engineer. The signs shall not be removed until after the work has begun. These signs shall have minimum dimensions of 48 inches by 48 inches and have a black legend and border on an orange reflectorized background. The legend shall read:

“ROADWORK TO BEGIN” “WEEK OF XX/XX/15”

After the milling of the roadway has begun the Contractor shall install one sign in each direction for each roadway that has been milled. The signs shall be placed approximately 100 feet in advance of the roadway resurfacing limits or as directed by the Engineer. The signs shall not be removed until after the permanent striping has been placed. These signs shall have minimum dimensions 48 inches by 48 inches and have a black legend and border on an orange reflectorized background. The legend shall read:

“NO PASSING” “NOT STRIPED” “FOR XX MILES”

At the preconstruction meeting, the Contractor shall furnish the name of the individual in his direct employ who is to be responsible for the installation and maintenance of the traffic control for this project. If the actual installation and maintenance are to be accomplished by a subcontractor, consent shall be requested of the Engineer at the time of the preconstruction meeting in accordance with Article 108.01 of the Standard Specifications. This shall not relieve the Contractor of the requirement to have a responsible individual in his direct employ supervise this work. The Engineer will provide the Contractor the name of its representative who will be responsible for the administration of the Traffic Control Plan.

Method of Measurement: Traffic Control and Protection shall be measured for payment as Lump Sum, which shall include all labor, materials, and equipment necessary to complete the work described above.

Basis of Payment: Traffic Control and Protection shall be paid for at the contract lump sum price for TRAFFIC CONTROL AND PROTECTION (SPECIAL).

ADJUSTMENTS AND RECONSTRUCTIONS

Effective: March 15, 2011

Revise the first paragraph of Article 602.04 to read:

“602.04 Concrete. Cast-in-place concrete for structures shall be constructed of Class SI concrete according to the applicable portions of Section 503. Cast-in-place concrete for pavement patching around adjustments and reconstructions shall be constructed of Class PP-1 concrete, unless otherwise noted in the plans, according to the applicable portions of Section 1020.”

Revise the third, fourth and fifth sentences of the second paragraph of Article 602.11(c) to read:

“Castings shall be set to the finished pavement elevation so that no subsequent adjustment will be necessary, and the space around the casting shall be filled with Class PP-1 concrete, unless otherwise noted in the plans, to the elevation of the surface of the base course or binder course. HMA surface or binder course material shall not be allowed. The pavement may be opened to traffic according to Article 701.17(e)(3)b.”

Revise Article 603.05 to read:

“603.05 Replacement of Existing Flexible Pavement. After the castings have been adjusted, the surrounding space shall be filled with Class PP-1 concrete, unless otherwise noted in the plans, to the elevation of the surface of the base course or binder course. HMA surface or binder course material shall not be allowed. The pavement may be opened to traffic according to Article 701.17(e)(3)b.”

Revise Article 603.06 to read:

“603.06 Replacement of Existing Rigid Pavement. After the castings have been adjusted, the pavement and HMA that was removed, shall be replaced with Class PP-1 concrete, unless otherwise noted in the plans, not less than 9 in. (225 mm) thick. The pavement may be opened to traffic according to Article 701.17(e)(3)b.

The surface of the Class PP concrete shall be constructed flush with the adjacent surface.”

Revise the first sentence of Article 603.07 to read:

“603.07 Protection Under Traffic. After the casting has been adjusted and the Class PP concrete has been placed, the work shall be protected by a barricade and two lights according to Article 701.17(e)(3)b.”

AGGREGATE SUBGRADE IMPROVEMENT (D-1)

Effective: February 22, 2012

Revised: March 3, 2015

Add the following Section to the Standard Specifications:

“SECTION 303. AGGREGATE SUBGRADE IMPROVEMENT

303.01 Description. This work shall consist of constructing an aggregate subgrade improvement.

303.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Coarse Aggregate	1004
(b) Reclaimed Asphalt Pavement (RAP) (Notes 1, 2 and 3)	1031

Note 1. Crushed RAP, from either full depth or single lift removal, may be mechanically blended with aggregate gradation CS 01 but shall not exceed 40 percent by weight of the total product. The top size of the Coarse RAP shall be less than 4 in. (100 mm) and well graded.

Note 2. RAP having 100 percent passing the 1 1/2 in (37.5 mm) sieve and being well graded, may be used as capping aggregate in the top 3 in. (75 mm) when aggregate gradation CS 01 is used in lower lifts. When RAP is blended with any of the coarse aggregates, the blending shall be done with mechanically calibrated feeders. The final product shall not contain more than 40 percent by weight of RAP.

Note 3. The RAP used for aggregate subgrade improvement shall be according to the current Bureau of Materials and Physical Research Policy Memorandum, "Reclaimed Asphalt Pavement (RAP) for Aggregate Applications".

303.03 Equipment. The vibratory machine shall be according to Article 1101.01, or as approved by the Engineer. The calibration for the mechanical feeders shall have an accuracy of ± 2.0 percent of the actual quantity of material delivered.

303.04 Soil Preparation. The stability of the soil shall be according to the Department's Subgrade Stability Manual for the aggregate thickness specified.

303.05 Placing Aggregate. The maximum nominal lift thickness of aggregate gradation CS 01 shall be 24 in. (600 mm).

303.06 Capping Aggregate. The top surface of the aggregate subgrade shall consist of a minimum 3 in. (75 mm) of aggregate gradations CA 06 or CA 10. When Reclaimed Asphalt Pavement (RAP) is used, it shall be crushed and screened where 100 percent is passing the 1 1/2 in. (37.5 mm) sieve and being well graded. RAP that has been fractionated to size will not be permitted for use in capping. Capping aggregate will not be required when the aggregate subgrade improvement is used as a cubic yard pay item for undercut applications. When RAP is blended with any of the coarse aggregates, the blending shall be done with mechanically calibrated feeders.

303.07 Compaction. All aggregate lifts shall be compacted to the satisfaction of the Engineer. If the moisture content of the material is such that compaction cannot be obtained, sufficient water shall be added so that satisfactory compaction can be obtained.

303.08 Finishing and Maintenance of Aggregate Subgrade Improvement. The aggregate subgrade improvement shall be finished to the lines, grades, and cross sections shown on the plans, or as directed by the Engineer. The aggregate subgrade improvement shall be maintained in a smooth and compacted condition.

303.09 Method of Measurement. This work will be measured for payment according to Article 311.08.

303.10 Basis of Payment. This work will be paid for at the contract unit price per cubic yard (cubic meter) for AGGREGATE SUBGRADE IMPROVEMENT or at the contract unit price per square yard (square meter) for AGGREGATE SUBGRADE IMPROVEMENT, of the thickness specified.

Add the following to Section 1004 of the Standard Specifications:

"1004.06 Coarse Aggregate for Aggregate Subgrade Improvement. The aggregate shall be according to Article 1004.01 and the following.

(a) Description. The coarse aggregate shall be crushed gravel, crushed stone, or crushed concrete. The top 12 inches of the aggregate subgrade improvement shall be 3 inches of capping material and 9 inches of crushed gravel, crushed stone or crushed concrete. In applications where greater than 36 inches of subgrade material is required, rounded gravel, meeting the CS01 gradation, may be used beginning at a depth of 12 inches below the bottom of pavement.

(b) Quality. The coarse aggregate shall consist of sound durable particles reasonably free of deleterious materials. Non-mechanically blended RAP may be allowed up to a maximum of 5.0 percent.

(c) Gradation.

(1) The coarse aggregate gradation for total subgrade thicknesses of 12 in. (300 mm) or greater shall be CS 01.

	COARSE AGGREGATE SUBGRADE GRADATIONS Sieve Size and Percent Passing				
Grad No.	8"	6"	4"	2"	#4
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20

	COARSE AGGREGATE SUBGRADE GRADATIONS (Metric)				
Grad No.	Sieve Size and Percent Passing				
	200 mm	150 mm	100 mm	50 mm	4.75 mm
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20

(2) The 3 in. (75 mm) capping aggregate shall be gradation CA 6 or CA 10.

COARSE AGGREGATE FOR BACKFILL, TRENCH BACKFILL AND BEDDING (D-1)

Effective: November 1, 2011

Revised: November 1, 2013

This work shall be according to Section 1004.05 of the Standard Specifications except for the following:

Reclaimed Asphalt Pavement (RAP) maybe blended with gravel, crushed gravel, crushed stone crushed concrete, crushed slag, chats, crushed sand stone or wet bottom boiler slag. The RAP used shall be according to the current Bureau of Materials and Physical Research Policy Memorandum, "Reclaimed Asphalt Pavement (RAP) for Aggregate Applications". The RAP shall be uniformly graded and shall pass the 1.0 in. (25 mm) screen. When RAP is blended with any of the coarse aggregate listed above, the blending shall be done mechanically with calibrated feeders. The feeders shall have an accuracy of ± 2.0 percent of the actual quantity of material delivered. The final blended product shall not contain more than 40 percent by weight RAP.

The coarse aggregate listed above shall meet CA 6 and CA 10 gradations prior to being blended with the processed and uniformly graded RAP. Gradation deleterious count shall not exceed 10% of total RAP and 5% of other by total weight.

EMBANKMENT I

Effective: March 1, 2011

Revised: November 1, 2013

Description. This work shall be according to Section 205 of the Standard Specifications except for the following.

Material. All material shall be approved by the District Geotechnical Engineer. The proposed material must meet the following requirements.

- a) The laboratory Standard Dry Density shall be a minimum of 90 lb/cu ft (1450 kg/cu m) when determined according to AASHTO T 99 (Method C).
- b) The organic content shall be less than ten percent determined according to AASHTO T 194 (Wet Combustion).
- c) Soils which demonstrate the following properties shall be restricted to the interior of the embankment and shall be covered on both the sides and top of the embankment by a minimum of 3 ft (900 mm) of soil not considered detrimental in terms of erosion potential or excess volume change.
 - 1) A grain size distribution with less than 35 percent passing the number 75 um (#200) sieve.
 - 2) A plasticity index (PI) of less than 12.
 - 3) A liquid limit (LL) in excess of 50.
- d) Reclaimed asphalt shall not be used within the ground water table or as a fill if ground water is present.
- e) The RAP used shall be according to the current Bureau of Materials and Physical Research Policy Memorandum, "Reclaimed Asphalt Pavement (RAP) for Aggregate Applications". Gradation deleterious count shall not exceed 10% of total RAP and 5% of other by total weight.

CONSTRUCTION REQUIREMENTS

Samples. Embankment material shall be sampled, tested, and approved before use. The contractor shall identify embankment sources, and provide equipment as the Engineer requires, for the collection of samples from those sources. Samples will be furnished to the Geotechnical Engineer a minimum of three weeks prior to use in order that laboratory tests for approval and compaction can be performed. Embankment material placement cannot begin until tests are completed and approval given.

Placing Material. In addition to Article 202.03, broken concrete, reclaimed asphalt with no expansive aggregate, or uncontaminated dirt and sand generated from construction or demolition activities shall be placed in 6 inches (150 mm) lifts and disked with the underlying lift until a uniform homogenous material is formed. This process also applies to the overlaying lifts. The disk must have a minimum blade diameter of 24 inches (600 mm).

When embankments are to be constructed on hillsides or existing slopes that are steeper than 3H:1V, steps shall be keyed into the existing slope by stepping and benching as shown in the plans or as directed by the engineer.

Compaction. Soils classification for moisture content control will be determined by the Soils

Inspector using visual field examination techniques and the IDH Textural Classification Chart. When tested for density in place each lift shall have a maximum moisture content as follows.

- a) A maximum of 110 percent of the optimum moisture for all forms of clay soils.
- b) A maximum of 105 percent of the optimum moisture for all forms of clay loam soils.

Stability. The requirement for embankment stability in Article 205.04 will be measured with a Dynamic Cone Penetrometer (DCP) according to the test method in the IDOT Geotechnical Manual. The penetration rate must be equal or less than 1.5 inches (38 mm) per blow.

Basis of Payment. This work will not be paid separately but will be considered as included in the various items of excavation.

FRICTION AGGREGATE (D-1)

Effective: January 1, 2011

Revised: July 24, 2015

Revise Article 1004.01(a)(4) of the Standard Specifications to read:

“(4) Crushed Stone. Crushed stone shall be the angular fragments resulting from crushing undisturbed, consolidated deposits of rock by mechanical means. Crushed stone shall be divided into the following, when specified.

a. Carbonate Crushed Stone. Carbonate crushed stone shall be either dolomite or limestone. Dolomite shall contain 11.0 percent or more magnesium oxide (MgO). Limestone shall contain less than 11.0 percent magnesium oxide (MgO).

b. Crystalline Crushed Stone. Crystalline crushed stone shall be either metamorphic or igneous stone, including but is not limited to, quartzite, granite, rhyolite and diabase.”

Revise Article 1004.03(a) of the Standard Specifications to read:

“**1004.03 Coarse Aggregate for Hot-Mix Asphalt (HMA).** The aggregate shall be according to Article 1004.01 and the following.

(a) Description. The coarse aggregate for HMA shall be according to the following table.

Use	Mixture	Aggregates Allowed
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Class A	Seal or Cover	<u>Allowed Alone or in Combination</u> ^{5/} : Gravel Crushed Gravel Carbonate Crushed Stone Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag Crushed Concrete
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Use	Mixture	Aggregates Allowed
HMA Low ESAL	Stabilized Subbase or Shoulders	<u>Allowed Alone or in Combination</u> ^{5/} : Gravel Crushed Gravel Carbonate Crushed Stone Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{1/} Crushed Concrete
HMA High ESAL Low ESAL	Binder IL-19.0 or IL-19.0L SMA Binder	<u>Allowed Alone or in Combination</u> ^{5/} : Crushed Gravel Carbonate Crushed Stone ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Concrete ^{3/}
HMA High ESAL Low ESAL	C Surface and Leveling Binder IL-9.5 or IL-9.5L SMA Ndesign 50 Surface	<u>Allowed Alone or in Combination</u> ^{5/} : Crushed Gravel Carbonate Crushed Stone ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{4/} Crushed Concrete ^{3/}

HMA High ESAL	D Surface and Leveling Binder IL-9.5 SMA Ndesign 50 Surface	Allowed Alone or in Combination ^{5/} : Crushed Gravel Carbonate Crushed Stone (other than Limestone) ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{4/} Crushed Concrete ^{3/}	
		<u>Other Combinations Allowed:</u>	
		<i>Up to...</i>	<i>With...</i>
		25% Limestone	Dolomite
		50% Limestone	Any Mixture D aggregate other than Dolomite

Use	Mixture	Aggregates Allowed	
		75% Limestone	Crushed Slag (ACBF) or Crushed Sandstone
HMA High ESAL	E Surface IL-9.5 SMA Ndesign 80 Surface	Allowed Alone or in Combination ^{5/} :	
		Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag No Limestone.	
		<u>Other Combinations Allowed:</u>	
		<i>Up to...</i>	<i>With...</i>
		50% Dolomite ^{2/}	Any Mixture E aggregate
		75% Dolomite ^{2/}	Crushed Sandstone, Crushed Slag (ACBF), Crushed Steel Slag, or Crystalline Crushed Stone

		75% Crushed Gravel ^{2/} or Crushed Concrete ^{3/}	Crushed Sandstone, Crystalline Crushed Stone, Crushed Slag (ACBF), or Crushed Steel Slag
HMA High ESAL	F Surface IL-9.5 SMA Ndesign 80 Surface	<u>Allowed Alone or in Combination</u> ^{5/} :	
		Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag No Limestone.	
		<u>Other Combinations Allowed:</u>	
		<i>Up to...</i>	<i>With...</i>
Use	Mixture	Aggregates Allowed	
		50% Crushed Gravel ^{2/} , Crushed Concrete ^{3/} , or Dolomite ^{2/}	Crushed Sandstone, Crushed Slag (ACBF), Crushed Steel Slag, or Crystalline Crushed Stone

1/ Crushed steel slag allowed in shoulder surface only.

2/ Carbonate crushed stone and/or crushed gravel shall not be used in SMA Ndesign 80.

In SMA Ndesign 50, carbonate crushed stone shall not be blended with any of the other aggregates allowed alone in Ndesign 50 SMA binder or Ndesign 50 SMA surface.

3/ Crushed concrete will not be permitted in SMA mixes.

4/ Crushed steel slag shall not be used as leveling binder.

When combinations of aggregates are used, the blend percent measurements shall be by volume.”

GROUND TIRE RUBBER (GTR) MODIFIED ASPHALT BINDER (D-1)

Effective: June 26, 2006

Revised: January 1, 2013

Add the following to the end of article 1032.05 of the Standard Specifications:

“(c) Ground Tire Rubber (GTR) Modified Asphalt Binder. A quantity of 10.0 to 14.0 percent GTR (Note 1) shall be blended by dry unit weight with a PG 64-28 to make a GTR 70-28 or a PG 58-28 to make a GTR 64-28. The base PG 64-28 and PG 58-28 asphalt binders shall meet the requirements of Article 1032.05(a). Compatible polymers may be added during production. The GTR modified asphalt binder shall meet the requirements of the following table.

Test	Asphalt Grade GTR 70-28	Asphalt Grade GTR 64-28
Flash Point (C.O.C.), AASHTO T 48, °F (°C), min.	450 (232)	450 (232)
Rotational Viscosity, AASHTO T 316 @ 275 °F (135 °C), Poises, Pa·s, max.	30 (3)	30 (3)
Softening Point, AASHTO T 53, °F (°C), min.	135 (57)	130 (54)
Elastic Recovery, ASTM D 6084, Procedure A (sieve waived) @ 77 °F, (25 °C), aged, ss, 100 mm elongation, 5 cm/min., cut immediately, %, min.	65	65

Note 1. GTR shall be produced from processing automobile and/or light truck tires by the ambient grinding method. GTR shall not exceed 1/16 in. (2 mm) in any dimension and shall contain no free metal particles or other materials. A mineral powder (such as talc) meeting the requirements of AASHTO M 17 may be added, up to a maximum of four percent by weight of GTR to reduce sticking and caking of the GTR particles. When tested in accordance with Illinois modified AASHTO T 27, a 50 g sample of the GTR shall conform to the following gradation requirements:

Sieve Size	Percent Passing
No. 16 (1.18 mm)	100
No. 30 (600 μ m)	95 \pm 5
No. 50 (300 μ m)	> 20

Add the following to the end of Note 1. of article 1030.03 of the Standard Specifications:

“A dedicated storage tank for the Ground Tire Rubber (GTR) modified asphalt binder shall be provided. This tank must be capable of providing continuous mechanical mixing throughout by continuous agitation and recirculation of the asphalt binder to provide a uniform mixture. The tank shall be heated and capable of maintaining the temperature of the asphalt binder at 300 \pm F to 350 \pm F (149 \pm C to 177 \pm C). The asphalt binder metering systems of dryer drum plants shall be calibrated with the actual GTR modified asphalt binder material with an accuracy of \pm 0.40 percent.”

Revise 1030.02(c) of the Standard Specifications to read:

“(c) RAP Materials (Note 3)1031”

Add the following note to 1030.02 of the Standard Specifications:

Note 3. When using reclaimed asphalt pavement and/or reclaimed asphalt shingles, the maximum asphalt binder replacement percentage shall be according to the most recent special provision for recycled materials.

HMA MIXTURE DESIGN REQUIREMENTS (D-1)

Effective: January 1, 2013

Revised: November 1, 2014

1) Design Composition and Volumetric Requirements

Revise the last sentence of the first paragraph of Article 312.05 of the Standard Specifications to read:

“ The minimum compacted thickness of each lift shall be according to Article 406.06(d).” Delete the minimum compacted lift thickness table in Article 312.05 of the Standard Specifications.

Revise the second paragraph of Article 355.02 of the Standard Specifications to read: “ The mixture composition used shall be IL-19.0.”

Revise Article 355.05(a) of the Standard Specifications to read:

“(a) The top lift thickness shall be 2 1/4 in. (60 mm) for mixture composition IL-19.0.”

Revise the Leveling Binder table and second paragraph of Article 406.05(c) of the Standard Specifications to read:

“Leveling Binder	
Nominal, Compacted, Leveling Binder Thickness, in. (mm)	Mixture Composition
≤ 1 1/4 (32)	IL-4.75, IL-9.5, or IL-9.5L
> 1 1/4 to 2 (32 to 50)	IL-9.5 or IL-9.5L

The density requirements of Article 406.07(c) shall apply for leveling binder, machine method, when the nominal compacted thickness is: 3/4 in. (19 mm) or greater for IL-4.75 mixtures; and 1 1/4 in. (32 mm) or greater for IL-9.5 and IL-9.5L mixtures.”

Revise the table in Article 406.06(d) of the Standard Specifications to read:

“MINIMUM COMPACTED LIFT THICKNESS	
Mixture Composition	Thickness, in. (mm)
IL-4.75	3/4 (19)
SMA-9.5, IL-9.5, IL-9.5L	1 1/2 (38)
SMA-12.5	2 (50)
IL-19.0, IL-19.0L	2 1/4 (57)”

Revise the ninth paragraph of Article 406.14 of the Standard Specifications to read:
“Test strip mixture will be evaluated at the contract unit price according to the following.”

Revise Article 406.14(a) of the Standard Specifications to read:

“(a) If the HMA placed during the initial test strip is determined to be acceptable the mixture will be paid for at the contract unit price.”

Revise Article 406.14(b) of the Standard Specifications to read:

“(b) If the HMA placed during the initial test strip (1) is determined to be unacceptable to remain in place by the Engineer, and (2) was not produced within 2.0 to 6.0 percent air voids or within the individual control limits of the JMF according to the Department’s test results, the mixture will not be paid for and shall be removed at the Contractor’s expense. An additional test strip shall be constructed and the mixture will be paid for in full, if produced within 2.0 to 6.0 percent air voids and within the individual control limits of the JMF.”

Revise Article 406.14(c) of the Standard Specifications to read:

“(c) If the HMA placed during the initial test strip (1) is determined to be unacceptable to remain in place by the Engineer, and (2) was produced within 2.0 to 6.0 percent air voids and within the individual control limits of the JMF according to the Department’s test results, the mixture shall be removed. Removal will be paid according to Article 109.04. This initial mixture will be paid for at the contract unit price. An additional test strip shall be constructed and the mixture will be paid for in full, if produced within 2.0 to 6.0 percent air voids and within the individual control limits of the JMF.”

Delete Article 406.14(d) of the Standard Specifications. Delete Article 406.14(e) of the Standard Specifications.

Delete the last sentence of Article 407.06(c) of the Standard Specifications. Revise Note 2. of Article 442.02 of the Standard Specifications to read:

“Note 2. The mixture composition of the HMA used shall be IL-19.0 binder, designed with the same Ndesign as that specified for the mainline pavement.”

Delete the second paragraph of Article 482.02 of the Standard Specifications.

Revise the first sentence of the sixth paragraph of Article 482.05 of the Standard Specifications to read:

“When the mainline HMA binder and surface course mixture option is used on resurfacing projects, shoulder resurfacing widths of 6 ft (1.8 m) or less may be placed simultaneously with the adjacent traffic lane for both the binder and surface courses.”

Revise the second sentence of the fourth paragraph of Article 601.04 of the Standard Specifications to read:

“The top 5 in. (125 mm) of the trench shall be backfilled with an IL-19.0L Low ESAL mixture meeting the requirements of Section 1030 and compacted to a density of not less than 90 percent of the theoretical density.”

Revise the second sentence of the fifth paragraph of Article 601.04 of the Standard Specifications to read:

“The top 8 in. (200 mm) of the trench shall be backfilled with an IL-19.0L Low ESAL mixture meeting the requirements of Section 1030 and compacted to a density of not less than 90 percent of the theoretical density.”

Revise Article 1003.03(c) of the Standard Specifications to read:

“(c) Gradation. The fine aggregate gradation for all HMA shall be FA 1, FA 2, FA 20, FA 21, or FA 22. The fine aggregate gradation for SMA shall be FA/FM 20.

For mixture IL-4.75 and surface mixtures with an Ndesign = 90, at least 50 percent of the required fine aggregate fraction shall consist of either stone sand, slag sand, or steel slag meeting the FA 20 gradation.

For mixture IL-19.0, Ndesign = 90 the fine aggregate fraction shall consist of at least 67 percent manufactured sand meeting FA 20 or FA 22 gradation. For mixture IL-19.0, Ndesign = 50 or 70 the fine aggregate fraction shall consist of at least 50 percent manufactured sand meeting FA 20 or FA 22 gradation. The manufactured sand shall be stone sand, slag sand, steel slag sand, or combinations thereof.

Gradation FA 1, FA 2, or FA 3 shall be used when required for prime coat aggregate application for HMA.”

Delete the last sentence of the first paragraph of Article 1004.03(b) of the Standard Specifications.

Revise the table in Article 1004.03(c) of the Standard Specifications to read:

“Use	Size/Application	Gradation No.
Class A-1, 2, & 3	3/8 in. (10 mm) Seal	CA 16
Class A-1	1/2 in. (13 mm) Seal	CA 15
Class A-2 & 3	Cover	CA 14
HMA High ESAL	IL-19.0 IL-9.5	CA 11 ^{1/} CA 16 CA 13 ^{3/}
HMA Low ESAL	IL-19.0L IL-9.5L Stabilized Subbase or Shoulders	CA 11 ^{1/} CA 16
SMA ^{2/}	1/2 in. (12.5mm) Binder & Surface IL 9.5 Surface	CA13 ^{3/} , CA14 or CA16 CA16, CA 13 ^{3/}

1/ CA 16 or CA 13 may be blended with the gradations listed.

2/ The coarse aggregates used shall be capable of being combined with stone sand, slag sand, or steel slag sand meeting the FA/FM 20 gradation and mineral filler to meet the approved mix design and the mix requirements noted herein.

3/ CA 13 shall be 100 percent passing the 1/2 in. (12.5mm) sieve. Revise Article 1004.03(e) of the

Supplemental Specifications to read:

“(e) Absorption. For SMA the coarse aggregate shall also have water absorption
≤ 2.0 percent.”

Revise the nomenclature table in Article 1030.01 of the Standard Specifications to read:

“High ESAL	IL-19.0 binder; IL-9.5 surface; IL-4.75; SMA-12.5, SMA-9.5
Low ESAL	IL-19.0L binder; IL-9.5L surface; Stabilized Subbase (HMA) ^{1/} ; HMA Shoulders ^{2/}

1/ Uses 19.0L binder mix.

2/ Uses 19.0L for lower lifts and 9.5L for surface lift.”

Revise Article 1030.02 of the Standard Specifications and Supplemental Specifications to read: “**1030.02**

Materials. Materials shall be according to the following.

Item	Article/Section
(a) Coarse Aggregate	1004.03
(b) Fine Aggregate	1003.03
(c) RAP Material	1031
(d) Mineral Filler	1011
(e) Hydrated Lime	1012.01
(f) Slaked Quicklime (Note 1)	
(g) Performance Graded Asphalt Binder (Note 2)	1032
(h) Fibers (Note 3)	
(i) Warm Mix Asphalt (WMA) Technologies (Note 4)	

Note 1. Slaked quicklime shall be according to ASTM C 5.

Note 2. The asphalt binder shall be an SBS PG 76-28 when the SMA is used on a full- depth asphalt pavement and SBS PG 76-22 when used as an overlay, except where modified herein. The asphalt binder shall be an Elvaloy or SBS PG 76-22 for IL-4.75, except where modified herein. The elastic recovery shall be a minimum of 80.

Note 3. A stabilizing additive such as cellulose or mineral fiber shall be added to the SMA mixture according to Illinois Modified AASHTO M 325. The stabilizing additive shall meet the Fiber Quality

Requirements listed in Illinois Modified AASHTO M 325. Prior to approval and use of fibers, the Contractor shall submit a notarized certification by the producer of these materials stating they meet these requirements. Reclaimed Asphalt Shingles (RAS) may be used in Stone Matrix Asphalt (SMA) mixtures designed with an SBA polymer modifier as a fiber additive if the mix design with RAS included meets AASHTO T305 requirements. The RAS shall be from a certified source that produces either Type I or Type 2. Material shall meet requirements noted herein and the actual dosage rate will be determined by the Engineer.

Note 4. Warm mix additives or foaming processes shall be selected from the current Bureau of Materials and Physical Research Approved List, “Warm Mix Asphalt Technologies”.”

Revise Article 1030.04(a)(1) of the Standard Specifications and the Supplemental Specifications to read:

“(1) High ESAL Mixtures. The Job Mix Formula (JMF) shall fall within the following limits.

High ESAL, MIXTURE COMPOSITION (% PASSING) ^{1/}										
Sieve Size	IL-19.0 mm		SMA ^{4/} IL-12.5 mm		SMA ^{4/} IL-9.5 mm		IL-9.5 mm		IL-4.75 mm	
	min	max	min	max	min	max	min	max	min	max
1 1/2 in. (37.5 mm)										
1 in. (25 mm)		100								
3/4 in. (19 mm)	90	100		100						
1/2 in. (12.5 mm)	75	89	80	100		100		100		100
3/8 in. (9.5 mm)				65	90	100	90	100		100
#4 (4.75 mm)	40	60	20	30	36	50	34	69	90	100
#8 (2.36 mm)	20	42	16	24 ^{5/}	16	32 ^{5/}	34 ^{6/}	52 ^{2/}	70	90
#16 (1.18 mm)	15	30					10	32	50	65
#30 (600 µm)			12	16	12	18				
#50 (300 µm)	6	15					4	15	15	30
#100 (150 µm)	4	9				9.5	3	10	10	18
#200 (75 µm)	3	6	7.0	3/	7.5	3/	4	6	7	3/
Ratio Dust/Asphalt Binder		1.0		1.5		1.5		1.0		1.0

1/ Based on percent of total aggregate weight.

2/ The mixture composition shall not exceed 44 percent passing the #8 (2.36 mm) sieve for surface courses with N_{design} = 90.

3/ Additional minus No. 200 (0.075 mm) material required by the mix design shall be mineral filler, unless otherwise approved by the Engineer.

4/ The maximum percent passing the #635 (20 μ m) sieve shall be \leq 3 percent.

5/ When establishing the Adjusted Job Mix Formula (AJMF) the percent passing the #8 (2.36 mm) sieve shall not be adjusted above the percentage stated on the table.

6/ When establishing the Adjusted Job Mix Formula (AJMF) the percent passing the #8 (2.36 mm) sieve shall not be adjusted below 34 percent.

Delete Article 1030.04(a)(3) of the Standard Specifications.

Delete Article 1030.04(a)(4) of the Standard Specifications.

Revise Article 1030.04(b)(1) of the Standard Specifications to read:

“(1) High ESAL Mixtures. The target value for the air voids of the HMA shall be 4.0 percent and for IL-4.75 it shall be 3.5 percent at the design number of gyrations. The VMA and VFA of the HMA design shall be based on the nominal maximum size of the aggregate in the mix, and shall conform to the following requirements.

VOLUMETRIC REQUIREMENTS High ESAL				
	Voids in the Mineral Aggregate (VMA), % minimum			Voids Filled with Asphalt Binder (VFA), %
N _{design}	IL-19.0	IL-9.5	IL-4.75 ^{1/}	
50	13.5	15.0	18.5	65 – 78 ^{2/}
70				65 - 75
90				

1/ Maximum Draindown for IL-4.75 shall be 0.3 percent

2/ VFA for IL-4.75 shall be 72-85 percent”

Revise the table in Article 1030.04(b)(2) of the Standard Specifications to read:

“VOLUMETRIC REQUIREMENTS Low ESAL				
Mixture Composition	Design Compactive Effort	Design Air Voids Target %	VMA (Voids in the Mineral Aggregate), % min.	VFA (Voids Filled with Asphalt Binder), %
IL-9.5L	NDES =30	4.0	15.0	65-78
IL-19.0L	NDES =30	4.0	13.5	N/A”

Replace Article 1030.04(b)(3) of the Standard Specifications with the following:

“(3) SMA Mixtures.

Volumetric Requirements			
SMA ^{1/}	Design Air Voids Target %	Voids in the Mineral Aggregate (VMA), % min.	Voids Filled with Asphalt (VFA), %
80 ^{4/}	3.5	17.0 ^{2/} 16.0 ^{3/}	75 - 83

1/ Maximum draindown shall be 0.3 percent. The draindown shall be determined at the JMF asphalt binder content at the mixing temperature plus 30 °F.

2/ Applies when specific gravity of coarse aggregate is ≥ 2.760 .

3/ Applies when specific gravity of coarse aggregate is < 2.760 .

4/ Blending of different types of aggregate will not be permitted.

For surface course, the coarse aggregate can be crushed steel slag, crystalline crushed stone or crushed sandstone. For binder course, coarse aggregate shall be crushed stone (dolomite), crushed gravel, crystalline crushed stone, or crushed sandstone.

Delete Article 1030.04(b)(4) of the Standard Specifications.

Delete Article 1030.04(b)(5) from the Supplemental Specifications.

Delete last sentence of the second paragraph of Article 1102.01(a) (13) a. Add to second paragraph in Article

1102.01 (a) (13) a.:

“As an option, collected bag-house dust may be used in lieu of manufactured mineral filler, provided; 1) there is enough available for the production of the SMA mix for the entire project and 2) a mix design was prepared with collected bag-house dust.”

Revise the table in Article 1030.05(d)(2)a. of the Standard Specifications to read:

Parameter	Frequency of Tests	Test Method
	High ESAL Mixture Low ESAL Mixture	See Manual of Test Procedures for Materials
Aggregate Gradation % passing sieves: 1/2 in. (12.5 mm), No. 4 (4.75 mm), No. 8 (2.36 mm), No. 30 (600 µm) No. 200 (75 µm)	1 washed ignition oven test on the mix per half day of production Note 3.	Illinois Procedure
Asphalt Binder Content by Ignition Oven Note 1.	1 per half day of production	Illinois-Modified AASHTO T 308
VMA Note 2.	Day's production ≥ 1200 tons: 1 per half day of production Day's production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)	Illinois-Modified AASHTO R 35
Air Voids Bulk Specific Gravity of Gyratory Sample Note 4.	Day's production ≥ 1200 tons: 1 per half day of production Day's production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)	Illinois-Modified AASHTO T 312
Maximum Specific Gravity of Mixture	Day's production ≥ 1200 tons: 1 per half day of production	Illinois-Modified AASHTO T 209

“Parameter	Frequency of Tests	Test Method See Manual of Test Procedures for Materials
	High ESAL Mixture Low ESAL Mixture	
	Day’s production < 1200 tons: 1 per half day of production for first 2 days and 1 per day thereafter (first sample of the day)	

Note 1. The Engineer may waive the ignition oven requirement for asphalt binder content if the aggregates to be used are known to have ignition asphalt binder content calibration factors which exceed 1.5 percent. If the ignition oven requirement is waived, other Department approved methods shall be used to determine the asphalt binder content.

Note 2. The G_{sb} used in the voids in the mineral aggregate (VMA) calculation shall be the same average G_{sb} value listed in the mix design.

Note 3. The Engineer reserves the right to require additional hot bin gradations for batch plants if control problems are evident.

Note 4. The WMA compaction temperature for mixture volumetric testing shall be 270 ± 5 °F (132 ± 3 °C) for quality control testing. The WMA compaction temperature for quality assurance testing will be 270 ± 5 °F (132 ± 3 °C) if the mixture is not allowed to cool to room temperature. If the mixture is allowed to cool to room temperature, it shall be reheated to standard HMA compaction temperatures.”

Revise the table in Article 1030.05(d)(2)b. of the Standard Specifications to read:

“Parameter	High ESAL Mixture Low ESAL Mixture
Ratio Dust/Asphalt Binder	0.6 to 1.2
Moisture	0.3 %”

Revise the Article 1030.05(d)(4) of the Supplemental Specifications to read:

“(4) Control Limits. Target values shall be determined by applying adjustment factors to the AJMF where applicable. The target values shall be plotted on the control charts within the following control limits.

“CONTROL LIMITS						
Parameter	High ESAL		SMA		IL-4.75	
	Individual Test	Moving Avg. of 4	Test	Moving Avg. of 4	Individual Test	Moving Avg. of 4
% Passing ^{1/}						
1/2 in. (12.5 mm)	± 6 %	± 4 %	± 6 %	± 4 %		
3/8 in. (9.5mm)			± 4 %	± 3 %		
No. 4 (4.75 mm)	± 5 %	± 4 %	± 5 %	± 4 %		
No. 8 (2.36 mm)	± 5 %	± 3 %	± 4 %	± 2 %		
No. 16 (1.18 mm)			± 4 %	± 2 %	± 4 %	± 3 %
No. 30 (600 µm)	± 4 %	± 2.5 %	± 4 %	± 2.5 %		
Total Dust Content No. 200 (75 µm)	± 1.5 %	± 1.0 %			± 1.5 %	± 1.0 %
Asphalt Binder Content	± 0.3 %	± 0.2 %	± 0.2 %	± 0.1 %	± 0.3 %	± 0.2 %
Voids	± 1.2 %	± 1.0 %	± 1.2 %	± 1.0 %	± 1.2 %	± 1.0 %
VMA	-0.7 % ^{2/}	-0.5 % ^{2/}	-0.7 % ^{2/}	-0.5 % ^{2/}	-0.7 % ^{2/}	-0.5 % ^{2/}

1/ Based on washed ignition oven

2/ Allowable limit below minimum design VMA requirement

DENSITY CONTROL LIMITS		
Mixture Composition	Parameter	Individual Test
IL-4.75	N _{design} = 50	93.0 - 97.4 % ^{1/}
IL-9.5	N _{design} = 90	92.0 - 96.0 %
IL-9.5, IL-9.5L	N _{design} < 90	92.5 - 97.4 %
IL-19.0	N _{design} = 90	93.0 - 96.0 %
IL-19.0, IL-19.0L	N _{design} < 90	93.0 ^{2/} - 97.4 %
SMA	N _{design} = 80	93.5 - 97.4 %

1/ Density shall be determined by cores or by correlated, approved thin lift nuclear gauge.

2/ 92.0 % when placed as first lift on an unimproved subgrade.”

Revise the table in Article 1030.05(d)(5) of the Supplemental Specifications to read:

“CONTROL CHART REQUIREMENTS	High ESAL, Low ESAL, SMA & IL-4.75
Gradation ^{1/} ^{3/}	% Passing Sieves: 1/2 in. (12.5 mm) ^{2/} No. 4 (4.75 mm) No. 8 (2.36 mm) No. 30 (600 µm)
Total Dust Content ^{1/}	No. 200 (75 µm)
	Asphalt Binder Content
	Bulk Specific Gravity
	Maximum Specific Gravity of Mixture
	Voids
	Density
	VMA

1/ Based on washed ignition oven.

2/ Does not apply to IL-4.75.

3/ SMA also requires the 3/8 in. (9.5 mm) sieve.” Delete Article 1030.05(d)(6)a.1.(b.) of the Standard Specifications.

Delete Article 1030.06(b) of the Standard Specifications. Delete Article 1102.01(e) of the Standard Specifications.

2) Design Verification and Production

Description. The following states the requirements for Hamburg Wheel and Tensile Strength testing for High ESAL, IL-4.75, and Stone Matrix Asphalt (SMA) hot-mix asphalt (HMA) mixes during mix design verification and production.

Mix Design Testing. Add the following below the referenced AASHTO standards in Article 1030.04 of the Standard Specifications: AASHTO T 324 Hamburg Wheel Test AASHTO T 283

Tensile Strength Test

Add the following to Article 1030.04 of the Standard Specifications:

“(d) Verification Testing. High ESAL, IL-4.75, and SMA mix designs submitted for verification will be tested to ensure that the resulting mix designs will pass the required criteria for the Hamburg Wheel Test (IL mod AASHTO T-324) and the Tensile Strength Test (IL mod AASHTO T-283). The Department will perform a verification test on gyratory specimens compacted by the Contractor. If the mix fails the Department’s verification test, the Contractor shall make the necessary changes to the mix and resubmit compacted specimens to the Department for verification. If the mix fails again, the mix design will be rejected.

All new and renewal mix designs will be required to be tested, prior to submittal for Department verification and shall meet the following requirements:

(1)Hamburg Wheel Test criteria. The maximum allowable rut depth shall be 0.5 in. (12.5 mm). The minimum number of wheel passes at the 0.5 in. (12.5 mm) rut depth criteria shall be based on the high temperature binder grade of the mix as specified in the mix requirements table of the plans.

Illinois Modified AASHTO T 324 Requirements ^{1/}

Asphalt Binder Grade	# Repetitions	Max Rut Depth (mm)
PG 70 -XX (or higher)	20,000	12.5
PG 64 -XX (or lower)	10,000	12.5

^{1/} When produced at temperatures of 275 ± 5 °F (135 ± 3 °C) or less, loose Warm Mix Asphalt shall be oven aged at 270 ± 5 °F (132 ± 3 °C) for two hours prior to gyratory compaction of Hamburg Wheel specimens.

Note: For SMA Designs (N-80) the maximum rut depth is 6.0 mm at 20,000 repetitions. For IL 4.75mm Designs (N-50) the maximum rut depth is 9.0mm at 15,000 repetitions.

(2) Tensile Strength Criteria. The minimum allowable conditioned tensile strength shall be 60 psi (415 kPa) for non-polymer modified performance graded (PG) asphalt binder and 80 psi (550 kPa) for polymer modified PG asphalt binder. The maximum allowable unconditioned tensile strength shall be 200 psi (1380 kPa).”

Production Testing. Revise Article 1030.06(a) of the Standard Specifications to read:

“(a) High ESAL, IL-4.75, WMA, and SMA Mixtures. For each contract, a 300 ton (275 metric tons) test strip, except for SMA mixtures it will be 400 ton (363 metric ton), will be required at the beginning of HMA production for each mixture with a quantity of 3000 tons (2750 metric tons) or more according to the Manual of Test Procedures for Materials “Hot Mix Asphalt Test Strip Procedures”.

Before start-up, target values shall be determined by applying gradation correction factors to the JMF when applicable. These correction factors shall be determined from previous experience. The target values, when approved by the Engineer, shall be used to control HMA production. Plant settings and control charts shall be set according to target values.

Before constructing the test strip, target values shall be determined by applying gradation correction factors to the JMF when applicable. After any JMF adjustment, the JMF shall become the Adjusted Job Mix Formula (AJMF). Upon completion of the first acceptable test strip, the JMF shall become the AJMF regardless of whether or not the JMF has been adjusted. If an adjustment/plant change is made, the Engineer may require a new test strip to be constructed. If the HMA placed during the initial test strip is determined to be unacceptable to remain in place by the Engineer, it shall be removed and replaced.

The limitations between the JMF and AJMF are as follows.

Parameter	Adjustment
1/2 in. (12.5 mm)	± 5.0 %
No. 4 (4.75 mm)	± 4.0 %
No. 8 (2.36 mm)	± 3.0 %
No. 30 (600 µm)	*
No. 200 (75 µm)	*
Asphalt Binder Content	± 0.3 %

* In no case shall the target for the amount passing be greater than the JMF. Any adjustments outside the above limitations will require a new mix design.

Mixture sampled to represent the test strip shall include additional material sufficient for the Department to conduct Hamburg Wheel testing according to Illinois Modified AASHTO T324 (approximately 60 lb (27 kg) total).

The Contractor shall immediately cease production upon notification by the Engineer of failing Hamburg Wheel test. All prior produced material may be paved out provided all other mixture criteria is being met. No additional mixture shall be produced until the Engineer receives passing Hamburg Wheel tests.

The Department may conduct additional Hamburg Wheel tests on production material as determined by the Engineer.”

Revise the title of Article 1030.06(b) of the Standard Specifications to read: “(b) Low ESAL Mixtures.”

Add the following to Article 1030.06 of the Standard Specifications:

“(c) Hamburg Wheel Test. All HMA mixtures shall be sampled within the first 500 tons (450 metric tons) on the first day of production or during start up with a split reserved for the Department. The mix sample shall be tested according to the Illinois Modified AASHTO T 324 and shall meet the requirements specified herein. Mix production shall not exceed 1500 tons (1350 metric tons) or one day’s production, whichever comes first, until the testing is completed and the mixture is found to be in conformance. The requirement to cease mix production may be waived if the plant produced mixture demonstrates conformance prior to start of mix production for a contract.

The Department may conduct additional Hamburg Wheel Tests on production material as determined by the Engineer. If the mixture fails to meet the Hamburg Wheel criteria, no further mixture will be accepted until the Contractor takes such action as is necessary to furnish a mixture meeting the criteria”

The Contractor shall immediately cease production upon notification by the Engineer of failing Hamburg Wheel test. All prior produced material may be paved out provided all other mixture criteria are being met. No additional mixture shall be produced until the Engineer receives passing Hamburg Wheel tests.

Method of Measurement:

Add the following after the fourth paragraph of Article 406.13 (b):

“The plan quantities of SMA mixtures shall be adjusted using the actual approved binder and surface Mix Design’s Gmb.”

Basis of Payment.

Replace the seventh paragraph of Article 406.14 of the Standard Specifications with the following:

“For all mixes designed and verified under the Hamburg Wheel criteria, the cost of furnishing and introducing anti-stripping additives in the HMA will not be paid for separately, but shall be considered as included in the contract unit price of the HMA item involved.

No additional compensation will be awarded to the Contractor because of reduced production rates associated with the addition of the anti-stripping additive.”

PUBLIC CONVENIENCE AND SAFETY (DIST 1)

Effective: May 1, 2012

Revised: July 15, 2012

Add the following to the end of the fourth paragraph of Article 107.09:

“If the holiday is on a Saturday or Sunday, and is legally observed on a Friday or Monday, the length of Holiday Period for Monday or Friday shall apply.”

Add the following sentence after the Holiday Period table in the fourth paragraph of Article 107.09:

“The Length of Holiday Period for Thanksgiving shall be from 5:00 AM the Wednesday prior to 11:59 PM the Sunday After”

Delete the fifth paragraph of Article 107.09 of the Standard Specifications:

“On weekends, excluding holidays, roadways with Average Daily Traffic of 25,000 or greater, all lanes shall be open to traffic from 3:00 P.M. Friday to midnight Sunday except where structure construction or major rehabilitation makes it impractical.”

RECLAIMED ASPHALT PAVEMENT AND RECLAIMED ASPHALT SHINGLES (D-1)

Effective: November 1, 2012

Revise: July 24, 2015

Revise Section 1031 of the Standard Specifications to read:

“SECTION 1031. RECLAIMED ASPHALT PAVEMENT AND RECLAIMED ASPHALT SHINGLES

1031.01 Description. Reclaimed asphalt pavement and reclaimed asphalt shingles shall be according to the following.

(a) Reclaimed Asphalt Pavement (RAP). RAP is the material resulting from cold milling or crushing an existing hot-mix asphalt (HMA) pavement. RAP will be considered processed FRAP after completion of both crushing and screening to size. The Contractor shall supply written documentation that the RAP originated from routes or airfields under federal, state, or local agency jurisdiction.

(b) Reclaimed Asphalt Shingles (RAS). Reclaimed asphalt shingles (RAS). RAS is from the processing and grinding of preconsumer or post-consumer shingles. RAS shall be a clean and uniform material with a maximum of 0.5 percent unacceptable material, as defined in Bureau of Materials and Physical Research Policy Memorandum “Reclaimed Asphalt Shingle (RAS) Sources”, by weight of RAS. All RAS used shall come from a Bureau of Materials and Physical Research approved processing facility where it shall be ground and processed to 100 percent passing the 3/8 in. (9.5 mm) sieve and

90 percent passing the #4 (4.75 mm) sieve . RAS shall meet the testing requirements specified herein. In addition, RAS shall meet the following Type 1 or Type 2 requirements.

- (1) Type 1. Type 1 RAS shall be processed, preconsumer asphalt shingles salvaged from the manufacture of residential asphalt roofing shingles.
- (2) Type 2. Type 2 RAS shall be processed post-consumer shingles only, salvaged from residential, or four unit or less dwellings not subject to the National Emission Standards for Hazardous Air Pollutants (NESHAP).

1031.02 Stockpiles. RAP and RAS stockpiles shall be according to the following.

(a) RAP Stockpiles. The Contractor shall construct individual, sealed RAP stockpiles meeting one of the following definitions. Additional processed RAP (FRAP) shall be stockpiled in a separate working pile, as designated in the QC Plan, and only added to the sealed stockpile when test results for the working pile are complete and are found to meet tolerances specified herein for the original sealed FRAP stockpile. Stockpiles shall be sufficiently separated to prevent intermingling at the base. All stockpiles (including unprocessed RAP and FRAP) shall be identified by signs indicating the type as listed below (i.e. “Non-Quality, FRAP -#4 or Type 2 RAS”, etc...).

(1) Fractionated RAP (FRAP). FRAP shall consist of RAP from Class I, Superpave HMA (High and Low ESAL) or equivalent mixtures. The coarse aggregate in FRAP shall be crushed aggregate and may represent more than one aggregate type and/or quality but shall be at least C quality. All FRAP shall be processed prior to testing and sized into fractions with the separation occurring on or between the #4 (4.75 mm) and 1/2 in. (12.5 mm) sieves. Agglomerations shall be minimized such that 100 percent of the RAP in the coarse fraction shall pass the maximum sieve size specified for the mix the FRAP will be used in.

(2) Restricted FRAP (B quality) stockpiles shall consist of RAP from Class I, Superpave (High ESAL), or HMA (High ESAL). If approved by the Engineer, the aggregate from a maximum 3.0 inch single combined pass of surface/binder milling will be classified as B quality. All millings from this application will be processed into FRAP as described previously.

(3) Conglomerate. Conglomerate RAP stockpiles shall consist of RAP from Class I, Superpave HMA (High and Low ESAL) or equivalent mixtures. The coarse aggregate in this RAP shall be crushed aggregate and may represent more than one aggregate type and/or quality but shall be at least C quality. This RAP may have an inconsistent gradation and/or asphalt binder content prior to processing. All conglomerate RAP shall be processed (FRAP) prior to testing. Conglomerate RAP stockpiles shall not contain steel slag or other expansive material as determined by the Department.

(4) Conglomerate “D” Quality (DQ). Conglomerate DQ RAP stockpiles shall consist of RAP from HMA shoulders, bituminous stabilized subbases or Superpave (Low ESAL)/HMA (Low ESAL) IL-19.0L binder mixture. The coarse aggregate in this RAP may be crushed or round but shall be at least D quality. This RAP may have an inconsistent gradation and/or asphalt binder content. Conglomerate DQ RAP stockpiles shall not contain steel slag or other expansive material as determined by the Department.

(5) Non-Quality. RAP stockpiles that do not meet the requirements of the stockpile categories listed above shall be classified as “Non-Quality”.

RAP or FRAP containing contaminants, such as earth, brick, sand, concrete, sheet asphalt, bituminous surface treatment (i.e. chip seal), pavement fabric, joint sealants, plant cleanout etc., will be unacceptable unless the contaminants are removed to the satisfaction of the Engineer. Sheet asphalt shall be stockpiled separately.

(b) RAS Stockpiles. Type 1 and Type 2 RAS shall be stockpiled separately and shall be sufficiently separated to prevent intermingling at the base. Each stockpile shall be signed indicating what type of RAS is present.

However, a RAS source may submit a written request to the Department for approval to blend mechanically a specified ratio of type 1 RAS with type 2 RAS. The source will not be permitted to change the ratio of the blend without the Department prior written approval. The Engineer's written approval will be required, to mechanically blend RAS with any fine aggregate produced under the AGCS, up to an equal weight of RAS, to improve workability. The fine aggregate shall be "B Quality" or better from an approved Aggregate Gradation Control System source. The fine aggregate shall be one that is approved for use in the HMA mixture and accounted for in the mix design and during HMA production.

Records identifying the shingle processing facility supplying the RAS, RAS type and lot number shall be maintained by project contract number and kept for a minimum of three years.

1031.03 Testing. FRAP and RAS testing shall be according to the following.

(a) FRAP Testing. When used in HMA, the FRAP shall be sampled and tested either during processing or after stockpiling. It shall also be sampled during HMA production.

(1) During Stockpiling. For testing during stockpiling, washed extraction samples shall be run at the minimum frequency of one sample per 500 tons (450 metric tons) for the first 2000 tons (1800 metric tons) and one sample per 2000 tons (1800 metric tons) thereafter. A minimum of five tests shall be required for stockpiles less than 4000 tons (3600 metric tons).

(2) Incoming Material. For testing as incoming material, washed extraction samples shall be run at a minimum frequency of one sample per 2000 tons (1800 metric tons) or once per week, whichever comes first.

(3) After Stockpiling. For testing after stockpiling, the Contractor shall submit a plan for approval to the District proposing a satisfactory method of sampling and testing the RAP/FRAP pile either in-situ or by restockpiling. The sampling plan shall meet the minimum frequency required above and detail the procedure used to obtain representative samples throughout the pile for testing.

Before extraction, each field sample of FRAP, shall be split to obtain two samples of test sample size. One of the two test samples from the final split shall be labeled and stored for Department use. The Contractor shall extract the other test sample according to Department procedure. The Engineer reserves the right to test any sample (split or Department-taken) to verify Contractor test results.

(b) RAS Testing. RAS shall be sampled and tested during stockpiling according to Bureau of Materials and Physical Research Policy Memorandum, "Reclaimed Asphalt Shingle (RAS) Sources". The Contractor shall also sample as incoming material at the HMA plant.

(1) During Stockpiling. Washed extraction and testing for unacceptable materials shall be run at the minimum frequency of one sample per 200 tons (180 metric tons) for the first 1000 tons (900 metric tons) and one sample per 1000 tons (900 metric tons) thereafter. A minimum of five samples are required for stockpiles less than 1000 tons (900 metric tons). Once a ≤ 1000 ton (900 metric ton), five-sample/test stockpile has been established it shall be sealed. Additional incoming RAS shall be in a separate working pile as designated in the Quality Control plan and only added to the sealed stockpile when the test results of the working pile are complete and are found to meet the tolerances specified herein for the original sealed RAS stockpile.

(2) Incoming Material. For testing as incoming material at the HMA plant, washed extraction shall be run at the minimum frequency of one sample per 250 tons (227 metric tons). A minimum of five samples are required for stockpiles less than 1000 tons (900 metric tons). The incoming material test results shall meet the tolerances specified herein.

The Contractor shall obtain and make available all test results from start of the initial stockpile sampled and tested at the shingle processing facility in accordance with the facility's QC Plan.

Before extraction, each field sample shall be split to obtain two samples of test sample size. One of the two test samples from the final split shall be labeled and stored for Department use. The Contractor shall extract the other test sample according to Department procedures. The Engineer reserves the right to test any sample (split or Department-taken) to verify Contractor test results.

1031.04 Evaluation of Tests.

Evaluation of tests results shall be according to the following.

(a) Evaluation of FRAP Test Results. All test results shall be compiled to include asphalt binder content, gradation and, when applicable (for slag), Gmm. A five test average of results from the original pile will be used in the mix designs. Individual extraction test results run thereafter, shall be compared to the average used for the mix design, and will be accepted if within the tolerances listed below.

Parameter	FRAP
No. 4 (4.75 mm)	\square 6 %
No. 8 (2.36 mm)	\square 5 %
No. 30 (600 \square m)	\square 5 %
No. 200 (75 \square m)	\square 2.0 %
Asphalt Binder	\square 0.3 %
Gmm	\square 0.03 ^{1/}

1/ For stockpile with slag or steel slag present as determined in the current Manual of Test Procedures Appendix B 21, "Determination of Reclaimed Asphalt Pavement Aggregate Bulk Specific Gravity".

If any individual sieve and/or asphalt binder content tests are out of the above tolerances when compared to the average used for the mix design, the FRAP stockpile shall not be used in Hot-Mix Asphalt unless the FRAP representing those tests is removed from the stockpile. All test data and acceptance ranges shall be sent to the District for evaluation.

The Contractor shall maintain a representative moving average of five tests to be used for Hot-Mix Asphalt production.

With the approval of the Engineer, the ignition oven may be substituted for extractions according to the Illinois Test Procedure, "Calibration of the Ignition Oven for the Purpose of Characterizing Reclaimed Asphalt Pavement (RAP)" or Illinois Modified AASHTO T-164-11, Test Method A.

(b) Evaluation of RAS Test Results. All of the test results, with the exception of percent unacceptable materials, shall be compiled and averaged for asphalt binder content and gradation. A five test average of results from the original pile will be used in the mix designs. Individual test results run thereafter, when compared to the average used for the mix design, will be accepted if within the tolerances listed below.

Parameter	RAS
No. 8 (2.36 mm)	± 5 %
No. 16 (1.18 mm)	± 5 %
No. 30 (600 µm)	± 4 %
No. 200 (75 µm)	± 2.5 %
Asphalt Binder Content	± 2.0 %

If any individual sieve and/or asphalt binder content tests are out of the above tolerances when compared to the average used for the mix design, the RAS shall not be used in Hot-Mix Asphalt unless the RAS representing those tests is removed from the stockpile. All test data and acceptance ranges shall be sent to the District for evaluation.

(c) Quality Assurance by the Engineer. The Engineer may witness the sampling and splitting conduct assurance tests on split samples taken by the Contractor for quality control testing a minimum of once a month.

The overall testing frequency will be performed over the entire range of Contractor samples for asphalt binder content and gradation. The Engineer may select any or all split samples for assurance testing. The test results will be made available to the Contractor as soon as they become available.

The Engineer will notify the Contractor of observed deficiencies.

Differences between the Contractor's and the Engineer's split sample test results will be considered acceptable if within the following limits.

Test Parameter	Acceptable Limits of Precision	
	FRAP	RAS
% Passing: 1/		
1/2 in.	5.0%	
No. 4	5.0%	
No. 8	3.0%	4.0%
No. 30	2.0%	3.0%
No. 200	2.2%	2.5%
Asphalt Binder Content	0.3%	1.0%
Gmm	0.030	

1/ Based on washed extraction.

In the event comparisons are outside the above acceptable limits of precision, the Engineer will immediately investigate.

(d) Acceptance by the Engineer. Acceptable of the material will be based on the validation of the Contractor's quality control by the assurance process.

1031.05 Quality Designation of Aggregate in RAP and FRAP.

(a) RAP. The aggregate quality of the RAP for homogenous, conglomerate, and conglomerate "D" quality stockpiles shall be set by the lowest quality of coarse aggregate in the RAP stockpile and are designated as follows.

(1) RAP from Class I, Superpave/HMA (High ESAL), or (Low ESAL) IL-9.5L surface mixtures are designated as containing Class B quality coarse aggregate.

(2) RAP from Superpave/HMA (Low ESAL) IL-19.0L binder mixture is designated as Class D quality coarse aggregate.

(3) RAP from Class I, Superpave/HMA (High ESAL) binder mixtures, bituminous base course mixtures, and bituminous base course widening mixtures are designated as containing Class C quality coarse aggregate.

(4) RAP from bituminous stabilized subbase and BAM shoulders are designated as containing Class D quality coarse aggregate.

(b) FRAP. If the Engineer has documentation of the quality of the FRAP aggregate, the Contractor shall use the assigned quality provided by the Engineer.

If the quality is not known, the quality shall be determined as follows. Fractionated RAP stockpiles containing plus #4 (4.75 mm) sieve coarse aggregate shall have a maximum tonnage of 5,000 tons (4,500 metric tons). The Contractor shall obtain a representative

sample witnessed by the Engineer. The sample shall be a minimum of 50 lb (25 kg). The sample shall be extracted according to Illinois Modified AASHTO T 164 by a consultant prequalified by the Department for the specified testing. The consultant shall submit the test results along with the recovered aggregate to the District Office. The cost for this testing shall be paid by the Contractor. The District will forward the sample to the BMPR Aggregate Lab for MicroDeval Testing, according to Illinois Modified AASHTO T 327. A maximum loss of 15.0 percent will be applied for all HMA applications. The fine aggregate portion of the fractionated RAP shall not be used in any HMA mixtures that require a minimum of "B" quality aggregate or better, until the coarse aggregate fraction has been determined to be acceptable thru a MicroDeval Testing.

1031.06 Use of FRAP and/or RAS in HMA. The use of FRAP and/or RAS shall be a Contractor's option when constructing HMA in all contracts. (a) FRAP. The use of FRAP in HMA shall be as follows.

(1) Coarse Aggregate Size (after extraction). The coarse aggregate in all FRAP shall be equal to or less than the nominal maximum size requirement for the HMA mixture to be produced.

(2) Steel Slag Stockpiles. FRAP stockpiles containing steel slag or other expansive material, as determined by the Department, shall be homogeneous and will be approved for use in HMA (High ESAL and Low ESAL) mixtures regardless of lift or mix type.

(3) Use in HMA Surface Mixtures (High and Low ESAL). FRAP stockpiles for use in HMA surface mixtures (High and Low ESAL) shall have coarse aggregate that is Class B quality or better. FRAP shall be considered equivalent to limestone for frictional considerations unless produced/screened to minus 3/8 inch.

(4) Use in HMA Binder Mixtures (High and Low ESAL), HMA Base Course, and HMA Base Course Widening. FRAP stockpiles for use in HMA binder mixtures (High and Low ESAL), HMA base course, and HMA base course widening shall be FRAP in which the coarse aggregate is Class C quality or better.

(5) Use in Shoulders and Subbase. FRAP stockpiles for use in HMA shoulders and stabilized subbase (HMA) shall be FRAP, Restricted FRAP, conglomerate, or conglomerate DQ.

(b) RAS. RAS meeting Type 1 or Type 2 requirements will be permitted in all HMA applications as specified herein.

(c) FRAP and/or RAS Usage Limits. Type 1 or Type 2 RAS may be used alone or in conjunction with FRAP in HMA mixtures up to a maximum of 5.0% by weight of the total mix.

When FRAP is used alone or FRAP is used in conjunction with RAS, the percent of virgin asphalt binder replacement (ABR) shall not exceed the amounts indicated in the table below for a given N Design.

Max Asphalt Binder Replacement for FRAP with RAS Combination

HMA Mixtures ^{1/ 2/ 4/}	Maximum % ABR		
Ndesign	Binder/Leveling Binder	Surface	Polymer Modified ^{3/}
30L	50	40	30
50	40	35	30
70	40	30	30
90	40	30	30
4.75 mm N-50			40
SMA N-80			30

1/ For HMA “All Other” (shoulder and stabilized subbase) N-30, the percent asphalt binder replacement shall not exceed 50% of the total asphalt binder in the mixture.

2/ When the binder replacement exceeds 15 percent for all mixes, except for SMA and IL-4.75, the high and low virgin asphalt binder grades shall each be reduced by one grade (i.e. 25 percent binder replacement using a virgin asphalt binder grade of PG64-22 will be reduced to a PG58-28). When constructing full depth HMA and the ABR is less than 15 percent, the required virgin asphalt binder grade shall be PG64-28.

3/ When the ABR for SMA or IL-4.75 is 15 percent or less, the required virgin asphalt binder shall be SBS PG76-22 and the elastic recovery shall be a minimum of 80. When the ABR for SMA or IL-4.75 exceeds 15%, the virgin asphalt binder grade shall be SBS PG70-28 and the elastic recovery shall be a minimum of 80.

4/ When FRAP or RAS is used alone, the maximum percent asphalt binder replacement designated on the table shall be reduced by 10 percent.

1031.07 HMA Mix Designs. At the Contractor’s option, HMA mixtures may be constructed utilizing RAP/FRAP and/or RAS material meeting the detailed requirements specified herein.

(a) FRAP and/or RAS. FRAP and /or RAS mix designs shall be submitted for verification. If additional FRAP or RAS stockpiles are tested and found to be within tolerance, as defined under “Evaluation of Tests” herein, and meet all requirements herein, the additional FRAP or RAS stockpiles may be used in the original design at the percent previously verified.

(b) RAS. Type 1 and Type 2 RAS are not interchangeable in a mix design. A RAS stone bulk specific gravity (Gsb) of 2.300 shall be used for mix design purposes.

1031.08 HMA Production. HMA production utilizing FRAP and/or RAS shall be as follows. To remove or reduce agglomerated material, a scalping screen, gator, crushing unit, or comparable sizing device approved by the Engineer shall be used in the RAS and FRAP feed system to remove or reduce oversized material. If material passing the sizing device adversely affects the mix production or quality of the mix, the sizing device shall be set at a size specified by the Engineer.

If during mix production, corrective actions fail to maintain FRAP, RAS or QC/QA test results within control tolerances or the requirements listed herein the Contractor shall cease production of the mixture containing FRAP or RAS and conduct an investigation that may require a new mix design.

(a) RAS. RAS shall be incorporated into the HMA mixture either by a separate weight depletion system or by using the RAP weigh belt. Either feed system shall be interlocked with the aggregate feed or weigh system to maintain correct proportions for all rates of production and batch sizes. The portion of RAS shall be controlled accurately to within ± 0.5 percent of the amount of RAS utilized. When using the weight depletion system, flow indicators or sensing devices shall be provided and interlocked with the plant controls such that the mixture production is halted when RAS flow is interrupted.

(b) HMA Plant Requirements. HMA plants utilizing FRAP and/or RAS shall be capable of automatically recording and printing the following information.

(1) Dryer Drum Plants.

a. Date, month, year, and time to the nearest minute for each print. b. HMA mix number assigned by the Department.

c. Accumulated weight of dry aggregate (combined or individual) in tons (metric tons) to the nearest 0.1 ton (0.1 metric ton).

d. Accumulated dry weight of RAS and FRAP in tons (metric tons) to the nearest 0.1 ton (0.1 metric ton).

e. Accumulated mineral filler in revolutions, tons (metric tons), etc. to the nearest 0.1 unit.

f. Accumulated asphalt binder in gallons (liters), tons (metric tons), etc. to the nearest 0.1 unit.

- g. Residual asphalt binder in the RAS and FRAP material as a percent of the total mix to the nearest 0.1 percent.
- h. Aggregate RAS and FRAP moisture compensators in percent as set on the control panel. (Required when accumulated or individual aggregate and RAS and FRAP are printed in wet condition.)
- i. When producing mixtures with FRAP and/or RAS, a positive dust control system shall be utilized.
- j. Accumulated mixture tonnage.
- k. Dust Removed (accumulated to the nearest 0.1 ton) (2) Batch Plants.
 - a. Date, month, year, and time to the nearest minute for each print.
 - b. HMA mix number assigned by the Department.
 - c. Individual virgin aggregate hot bin batch weights to the nearest pound (kilogram).
 - d. Mineral filler weight to the nearest pound (kilogram).
 - f. RAS and FRAP weight to the nearest pound (kilogram).
 - g. Virgin asphalt binder weight to the nearest pound (kilogram).
 - h. Residual asphalt binder in the RAS and FRAP material as a percent of the total mix to the nearest 0.1 percent.

The printouts shall be maintained in a file at the plant for a minimum of one year or as directed by the Engineer and shall be made available upon request. The printing system will be inspected by the Engineer prior to production and verified at the beginning of each construction season thereafter.

1031.09 RAP in Aggregate Surface Course and Aggregate Shoulders. The use of RAP or FRAP in aggregate surface course and aggregate shoulders shall be as follows.

(a) Stockpiles and Testing. RAP stockpiles may be any of those listed in Article 1031.02, except “Non-Quality” and “FRAP”. The testing requirements of Article 1031.03 shall not apply. RAP used to construct aggregate surface course and aggregate shoulders shall be according to the current Bureau of Materials and Physical Research’s Policy Memorandum, “Reclaimed Asphalt Pavement (RAP) for Aggregate Applications”

(c) Gradation. The RAP material shall meet the gradation requirements for CA 6 in accordance with Art.1004.01 (c), except the requirements for the minus No. 200 (75µm) sieve will not apply. The sample for the RAP material shall be air dried to constant weight prior to being tested for gradation.”

STONE MATRIX ASPHALT (SMA) (DIST 1)

Effective: April 1, 1997

Revised: November 1, 2013

Description. This Special Provision establishes and describes the responsibilities of the Contractor in producing and constructing Polymerized Hot Mix Asphalt Binder Course, Stone Matrix Asphalt, N 80, or Polymerized Hot Mix Asphalt Surface Course, Stone Matrix Asphalt, N 80. The work shall be according to Sections 1003, 1004, 1011, 1102, 406, 1030, and 1032 except as modified herein.

Revise the last sentence of the first paragraph of Article 1003.03 (a):

“Fine aggregate for SMA shall consist of Class B Quality stone sand.”

Revise the following note in Article 1004.03 (c) to read:

“3/ The coarse aggregate gradation(s) used shall be capable of being combined with FA 20 stone sand and mineral filler to meet the approved mix design and the mix requirements noted herein.”

Revise Article 1004.03 (e) to read:

“Absorption. For SMA the coarse aggregate shall also have water absorption ≤ 2.0 .”

Add the following to Article 1011.01 (c):

“Mineral filler shall be commercially manufactured mineral filler meeting gradation requirements of this article and the following additional requirement: Collected HMA baghouse dust may be used as Mineral Filler provided it meets the gradation outlined in this article and a separate mix design is created.”

Delete last sentence of second paragraph of Article 1102.01(a) (13) a.

Revise Article (h) to read:

“(h) Fiber Additive (Note 4)”

Add the following to Note 4 in Article 1030.02:

“Reclaimed Asphalt Shingles (RAS) may be used in Stone Matrix Asphalt (SMA) mixtures designed with an SBA polymer modifier as a fiber additive if the mix design with RAS included meets AASHTO T 305 requirements. The RAS shall be from a certified source that produces either Type 1 or Type 2. Material shall be in accordance with the District’s special provision for Reclaimed Asphalt Pavement and Reclaimed Asphalt Shingles (D-1).

The actual dosage rate will be determined by the Engineer.”

Revise the following note under Table 1 in Article 1032.05 (b) of Standard Specification:

“When SBS/SBR PG 76-22 or SBS/SBR PG 76-28 is specified for mixtures IL-4.75 or SMA, the elastic recovery shall be a minimum of 80.”

Mix Design.

The use of Reclaimed Asphalt Pavement (RAP) and use of Recycled Asphalt Shingles (RAS) will be permitted concurrently in the production of SMA mixtures. Material shall be in accordance with Special Provision for Recycled Asphalt Pavement & Recycled Asphalt Shingles (D-1).

The drain down shall be determined at the JMF Asphalt Binder content at the mixing temperature plus 30 °F.

Each specific SMA mixture design shall be submitted to and verified by the Department as detailed in the Department’s current “Bituminous Mixture Design Verification Procedure”. The Contractor shall submit samples of all appropriate materials to the Department at least six weeks prior to production for mixture design verification.

The polymer asphalt supplier shall provide the Contractor with the temperature viscosity curves.

The Contractor shall supply the average gradation and the gradation ranges (including the Master Band on the critical sieve, if required) for each aggregate and asphalt binder content and gradations for each recycled products designated for use in the mixture. This information shall be used to judge whether the aggregates are compatible to produce an acceptable mix.

Plant Requirements.

Asphalt Cement. The polymer modified asphalt cement shall be shipped, maintained and stored at the mix plant according to the manufacturer’s requirements. Polymer asphalt cement shall be placed in an empty tank and not blended with other asphalt cements.

Replace second paragraph in Article 1102.01 (a) (13) a.:

“Only dust collected during the production of SMA may be returned to the SMA mixture. Any additional minus No. 200 (0.075 mm) material needed to produce the SMA shall be mineral filler meeting the requirements stated herein. As an option, collected bag-house dust may be used in lieu of manufactured mineral filler, provided; 1) there is enough available for the production of the SMA mix for the entire project and 2) a mix design was prepared with collected bag-house dust.”

Add the following to Article 1102.01 (a) (13) b. 1.:

“The batch size shall not exceed 75 percent of pugmill size as rated by the Department.

The fibers are to be uniformly distributed prior to the injection of asphalt cement into the mixes.”

Add the following to Article 1102.01(a) (13) b. 2.:

“The fiber shall be added to the aggregate and uniformly dispersed prior to the injection of asphalt cement

Fiber Supply System: The fiber system shall automatically adjust the feed rate to maintain the material within this tolerance at all times.”

Replace the following in Article 1102.014 (a) (13):

“c. Hot-Mix Storage. The mixture shall not be stored more than four hours without the approval of the Engineer. The Engineer will assess the drain down of the mix in making this determination.”

Mix Production.

The mixtures shall be produced at a temperature range recommended by the polymer asphalt supplier and approved by the Engineer to allow adequate compaction. The actual production temperature will be selected from the range by the Engineer based on individual plant characteristics and modifier used in the mixtures.

A manufacturer's representative from the polymer asphalt cement producer shall be present during each polymer mixture start-up and shall be available at all times during production and lay-down of the mix. A manufacturer's representative for the supplier/manufacture of the fibers and the equipment to introduce fibers into the mixture shall be present for calibration and first day of production (test strip).

Hauling/Laydown Equipment.

The Contractor shall provide a release agent that minimizes sticking to equipment and is acceptable to the Engineer. The Contractor shall furnish a laborer to ensure that all truck beds are clean and no excess release agent is used prior to being loaded. All trucks shall be insulated and tarped when hauling the mixture to the paver.

Add after second sentence of Table 1 Note 5 in Article 406.07 (a) the following:

“Except one of the T_B shall be 84 inches (2.14 m) wide and a weight of 315 pound per linear inch (PLI) (5.63 kg/mm).”

Additional breakdown rollers will be required if compaction is not achieved using the speeds listed in Article 406.07.

Mix Placement.

Add the following to Article 406.06 (b):

“(3) Special Conditions for SMA

- a. SMA mixture shall be placed on a dry surface when the temperature of the roadbed is above 60 °F (15 °C).

- b. The mixture shall be placed at 325 °F (152 °C) or a minimum mixture temperature recommended by the polymer asphalt supplier and approved by the Engineer. The mixture temperature shall be measured in the truck just prior to placement in the paver.”

Add to the end of the third paragraph of Article 406.06 (e) the following:

“For SMA mixtures, the paver speed shall not exceed 25 ft/min (8.8 m/min) during placement, provided the pavement shows no signs of rippling, chatter, or other distresses.”

Compaction shall commence immediately after the mixture has been placed. Compaction shall continue until the required density range has been achieved. Care shall be taken to avoid excessive aggregate breakage.

A QC/QA mixture Test Strip will be required. The Test Strip shall be constructed at a location approved by the Engineer to determine the mix properties, density, and laydown characteristics. An offsite test strip will be required if WMA technology is used. These test results and visual inspections on the mixture shall be used to make corrective adjustments if necessary.

Prior to the start of mix production and placement, the Engineer will review and approve all test strip results and rolling pattern. The test strip shall be constructed by the standard method except for the following changes:

- (a) The Test Strip shall consist of approximately 400 tons (375 metric tons). It shall contain two growth curves which shall be compacted by a static steel-wheeled roller
 - (1) Compaction Temperature. In order to make an accurate analysis of the density potential of the HMA mixture, the temperature of the mixture on the pavement at the beginning of the growth curve shall be 325 °F (152 °C).
 - (2) Compaction and Testing. The Engineer will specify the roller(s) speed and number of passes required to obtain a completed growth curve. The nuclear gauge shall be placed near the center of the hot mat and the position marked for future reference. With the bottom of the nuclear gauge and the source rod clean, a 15 seconds nuclear reading (without mineral filler) shall be taken after each pass of the roller. Rolling shall continue until the maximum density is achieved and three consecutive passes show no appreciable increase in density or no evidence of destruction of the mat. The growth curve shall be plotted. No testing of initial passes shall be taken until the fourth pass is completed.
 - (3) Final Testing. After the growth curve information is obtained, a final one minute nuclear reading, using mineral filler to eliminate surface voids, shall be taken at the marked position. This reading is used to adjust the maximum density reading obtained during the growth curve.
- (b) Documentation. The Test Strip and rolling pattern information (including growth curves) will be tabulated by the contractor and the original report submitted to the Engineer. Any change to the rolling pattern shall be approved by the Engineer.

The density of the finished SMA binder course shall be measured either by nuclear test methods or from cores obtained by the contractor at random locations. For the SMA surface course mixes containing steel slag aggregate only the core method will be accepted.

If the nuclear density potential of the mixture does not exceed 91.0 percent, the operation will cease until all test data is analyzed or a new mix design is produced.

In addition, other aspects of the mixture, such as appearance, segregation, uneven texture, flushing, or other evidence of mix problems, should be noted and corrective action taken immediately. The Engineer will determine the acceptability of the placed mixture. Unacceptable areas will be removed and replaced by the Contractor at no additional cost to the Department.

Control Charts/Limits.

Add and revise the following to Control Limits Table in Article 1030.05(d) (4) of the Standard Specifications:

“For SMA mixtures, Control charts/limits shall be according to QC/QA requirements except density shall be plotted on the control charts within the following control limits:

Control Limits	
SMA	
<u>Parameter</u>	<u>Individual Test</u>
Density	94 % - 97 %”

Basis of Payment.

Add the following to the end of Article 406.14:

“The plan quantities shall be adjusted using the actual approved binder and surface Mix Design’s G_{mb} .”

The test strip will be paid for at the contract unit price each for CONSTRUCTING TEST STRIP, which price shall not include the 400 tons (360 metric tons) of mix, as well as the appropriate testing, which will be paid for at the unit price in the contract for the item being placed.

TEMPORARY PAVEMENT

Effective: March 1, 2003

Revised: April 10, 2008

Description. This work shall consist of constructing a temporary pavement at the locations shown on the plans or as directed by the engineer.

The contractor shall use either Portland cement concrete according to Sections 353 and 354 of the Standard Specifications or HMA according to Sections 355, 356, 406 of the Standard Specifications, and other applicable HMA special provisions as contained herein. The HMA mixtures to be used shall be specified in the plans. The thickness of the Temporary Pavement shall be as described in the plans.

The contractor shall have the option of constructing either material type if both Portland cement concrete and HMA are shown in the plans.

Articles 355.08 and 406.11 of the Standard Specifications shall not apply.

The removal of the Temporary Pavement, if required, shall conform to Section 440 of the Standard Specification.

Method of Measurement. Temporary pavement will be measured in place and the area computed in square yards (square meters).

Basis of Payment. This work will be paid for at the contract unit price per square yard (square meter) for TEMPORARY PAVEMENT and TEMPORARY PAVEMENT (INTERSTATE).

Removal of temporary pavement will be paid for at the contract unit price per square yard (square meter) for PAVEMENT REMOVAL.

WINTERIZED TEMPORARY ACCESS

Effective: January 1, 2012

Revised: March 5, 2012

Description. This work shall consist of constructing, maintaining and removing winterized temporary access for private and commercial entrances and side roads designed for use throughout the winter months.

Materials. Materials shall be according to the following.

ITEM	ARTICLE/SECTION
Hot-Mix Asphalt	1030

Construction Requirements

For projects lasting longer than one construction season, the contractor shall construct and maintain temporary access composed of an HMA surface course over an existing aggregate temporary access. The contractor shall install the winterized temporary access prior to winter shut down at the direction of the engineer. The top 2" of the existing aggregate temporary access should be removed and replaced with 2" of Hot-Mix Asphalt. Compensation will be given for the winterized temporary access at the time of the installation of the Hot-Mix Asphalt surface course.

HMA Surface Course. The Hot-Mix Asphalt surface course shall be 2 in. thick when compacted. HMA Surface Course, Mix "D", N50 shall be used except as modified by the plans or as directed by the Engineer. This work shall be constructed in accordance with the applicable portions of Section 406 of the Standard Specifications and as directed by the Engineer. The material shall conform to the applicable portions of Section 1030 of the Standard Specifications.

The winterized temporary access shall be constructed to the dimensions and grades of the existing aggregate temporary access.

Maintaining the winterized temporary access shall include repairing the HMA surface course after any operation that may disturb or remove the winterized temporary access to the satisfaction of the Engineer.

When use of the winterized temporary access is discontinued, the winterized temporary access shall be removed according to Article 440.03 of the Standard Specifications. The material shall be disposed of according to Article 202.03 of the Standard Specifications or may be utilized in the permanent construction with the approval of the Engineer.

Method of Measurement. Winterized temporary access for private and commercial entrances and roads will be measured for payment at the contract unit price per square yard for every private entrance, commercial entrance or road constructed for the purpose of winterized temporary access.

Basis of Payment. Winterized temporary access for private and commercial entrances and roads will be paid for at the contract unit price per square yard for TEMPORARY ACCESS (WINTERIZE) as specified in the plans.

Partial payment of the square yard amount bid for each winterized temporary access will be paid according to the following schedule:

(a) Upon construction of the winterized temporary access, sixty percent of the contract unit price per square yard will be paid.

(b) Subject to the approval of the Engineer for the adequate maintenance and removal of the winterized temporary access, the remaining forty percent of the pay item will be paid upon the permanent removal of the temporary access.

ELECTRIC SERVICE DISCONNECT, LIGHTING AND TRAFFIC SIGNAL

Effective: January 1, 2012

Description:

This item shall consist of furnishing and installing for the Lighting and Traffic Signal System a service disconnect box, 2 or 3 wire mounted on a wood pole as specified below, and as shown on the detail drawings and as directed by the Engineer.

Materials:

The disconnect box shall be NEMA 4X stainless steel, nominally 12" W x 16" H x 8" D with piano hinged door, steel back panel, fast acting stainless steel enclosure clamps, padlock provisions and door stop kit (Hoffman catalog #A-16H1208SS6LP/A-16P12/A-DSTOPK/C-PMK12, or approved equal).

Circuit Breakers shall be thermal magnetic bolt-on type with a minimum interrupt capacity of 25,000 symmetrical amperes at 240 volts. Breakers shall be lockable in the off position for lockout/tag-out compliance.

Bus bars, connectors, and lugs shall be copper, insulated and isolated, and configured to prevent shorted conditions from tightening terminations. Lugs and connectors shall be rated for 75°C. Overall bus

sections shall be configured behind an insulating barrier shield which is removable for access to connections. The circuit breakers and bus may be part of an approved panel board assembly.

Disconnect surge protector shall be suitable for 240/120 volt single phase 60Hz, AC electrical service. Protector shall have a surge energy capability of 2160 joules or better at 8/20 microseconds, rate -40 to 60°C., with LED operating indicators and shall be UL listed per UL 1449. The surge protector shall be a Cutler Hammer CMOV230L065XST or approved equal.

Conduit, wire, and ground rods to complete the installation of the disconnect box shall be included as part of this item, as required and as indicated.

Combination ground and neutral bar shall be configured with separate ground and neutral sections and spare terminals as indicated. The heads of grounding screws shall be painted green. The heads of neutral screws shall be painted white.

A plastic laminated layout and circuit diagram shall be affixed to the interior side of the enclosure door.

A 2-color engraved plastic nameplate, attached with screws and engraved as indicated, shall be provided for each main breaker.

The exact mounting height for the Electric Service Disconnect shall be field determined and marked by the Engineer.

Electrical service shall be of the voltage indicated. Where 120 volt service is indicated, service drop cable shall be installed accordingly and lighting main breaker and all other service appurtenances shall be included regardless of the service voltage applied to the installation.

The electric service equipment assembly shall be UL labeled, suitable for use as service equipment.

Steel strut channel shall be provided for proper installation of the disconnect, as shown on the disconnect mounting detail.

Electric Utility charges will be paid separately and are not part of this item.

Installation:

The Electric Service Disconnect shall be installed as indicated in the Electric Service Disconnect detail. All work shall be fully coordinated with the electric utility company by the Contractor.

Method Of Measurement:

Each Electric Service Disconnect, installed complete as specified and as indicated on the plans, shall be counted each for payment.

Basis Of Payment:

This item shall be paid for at the contract unit price, each, for ELECTRIC SERVICE DISCONNECT, LIGHTING AND TRAFFIC SIGNAL, which shall be payment in full for the work.

GENERAL ELECTRICAL REQUIREMENTS

Effective: January 1, 2012

Add the following to Article 801 of the Standard Specifications:

“Maintenance transfer and Preconstruction Inspection:

General. Before performing any excavation, removal, or installation work (electrical or otherwise) at the site, the Contractor shall request a maintenance transfer and preconstruction site inspection, to be held in the presence of the Engineer and a representative of the party or parties responsible for maintenance of any lighting and/or traffic control systems which may be affected by the work. The request for the maintenance transfer and preconstruction inspection shall be made no less than seven (7) calendar days prior to the desired inspection date. The maintenance transfer and preconstruction inspection shall:

Establish the procedures for formal transfer of maintenance responsibility required for the construction period.

Establish the approximate location and operating condition of lighting and/or traffic control systems which may be affected by the work

Marking of Existing Cable Systems. The party responsible for maintenance of any existing lighting and/or traffic control systems at the project site will, at the Contractor's request, mark and/or stake, once per location, all underground cable routes owned or maintained by the State. A project may involve multiple "locations" where separated electrical systems are involved (i.e. different controllers). The markings shall be taken to have a horizontal tolerance of at least 304.8 mm (one (1) foot) to either side.. The request for the cable locations and marking shall be made at the same time the request for the maintenance transfer and preconstruction inspection is made. The Contractor shall exercise extreme caution where existing buried cable runs are involved. The markings of existing systems are made strictly for assistance to the Contractor and this does not relieve the Contractor of responsibility for the repair or replacement of any cable run damaged in the course of his work, as specified elsewhere herein. Note that the contractor shall be entitled to only one request for location marking of existing systems and that multiple requests may only be honored at the contractor's expense. No locates will be made after maintenance is transferred, unless it is at the contractor's expense.

Condition of Existing Systems. The Contractor shall conduct an inventory of all existing electrical system equipment within the project limits, which may be affected by the work, making note of any parts which are found broken or missing, defective or malfunctioning. Megger and load readings shall be taken for all existing circuits which will remain in place or be modified. If a circuit is to be taken out in its entirety, then readings do not have to be taken. The inventory and test data shall be reviewed with and approved by the Engineer and a record of the inventory shall be submitted to the Engineer for the record. Without such a record, all systems transferred to the Contractor for maintenance during construction shall be returned at the end of construction in complete, fully operating condition.”

Add the following to the 1st paragraph of Article 801.05(a) of the Standard Specifications:

“Items from multiple disciplines shall not be combined on a single submittal and transmittal. Items for lighting, signals, surveillance and CCTV must be in separate submittals since they may be reviewed by various personnel in various locations.”

Revise the second sentence of the 5th paragraph of Article 801.05(a) of the Standard Specifications to read:

“The Engineer will stamp the submittals indicating their status as ‘Approved’, ‘Approved as Noted’, ‘Disapproved’, or ‘Information Only’.

Revise the 6th paragraph of Article 801.05(a) of the Standard Specifications to read:

“Resubmittals. All submitted items reviewed and marked ‘Approved as Noted’, or ‘Disapproved’ are to be resubmitted in their entirety with a disposition of previous comments to verify contract compliance at no additional cost to the state unless otherwise indicated within the submittal comments.”

Revise Article 801.11(a) of the Standard Specifications to read:

“Lighting Operation and Maintenance Responsibility. The scope of work shall include the assumption of responsibility for the continuing operation and maintenance the of existing, proposed, temporary, sign and navigation lighting, or other lighting systems and all appurtenances affected by the work as specified elsewhere herein. Maintenance of lighting systems is specified elsewhere and will be paid for separately

Energy and Demand Charges. The payment of basic energy and demand charges by the electric utility for existing lighting which remains in service will continue as a responsibility of the Owner, unless otherwise indicated. Unless otherwise indicated or required by the Engineer duplicate lighting systems (such as temporary lighting and proposed new lighting) shall not be operated simultaneously at the Owner's expense and lighting systems shall not be kept in operation during long daytime periods at the Owner's expense. Upon written authorization from the Engineer to place a proposed new lighting system in service, whether the system has passed final acceptance or not, (such as to allow temporary lighting to be removed), the Owner will accept responsibility for energy and demand charges for such lighting, effective the date of authorization. All other energy and demand payments to the utility shall be the responsibility of the Contractor until final acceptance.”

Add the following to Section 801 of the Standard Specifications:

“Lighting Cable Identification. Each wire installed shall be identified with its complete circuit number at each termination, splice, junction box or other location where the wire is accessible.”

“Lighting Cable Fuse Installation. Standard fuse holders shall be used on non-frangible (non-breakaway) light pole installations and quick-disconnect fuse holders shall be used on frangible (breakaway) light pole installations. Wires shall be carefully stripped only as far as needed for connection to the device. Over-stripping shall be avoided. An oxide inhibiting lubricant shall be applied to the wire for minimum connection resistance before the terminals are crimped-on. Crimping

shall be performed in accordance with the fuse holder manufacturer's recommendations. The exposed metal connecting portion of the assembly shall be taped with two half-lapped wraps of electrical tape and then covered by the specified insulating boot. The fuse holder shall be installed such that the fuse side is connected to the pole wire (load side) and the receptacle side of the holder is connected to the line side."

Revise the 2nd paragraph of Article 801.16 of the Standard Specifications to read:

"When the work is complete, and seven days before the request for a final inspection, the full-size set of contract drawings. Stamped "RECORD DRAWINGS", shall be submitted to the Engineer for review and approval and shall be stamped with the date and the signature of the Contractor's supervising Engineer or electrician. The record drawings shall be submitted in PDF format on CDROM as well as hardcopy for review and approval. In addition to the record drawings, copies of the final catalog cuts which have been Approved or Approved as Noted shall be submitted in PDF format along with the record drawings. The PDF files shall clearly indicate either by filename or PDF table of contents the respective pay item number. Specific part or model numbers of items which have been selected shall be clearly visible."

Add the following to Article 801.16 of the Standard Specifications:

"In addition to the specified record drawings, the Contactor shall record GPS coordinates of the following electrical components being installed, modified or being affected in other ways by this contract:

- Last light pole on each circuit
- Handholes
- Conduit roadway crossings
- Controllers
- Control Buildings
- Structures with electrical connections, i.e. DMS, lighted signs.
- Electric Service locations
- CCTV Camera installations
- Fiber Optic Splice Locations

Datum to be used shall be North American 1983.

Data shall be provided electronically and in print form. The electronic format shall be compatible with MS Excel. Latitude and Longitude shall be in decimal degrees with a minimum of 6 decimal places. Each coordinate shall have the following information:

1. Description of item
2. Designation or approximate station if the item is undesignated
3. Latitude
4. Longitude

Examples:

Equipment	Equipment Designation	Latitude	Longitude
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Description			
CCTV Camera pole	ST42	41.580493	-87.793378
FO mainline splice handhole	HHL-ST31	41.558532	-87.792571
Handhole	HH at STA 234+35	41.765532	-87.543571
Electric Service	Elec Srv	41.602248	-87.794053
Conduit crossing	SB IL83 to EB I290 ramp SIDE A	41.584593	-87.793378
Conduit crossing	SB IL83 to EB I290 ramp SIDE B	41.584600	-87.793432
Light Pole	DA03	41.558532	-87.792571
Lighting Controller	X	41.651848	-87.762053
Sign Structure	FGD	41.580493	-87.793378
Video Collection Point	VCP-IK	41.558532	-87.789771
Fiber splice connection	Toll Plaza34	41.606928	-87.794053

Prior to the collection of data, the contractor shall provide a sample data collection of at least six data points of known locations to be reviewed and verified by the Engineer to be accurate within 100 feet. Upon verification, data collection can begin. Data collection can be made as construction progresses, or can be collected after all items are installed. If the data is unacceptable the contractor shall make corrections to the data collection equipment and or process and submit the data for review and approval as specified.

Accuracy. Data collected is to be mapping grade. A handheld mapping grade GPS device shall be used for the data collection. The receiver shall support differential correction and data shall have a minimum 5 meter accuracy after post processing.

GPS receivers integrated into cellular communication devices, recreational and automotive GPS devices are not acceptable.

The GPS shall be the product of an established major GPS manufacturer having been in the business for a minimum of 6 years.”

EXPOSED RACEWAYS

Effective: January 1, 2012

Revise the first paragraph of Article 811.03(a) of the Standard Specifications to read:

“General. Rigid metal conduit installation shall be according to Article 810.05(a). Conduits terminating in junction and pull boxes shall be terminated with insulated and gasketed watertight threaded NEMA 4X conduit hubs. The hubs shall be Listed under UL 514B. The insulated throat shall be rated up to 105° C. When PVC coated conduit is utilized, the aforementioned hubs shall also be PVC coated.”

Add the following to Article 811.03(b) of the Standard Specifications:

“Where PVC coated conduit is utilized, all conduit fittings, couplings and clamps shall be PVC coated. All other mounting hardware and appurtenances shall be stainless steel.”

“The personnel installing the PVC coated conduit must be trained and certified by the PVC coated conduit Manufacturer or Manufacturer’s representative to install PVC coated conduit. Documentation demonstrating this requirement must be submitted for review and approval.”

Add the following to Article 1088.01(a) of the Standard Specifications:

All iron and steel products, which are to be incorporated into the work, including conduit and all conduit fittings, shall be domestically manufactured or produced and fabricated as specified in Article 106.”

Revise Article 1088.01(a)(3) of the Standard Specifications to read:

- “a. PVC Coated Steel Conduit. The PVC coated rigid metal conduit shall be UL Listed (UL 6). The PVC coating must have been investigated by UL as providing the primary corrosion protection for the rigid metal conduit. Ferrous fittings for general service locations shall be UL Listed with PVC as the primary corrosion protection. Hazardous location fittings, prior to plastic coating shall be UL listed.

- b. The PVC coating shall have the following characteristics:

Hardness:	85+ Shore A Durometer
Dielectric Strength:	400V/mil @ 60 Hz
Aging:	1,000 Hours Atlas Weatherometer
Temperature	The PVC compound shall conform at 0° F. to Federal Specifications PL-406b, Method 2051, Amendment 1 of 25 September 1952 (ASTM D 746)
Elongation:	200%

- c. The exterior and interior galvanized conduit surface shall be chemically treated to enhance PVC coating adhesion and shall also be coated with a primer before the PVC coating to ensure a bond between the zinc substrate and the PVC coating. The bond strength created shall be greater than the tensile strength of the plastic coating.
- d. The nominal thickness of the PVC coating shall be 1 mm (40 mils). The PVC exterior and urethane interior coatings applied to the conduit shall afford sufficient flexibility to permit field bending without cracking or flaking at temperatures above -1°C (30°F).
- e. An interior urethane coating shall be uniformly and consistently applied to the interior of all conduit and fittings. This internal coating shall be a nominal 2 mil thickness. The interior coating shall be applied in a manner so there are no runs, drips, or pinholes at any point. The coating shall not peel, flake, or chip off after a cut is made in the conduit or a scratch is made in the coating.

- f. Conduit bodies shall have a tongue-in-groove gasket for maximum sealing capability. The design shall incorporate a positive placement feature to assure proper installation. Certified test results confirming seal performance at 15 psig (positive) and 25 in. of mercury (vacuum) for 72 hours shall be submitted for review when requested by the Engineer.
- g. The PVC conduit shall pass the following tests:

Exterior PVC Bond test RN1:

Two parallel cuts 13 mm (1/2 inch) apart and 40 mm (1 1/2 inches) in length shall be made with a sharp knife along the longitudinal axis. A third cut shall be made perpendicular to and crossing the longitudinal cuts at one end. The knife shall then be worked under the PVC coating for 13 mm (1/2 inch) to free the coating from the metal.

Using pliers, the freed PVC tab shall be pulled with a force applied vertically and away from the conduit. The PVC tab shall tear rather than cause any additional PVC coating to separate from the substrate.

Boil Test:

Acceptable conduit coating bonds (exterior and interior) shall be confirmed if there is no disbondment after a minimum average of 200 hours in boiling water or exposure to steam vapor at one atmosphere. Certified test results from a national recognized independent testing laboratory shall be submitted for review and approval. The RN1 Bond Test and the Standard Method for Measuring Adhesion by Tape Test shall be utilized.

Exterior Adhesion. In accordance with ASTM D870, a 6" length of conduit test specimen shall be placed in boiling water. The specimen shall be periodically removed, cooled to ambient temperature and immediately tested according to the bond test (RN1). When the PVC coating separates from the substrate, the boil time to failure in hours shall be recorded.

Interior Adhesion. In accordance with ASTM D3359, a 6" conduit test specimen shall be cut in half longitudinally and placed in boiling water or directly above boiling water with the urethane surface facing down. The specimen shall be periodically removed, cooled to ambient temperature and tested in accordance with the Standard Method of Adhesion by Tape Test (ASTM D3359). When the coating disbonds, the time to failure in hours shall be recorded.

Heat/Humidity Test:

Acceptable conduit coating bonds shall be confirmed by a minimum average of 30 days in the Heat and Humidity Test. The RN1 Bond Test and the Standard Method for Measuring Adhesion by Tape Test shall be utilized.

Exterior Adhesion. In accordance with ASTM D1151, D1735, D2247 and D4585, conduit specimens shall be placed in a heat and humidity environment where the temperature is maintained at 150°F (66°C) and 95% relative humidity. The specimens shall be periodically removed and a bond test (RN1) performed. When the PVC coating separates from the substrate, the exposure time to failure in days shall be recorded.

Interior Adhesion. In accordance with ASTM D3359, conduit specimens shall be placed in a heat and humidity environment where the temperature is maintained at 150°F (66°C) and 95% relative humidity. When the coating disbonds, the time to failure in hours shall be recorded.

Add the following to Article 1088.01(a)(4) of the Standard Specifications:

“All liquid tight flexible metal conduit fittings shall have an insulated throat to prevent abrasion of the conductors and shall have a captive sealing O-ring gasket. The fittings shall be Listed under UL 514B. The insulated throat shall be rated up to 105° C.”

Revise the second paragraph of Article 811.04 of the Standard Specifications to read:

“Expansion fittings and LFNC will not be measured for payment.”

Revise Article 811.05 of the Standard Specifications to read:

“811.05 Basis of Payment. This work will be paid for at the contract unit price per meter (foot) for CONDUIT ATTACHED TO STRUCTURE, of the diameter specified, RIGID GALVANIZED STEEL or CONDUIT ATTACHED TO STRUCTURE, of the diameter specified, RIGID GALVANIZED STEEL, PVC COATED.”

UNDERGROUND RACEWAYS

Effective: January 1, 2012

Revise Article 810.04 of the Standard Specifications to read:

“Installation. All underground conduit shall have a minimum depth of 30-inches (700 mm) below the finished grade.”

Add the following to Article 810.04 of the Standard Specifications:

“All metal conduit installed underground shall be Rigid Steel Conduit unless otherwise indicated on the plans.”

Add the following to Article 810.04 of the Standard Specifications:

“All raceways which extend outside of a structure or duct bank but are not terminated in a cabinet, junction box, pull box, handhole, post, pole, or pedestal shall extend a minimum of 300 mm (12”) or the length shown on the plans beyond the structure or duct bank. The end of this extension shall be capped and sealed with a cap designed for the conduit to be capped. The ends of rigid metal conduit to be capped shall be threaded, the threads protected with full galvanizing, and capped with a threaded galvanized steel cap. The ends of rigid nonmetallic conduit and

coilable nonmetallic conduit shall be capped with a rigid PVC cap of not less than 3 mm (0.125") thick. The cap shall be sealed to the conduit using a room-temperature-vulcanizing (RTV) sealant compatible with the material of both the cap and the conduit. A washer or similar metal ring shall be glued to the inside center of the cap with epoxy, and the pull cord shall be tied to this ring."

Add the following to Article 810.04(c) of the Standard Specifications:

"Coilable non-metallic conduit shall be machine straightened to remove the longitudinal curvature caused by coiling the conduit onto reels prior to installing in trench, encasing in concrete or embedding in structure. The straightening shall not deform the cross-section of the conduit such that any two measured outside diameters, each from any location and at any orientation around the longitudinal axis along the conduit differ by more than 6 mm (0.25")." The longitudinal axis of the straightened conduit shall not deviate by more than 20 mm per meter (0.25" per foot" from a straight line. The HDPE and straightening mechanism manufacturer operating temperatures shall be followed.

ELECTRIC UTILITY SERVICE CONNECTION (COMED)

Effective: January 1, 2012

Description. This item shall consist of payment for work performed by ComEd in providing or modifying electric service as indicated. THIS MAY INVOLVE WORK AT MORE THAN ONE ELECTRIC SERVICE. For summary of the Electrical Service Drop Locations see the schedule contained elsewhere herein.

CONSTRUCTION REQUIREMENTS

General. It shall be the Contractor's responsibility to contact ComEd. The Contractor shall coordinate his work fully with the ComEd both as to the work required and the timing of the installation. No additional compensation will be granted under this or any other item for extra work caused by failure to meet this requirement. **Please contact ComEd, New Business Center Call Center, at 866 NEW ELECTRIC (1-866-639-3532) to begin the service connection process. The Call Center Representatives will create a work order for the service connection. The representative will ask the requestor for information specific to the request. The representative will assign the request based upon the location of project.**

The Contractor should make particular note of the need for the earliest attention to arrangements with ComEd for service. In the event of delay by ComEd, no extension of time will be considered applicable for the delay unless the Contractor can produce written evidence of a request for electric service within 30 days of execution.

Method of Payment. The Contractor will be reimbursed to the exact amount of money as billed by ComEd for its services. Work provided by the Contractor for electric service will be paid separately as described under ELECTRIC SERVICE INSTALLATION. No extra compensation shall be paid to the Contractor for any incidental materials and labor required to fulfill the requirements as shown on the plans and specified herein.

For bidding purposes, this item shall be estimated as \$22,500

Basis of Payment. This work will be paid for at the contract lump sum price for ELECTRIC UTILITY SERVICE CONNECTION which shall be reimbursement in full for electric utility service charges.

ELECTRIC SERVICE INSTALLATION

Effective: January 1, 2012

Description. This item shall consist of all material and labor required to extend, connect or modify the electric services, as indicated or specified, which is over and above the work performed by the utility. Unless otherwise indicated, the cost for the utility work, if any, will be reimbursed to the Contractor separately under ELECTRIC UTILITY SERVICE CONNECTION. This item may apply to the work at more than one service location and each will be paid separately.

Materials. Materials shall be in accordance with the Standard Specifications.

CONSTRUCTION REQUIREMENTS

General. The Contractor shall ascertain the work being provided by the electric utility and shall provide all additional material and work not included by other contract pay items required to complete the electric service work in complete compliance with the requirements of the utility.

No additional compensation will be allowed for work required for the electric service, even though not explicitly shown on the Drawings or specified herein

Method of Measurement. Electric Service Installation shall be counted, each.

Basis of Payment. This work will be paid for at the contract unit price each for ELECTRIC SERVICE INSTALLATION which shall be payment in full for the work specified herein.

WIRE AND CABLE

Effective: January 1, 2012

Add the following to the first paragraph of Article 1066.02(a):

“The cable shall be rated at a minimum of 90°C dry and 75°C wet and shall be suitable for installation in wet and dry locations, and shall be resistant to oils and chemicals.”

Revise the Aerial Electric Cable Properties table of Article 1066.03(a)(3) to read:

Aerial Electric Cable Properties

Phase Conductor			Messenger wire	
Size AWG	Stranding	Average Insulation	Minimum Size	Stranding

		Thickness		AWG	
		mm	mils		
6	7	1.1	(45)	6	6/1
4	7	1.1	(45)	4	6/1
2	7	1.1	(45)	2	6/1
1/0	19	1.5	(60)	1/0	6/1
2/0	19	1.5	(60)	2/0	6/1
3/0	19	1.5	(60)	3/0	6/1
4/0	19	1.5	(60)	4/0	6/1

Add the following to Article 1066.03(b) of the Standard Specifications:

“Cable sized No. 2 AWG and smaller shall be U.L. listed Type RHH/RHW and may be Type RHH/RHW/USE. Cable sized larger than No. 2 AWG shall be U.L. listed Type RHH/RHW/USE.”

Revise Article 1066.04 to read:

“Aerial Cable Assembly. The aerial cable shall be an assembly of insulated aluminum conductors according to Section 1066.02 and 1066.03. Unless otherwise indicated, the cable assembly shall be composed of three insulated conductors and a steel reinforced bare aluminum conductor (ACSR) to be used as the ground conductor. Unless otherwise indicated, the code word designation of this cable assembly is “Palomino”. The steel reinforced aluminum conductor shall conform to ASTM B-232. The cable shall be assembled according to ANSI/ICEA S-76-474.”

Revise the second paragraph of Article 1066.05 to read:

“The tape shall have reinforced metallic detection capabilities consisting of a woven reinforced polyethylene tape with a metallic core or backing.”

ELECTRIC CABLE IN CONDUIT NO. 20 3/C, TWISTED, SHIELDED

This work shall consist of furnishing and installing electric cable in conduit of the type, size, and number of conductors specified. The construction performed and materials used shall be in conformance with applicable parts of Sections 873 and 1076 of the Standard Specifications. Physical specifications are as follows:

Conductors: 3-20 awg (7/28) tinned copper .037”

Dielectric: 25 mil wall of color coded polyethylene to a nominal diameter of .191”

Cabling: Cable together three insulated conductors with an overall aluminum/polyester tape, the foil out, and a 20 awg (7/28) tinned copper drain, in a 3” left hand lay to a nominal diameter of .191”

Jacket: 45 mil wall of black moisture resistant sunlight resistant polyvinyl chloride to a nominal diameter of .281” +/- .010”

Basis of Payment. This work will be paid for at the contract unit price foot for ELECTRIC CABLE IN CONDUIT NO. 20 3/C, TWISTED, SHIELDED, which price shall include all labor, equipment and materials necessary to complete the work.

TYPE III TEMPORARY TAPE FOR WET CONDITIONS

Effective: February 1, 2007

Revised: February 1, 2011

Description. This work shall consist of furnishing, installing, and maintaining Type III Temporary Pavement Marking Tape for Wet Conditions.

Materials. Materials shall be according to the following.

Item	Article/Section
(a) Pavement Marking Tape	1095.06

Initial minimum reflectance values under dry and wet conditions shall be as specified in Article 1095.06. The marking tape shall maintain its reflective properties when submerged in water. The wet reflective properties will be verified by a visual inspection method performed by the Department. The surface of the material shall provide an average skid resistance of 45 BPN when tested according to ASTM E 303.

CONSTRUCTION REQUIREMENTS

Type III Temporary Tape for Wet Conditions shall meet the requirements of Article 703.03 and 703.05. Application shall follow manufacturer's recommendations.

Method of Measurement. This work will be measured for payment in place, in feet (meters).

Basis of Payment. This work will be paid for at the contract unit price per foot (meter) for WET REFLECTIVE TEMPORARY TAPE TYPE III of the line width specified, and at the contract unit price per square foot (square meter) for WET REFLECTIVE TEMPORARY TAPE TYPE III, LETTERS AND SYMBOLS.

TEMPORARY INFORMATION SIGNING

Effective: November 13, 1996

Revised: January 2, 2007

Description.

This work shall consist of furnishing, installing, maintaining, relocating for various states of construction and eventually removing temporary informational signs. Included in this item may be ground mount signs, skid mount signs, truss mount signs, bridge mount signs, and overlay sign panels which cover portions of existing signs.

Materials.

Materials shall be according to the following Articles of Section 1000 - Materials:

	<u>Item</u>	<u>Article/Section</u>
a.)	Sign Base (Notes 1 & 2)	1090
b.)	Sign Face (Note 3)	1091
c.)	Sign Legends	1092
d.)	Sign Supports	1093
e.)	Overlay Panels (Note 4)	1090.02

Note 1. The Contractor may use 5/8 inch (16 mm) instead of 3/4 inch (19 mm) thick plywood.

Note 2. Type A sheeting can be used on the plywood base.

Note 3. All sign faces shall be Type A except all orange signs shall meet the requirements of Article 1106.01.

Note 4. The overlay panels shall be 0.08 inch (2 mm) thick.

GENERAL CONSTRUCTION REQUIRMENTS

Installation.

The sign sizes and legend sizes shall be verified by the Contractor prior to fabrication.

Signs which are placed along the roadway and/or within the construction zone shall be installed according to the requirements of Article 701.14 and Article 720.04. The signs shall be 7 ft (2.1 m) above the near edge of the pavement and shall be a minimum of 2 ft (600 mm) beyond the edge of the paved shoulder. A minimum of two (2) posts shall be used.

The attachment of temporary signs to existing sign structures or sign panels shall be approved by the Engineer. Any damage to the existing signs due to the Contractor's operations shall be repaired or signs replaced, as determined by the Engineer, at the Contractor's expense.

Signs which are placed on overhead bridge structures shall be fastened to the handrail with stainless steel bands. These signs shall rest on the concrete parapet where possible. The Contractor shall furnish mounting details for approval by the Engineer.

Method Of Measurement.

This work shall be measured for payment in square feet (square meters) edge to edge (horizontally and vertically).

All hardware, posts or skids, supports, bases for ground mounted signs, connections, which are required for mounting these signs will be included as part of this pay item.

Basis Of Payment.

This work shall be paid for at the contract unit price per square foot (square meter) for TEMPORARY INFORMATION SIGNING.

MAST ARM SIGN PANELS

Effective: May 22, 2002

Revised: July 1, 2015

720.01TS

Add the following to Article 720.02 of the Standard Specifications:

Sign stiffening channel systems shall be aluminum and meet the requirements of ASTM 6261-T5. Sign mounting banding, buckles and buckle straps shall be manufactured from AISI 201 stainless steel.

KEEPING ARTERIAL ROADWAYS OPEN TO TRAFFIC

Effective: January 22, 2003

Revised: January 1, 2007

The Contractor shall provide the necessary traffic control devices to warn the public and to delineate the work zone as required in these Special Provisions, the Standard Specifications, and the State Standards.

Daily arterial lane closures shall be in accordance with the Standard Specifications, Highway Standards and the direction of the Engineer. The Contractor shall request and gain approval from the Illinois Department of Transportation's Arterial Traffic Control Supervisor at (847-705-4470) seventy-two (72) hours in advance of all long term (24 hrs. or longer) lane closures.

Arterial lane closures will only be permitted during the **off-peak** traffic volume hours. **Peak traffic volume hours are defined as weekdays (Monday through Friday) from 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM.**

Full closure of any arterial lanes will only be permitted for a maximum period of 15 minutes during the **off-peak** traffic volume hours. During full roadway closures, the Contractor will be required to reduce the roadway to only one open traffic lane in the affected direction of travel using the appropriate State Standard. Police forces shall be notified and requested to close the remaining lane to facilitate the necessary work activities. The Contractor shall notify the District One Arterial Traffic Control Supervisor at (847) 705-4470 seventy-two (72) hours in advance of the proposed road closure.

Private vehicles shall not be parked in the work zone. Contractor's equipment and/or vehicles shall not be parked on the shoulders or in the median during non-working hours. The parking of equipment and/or vehicles on State right-of-way will only be permitted at locations approved by the Engineer.

TRAFFIC SIGNAL GENERAL REQUIREMENTS

Effective: May 22, 2002

Revised: July 1, 2015

800.01TS

These Traffic Signal Special Provisions and the "District One Standard Traffic Signal Design Details" supplement the requirements of the State of Illinois "Standard Specifications for Road and Bridge Construction." The intent of these Special Provisions is to prescribe the materials and construction methods commonly used for traffic signal installations.

- All material furnished shall be new unless otherwise noted herein.
- Traffic signal construction and maintenance work shall be performed by personnel holding current IMSA Traffic Signal Technician Level II certification. A copy of the certification shall be immediately available upon request of the Engineer.
- The work to be done under this contract consists of furnishing, installing and maintaining all traffic signal work and items as specified in the Plans and as specified herein in a manner acceptable and approved by the Engineer.

Definitions of Terms.

Add the following to Section 101 of the Standard Specifications:

101.56 Vendor. Company that sells a particular type of product directly to the contractor or the Equipment Supplier.

101.57 Equipment supplier. Company that supplies, represents and provides technical support for IDOT District One approved traffic signal controllers and other related equipment. The Equipment Supplier shall be located within IDOT District One and shall:

- Be full service with on-site facilities to assemble, test and trouble-shoot traffic signal controllers and cabinet assemblies.
- Maintain an inventory of IDOT District One approved controllers and cabinets.
- Be staffed with permanent sales and technical personnel able to provide traffic signal controller and cabinet expertise and support.
- Technical staff shall hold current IMSA Traffic Signal Technician Level III certification and shall attend traffic signal turn-ons and inspections with a minimum 14 calendar day notice.

Submittals.

Revise Article 801.05 of the Standard Specifications to read:

All material approval requests shall be submitted electronically through the District's SharePoint System unless directed otherwise by the Engineer. Electronic material submittals shall follow the District's Traffic Operations Construction Submittals guidelines. General requirements include:

1. All material approval requests shall be made prior to or no later than the date of the preconstruction meeting. A list of major traffic signal items can be found in Article 801.05. Material or equipment which is similar or identical shall be the product of the same manufacturer, unless necessary for system continuity. Traffic signal materials and equipment shall bear the U.L. label whenever such labeling is available.

2. Product data and shop drawings shall be assembled by pay item. Only the top sheet of each pay item submittal will be stamped by the Department with the review status, except shop drawings for mast arm pole assemblies and the like will be stamped with the review status on each sheet.
3. Original manufacturer published product data and shop drawing sheets with legible dimensions and details shall be submitted for review.
4. When hard copy submittals are necessary, four complete copies of the manufacturer's descriptive literatures and technical data for the traffic signal materials shall be submitted. For hard copy or electronic submittals, the descriptive literature and technical data shall be adequate for determining whether the materials meet the requirements of the plans and specifications. If the literature contains more than one item, the Contractor shall indicate which item or items will be furnished.
5. When hard copy submittals are necessary for structural elements, four complete copies of the shop drawings for the mast arm assemblies and poles, and the combination mast arm assemblies and poles showing, in detail, the fabrication thereof and the certified mill analyses of the materials used in the fabrication, anchor rods, and reinforcing materials shall be submitted.
6. Partial or incomplete submittals will be returned without review.
7. Certain non-standard mast arm poles and special structural elements will require additional review from IDOT's Central Office. Examples include ornamental/decorative, non-standard length mast arm pole assemblies and monotube structures. The Contractor shall account for the additional review time in his schedule.
8. The contract number or permit number, project location/limits and corresponding pay code number must be on each sheet of correspondence, catalog cuts and mast arm poles and assemblies drawings.
9. Where certifications and/or warranties are specified, the information submitted for approval shall include certifications and warranties. Certifications involving inspections, and/or tests of material shall be complete with all test data, dates, and times.
10. After the Engineer reviews the submittals for conformance with the design concept of the project, the Engineer will stamp the drawings indicating their status as 'Approved', 'Approved-As-Noted', 'Disapproved', or 'Incomplete'. Since the Engineer's review is for conformance with the design concept only, it is the Contractor's responsibility to coordinate the various items into a working system as specified. The Contractor shall not be relieved from responsibility for errors or omissions in the shop, working, layout drawings, or other documents by the Department's approval thereof. The Contractor must still be in full compliance with contract and specification requirements.
11. The Contractor shall secure approved materials in a timely manner to assure construction schedules are not delayed.
12. All submitted items reviewed and marked 'APPROVED AS NOTED', 'DISAPPROVED', or 'INCOMPLETE' are to be resubmitted in their entirety, unless otherwise indicated within the submittal comments, with a disposition of previous comments to verify contract compliance at no additional cost to the contract.
13. Exceptions to and deviations from the requirements of the Contract Documents will not be allowed. It is the Contractor's responsibility to note any deviations from Contract requirements at the time of submittal and to make any requests for deviations in writing to the Engineer. In general, substitutions will not be acceptable. Requests for substitutions must demonstrate that the proposed substitution is superior to the material or equipment required by the Contract Documents. No exceptions, deviations or substitutions will be permitted without the approval of the Engineer.

14. Contractor shall not order major equipment such as mast arm assemblies prior to Engineer approval of the Contractor marked proposed traffic signal equipment locations to assure proper placement of contract required traffic signal displays, push buttons and other facilities. Field adjustments may require changes in proposed mast arm length and other coordination.

Marking Proposed Locations.

Revise “Marking Proposed Locations for Highway Lighting System” of Article 801.09 to read “Marking Proposed Locations for Highway Lighting System and Traffic Signals.”

Add the following to Article 801.09 of the Standard Specifications:

It shall be the contractor's responsibility to verify all dimensions and conditions existing in the field prior to ordering materials and beginning construction. This shall include locating the mast arm foundations and verifying the mast arms lengths.

Inspection of Electrical Systems.

Add the following to Article 801.10 of the Standard Specifications:

- (c) All cabinets including temporary traffic signal cabinets shall be assembled by an approved equipment supplier in District One. The Department reserves the right to request any controller and cabinet to be tested at the equipment supplier's facility prior to field installation, at no extra cost to this contract.

Maintenance and Responsibility.

Revise Article 801.11 of the Standard Specifications to read:

- a. Existing traffic signal installations and/or any electrical facilities at all or various locations may be altered or reconstructed totally or partially as part of the work on this Contract. The Contractor is hereby advised that all traffic control equipment, presently installed at these locations, may be the property of the State of Illinois, Department of Transportation, Division of Highways, County, Private Developer, Municipality or Transit Agency in which they are located. Once the Contractor has begun any work on any portion of the project, all traffic signals within the limits of this contract or those which have the item "Maintenance of Existing Traffic Signal Installation," "Temporary Traffic Signal Installation(s)" and/or "Maintenance of Existing Flashing Beacon Installation," shall become the full responsibility of the Contractor. The Contractor shall supply the Engineer, Area Traffic Signal Maintenance and Operations Engineer, IDOT ComCenter and the Department's Electrical Maintenance Contractor with two 24-hour emergency contact names and telephone numbers.
- b. Automatic Traffic Enforcement equipment such as red lighting running and railroad crossing camera systems are owned and operated by others and the Contractor shall not be responsible for maintaining this equipment.
- c. Regional transit, County and other agencies may also have equipment connected to existing traffic signal or peripheral equipment such as PTZ cameras, switches, transit signal priority (TSP and BRT) servers and other devices that shall be included with traffic signal maintenance at no additional cost to the contract.

- d. When the project has a pay item for “Maintenance of Existing Traffic Signal Installation,” “Temporary Traffic Signal Installation(s)” and/or “Maintenance of Existing Flashing Beacon Installation,” the Contractor must notify both the Area Traffic Signal Maintenance and Operations Engineer at (847) 705-4424 and the Department’s Electrical Maintenance Contractor, of their intent to begin any physical construction work on the Contract or any portion thereof. This notification must be made a minimum of seven (7) working days prior to the start of construction to allow sufficient time for inspection of the existing traffic signal installation(s) and transfer of maintenance to the Contractor. If work is started prior to an inspection, maintenance of the traffic signal installation(s) will be transferred to the Contractor without an inspection. The Contractor will become responsible for repairing or replacing all equipment that is not operating properly or is damaged at no cost to the owner of the traffic signal. Final repairs or replacement of damaged equipment must meet the approval of the Engineer prior to or at the time of final inspection otherwise the traffic signal installation will not be accepted.
- e. The Contractor is advised that the existing and/or temporary traffic signal installation must remain in operation during all construction stages, except for the most essential down time. Any shutdown of the traffic signal installation, which exceeds fifteen (15) minutes, must have prior approval of the Engineer. Approval to shut down the traffic signal installation will only be granted during the period extending from 10:00 a.m. to 3:00 p.m. on weekdays. Shutdowns shall not be allowed during inclement weather or holiday periods.
- f. The Contractor shall be fully responsible for the safe and efficient operation of the traffic signals and other equipment noted herein. Any inquiry, complaint or request by the Department, the Department’s Electrical Maintenance Contractor or the public, shall be investigated and repairs begun within one hour. Failure to provide this service will result in liquidated damages of \$1000 per day per occurrence. In addition, the Department reserves the right to assign any work not completed within this timeframe to the Electrical Maintenance Contractor. All costs associated to repair this uncompleted work shall be the responsibility of the Contractor. Failure to pay these costs to the Electrical Maintenance Contractor within one month after the incident will result in additional liquidated damages of \$1000 per month per occurrence. Unpaid bills will be deducted from the cost of the Contract. The Department may inspect any signalizing device on the Department’s highway system at any time without notification.
- g. Any proposed activity in the vicinity of a highway-rail grade crossing must adhere to the guidelines set forth in the current edition of the Manual on Uniform Traffic Control Devices (MUTCD) regarding work in temporary traffic control zones in the vicinity of highway-rail grade crossings which states that lane restrictions, flagging, or other operations shall not create conditions where vehicles can be queued across the railroad tracks. If the queuing of vehicles across the tracks cannot be avoided, a uniformed law enforcement officer or flagger shall be provided at the crossing to prevent vehicles from stopping on the tracks, even if automatic warning devices are in place.
- h. The Contractor shall be responsible to clear snow, ice, dirt, debris or other condition that obstructs visibility of any traffic signal display or access to traffic signal equipment.

- i. The Contractor shall maintain the traffic signal in normal operation during short or long term loss of utility or battery back-up power at critical locations designated by the Engineer. Critical locations may include traffic signals interconnected to railroad warning devices, expressway ramps, intersection with an SRA route, critical corridors or other locations identified by the Engineer. Temporary power to the traffic signal must meet applicable NEC and OSHA guidelines and may include portable generators and/or replacement batteries. Temporary power to critical locations shall not be for separately but shall be included in the contract.

Damage to Traffic Signal System.

Add the following to Article 801.12(b) of the Standard Specifications to read:

Any traffic signal control equipment damaged or not operating properly from any cause shall be replaced with new equipment meeting current District One traffic signal specifications and provided by the Contractor at no additional cost to the Contract and/or owner of the traffic signal system, all as approved by the Engineer. Final replacement of damaged equipment must meet the approval of the Engineer prior to or at the time of final inspection otherwise the traffic signal installation will not be accepted. Cable splices are only allowed at the bases of post and mast arms.

Temporary replacement of damaged or knockdown of a mast arm pole assembly shall require construction of a full or partial span wire signal installation or other method approved by the Engineer to assure signal heads are located overhead and over traveled pavement. Temporary replacement of mast arm mount signals with post mount signals will not be permitted.

Automatic Traffic Enforcement equipment, such as Red Light Enforcement cameras, detectors, and peripheral equipment, damaged or not operating properly from any cause, shall be the responsibility of the municipality or the Automatic Traffic Enforcement company per Permit agreement.

Traffic Signal Inspection (TURN-ON).

Revise Article 801.15(b) of the Standard Specifications to read:

It is the intent to have all electric work completed and equipment field tested by the Equipment Supplier prior to the Department's "turn-on" field inspection. If in the event the Engineer determines work is not complete and the inspection will require more than two (2) hours to complete, the inspection shall be canceled and the Contractor will be required to reschedule at another date. The maintenance of the traffic signals will not be accepted until all punch list work is corrected and re-inspected.

When the road is open to traffic, except as otherwise provided in Section 850 of the Standard Specifications, the Contractor may request a turn-on and inspection of the completed traffic signal installation at each separate location. This request must be made to the Area Traffic Signal Maintenance and Operations Engineer at (847) 705-4424 a minimum of seven (7) working days prior to the time of the requested inspection. The Department will not grant a field inspection until written or electronic notification is provided from the Contractor that the equipment has been field tested and the intersection is operating according to Contract requirements. The Contractor must invite local fire department personnel to the turn-on when Emergency Vehicle Preemption (EVP) is included in the project. When the contract includes the item RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM,

OPTIMIZE TRAFFIC SIGNAL SYSTEM, or TEMPORARY TRAFFIC SIGNAL TIMINGS, the Contractor must notify the SCAT Consultant of the turn-on/detour implementation schedule, as well as stage changes and phase changes during construction.

The Contractor must have all traffic signal work completed and the electrical service installation connected by the utility company prior to requesting an inspection and turn-on of the traffic signal installation. The Contractor shall be responsible to provide a police officer to assist with traffic control at the time of testing.

The Contractor shall provide a representative from the control equipment vendor's office who is knowledgeable of the cabinet design and controller functions to attend the traffic signal inspection for both permanent and temporary traffic signal turn-ons.

Upon demonstration that the signals are operating and all work is completed in accordance with the Contract and to the satisfaction of the Engineer, the Engineer will then allow the signals to be placed in continuous operation. The Agency that is responsible for the maintenance of each traffic signal installation will assume the maintenance upon successful completion of this inspection.

The District requires the following Final Project Documentation from the Contractor at traffic signal turn-ons in electronic format in addition to hard copies where noted. A CD/DVD shall be submitted with separate folders corresponding to each numbered title below. The CD/DVD shall be labelled with date, project location, company and contract or permit number. Record Drawings, Inventory and Material Approvals shall be submitted prior to traffic signal turn-on for review by the Department as described here-in.

Final Project Documentation:

1. Record Drawings. Signal plans of record with field revisions marked in red ink. One hard copy set of 11"x17" record drawings shall also be provided.
2. Inventory. Inventory of new and existing traffic signal equipment including cabinet types and devices within cabinets in an Excel spread sheet format. One hard copy shall also be provided.
3. Pictures. Digital pictures of a minimum 12M pixels of each intersection approach showing all traffic signal displays and equipment. Pictures shall include controller cabinet equipment in enough detail to clearly identify manufacture and model of major equipment.
4. Field Testing. Written notification from the Contractor and the equipment vendor of satisfactory field testing with corresponding material performance measurements, such as for detector loops and fiber optic systems (see Article 801.13). One hard copy of all contract required performance measurement testing shall also be provided.
5. Materials Approval. The material approval letter. A hard copy shall also be provided.
6. Manuals. Operation and service manuals of the signal controller and associated control equipment. One hard copy shall also be provided.
7. Cabinet Wiring Diagram and Cable Logs. Five (5) hard copies 11" x 17" of the cabinet wiring diagrams shall be provided along with electronic pdf and dgn files of the cabinet wiring diagram. Five hard copies of the cable logs and electronic excel files shall be provided with cable #, number of conductors and spares, connected device/signal head and intersection location.
8. Controller Programming Settings. The traffic signal controller's timings; backup timings; coordination splits, offsets, and cycles; TBC Time of Day, Week and Year Programs;

Traffic Responsive Program, Detector Phase Assignment, Type and Detector Switching; and any other functions programmable from the keyboard. The controller manufacturer shall also supply a printed form, not to exceed 11" x 17" for recording that data noted above. The form shall include a location, date, manufacturer's name, controller model and software version. The form shall be approved by the Engineer and a minimum of three (3) copies must be furnished at each turn-on. The manufacturer must provide all programming information used within the controller at the time of turn-on.

9. Warrantees and Guarantees. All manufacturer and contractor warrantees and guarantees required by Article 801.14.
10. GPS coordinate of traffic signal equipment as describe in the Record Drawings section herein.

Acceptance of the traffic signal equipment by the Department shall be based upon inspection results at the traffic signal "turn on", completeness of the required documentation and successful operation during a minimum 72 hour "burn-in" period following activation of the traffic signal. If approved, traffic signal acceptance shall be verbal at the "turn on" inspection followed by written correspondence from the Engineer. The Contractor shall be responsible for all traffic signal equipment and associated maintenance thereof until Departmental acceptance is granted.

All equipment and/or parts to keep the traffic signal installation operating shall be furnished by the Contractor. No spare traffic signal equipment is available from the Department.

All punch list work shall be completed within two (2) weeks after the final inspection. The Contractor shall notify the Electrical Maintenance Contractor to inspect all punch list work. Failure to meet these time constraints shall result in liquidated damage charges of \$500 per month per incident.

All cost of work and materials required to comply with the above requirements shall be included in the pay item bid prices, under which the subject materials and signal equipment are paid, and no additional compensation will be allowed. Materials and signal equipment not complying with the above requirements shall be subject to removal and disposal at the Contractor's expense.

Record Drawings.

The requirements listed for Electrical Installation shall apply for Traffic Signal Installations in Article 801.16. Revise the 2nd paragraph of Article 801.16 of the Standard Specifications to read:

"When the work is complete, and seven days before the request for a final inspection, the reduced-size set of contract drawings, stamped "RECORD DRAWINGS", shall be submitted to the Engineer for review and approval and shall be stamped with the date and the signature of the Contractor's supervising Engineer or electrician. The record drawings shall be submitted in PDF format on CDROM as well as hardcopy for review and approval. If the contract consists of multiple intersections, each intersection shall be saved as an individual PDF file with TS# and location name in its file name.

In addition to the record drawings, copies of the final catalog cuts which have been Approved or Approved as Noted shall be submitted in PDF format along with the record drawings. The PDF files shall clearly indicate the pay item either by filename or PDF Table of Contents referencing the respective pay item number for multi-item PDF files. Specific part or model numbers of items which have been selected shall be clearly visible."

As part of the record drawings, the Contractor shall inventory all traffic signal equipment, new or existing, on the project and record information in an Excel spreadsheet. The inventory shall include equipment type, model numbers, software manufacturer and version and quantities.

Add the following to Article 801.16 of the Standard Specifications:

“In addition to the specified record drawings, the Contractor shall record GPS coordinates of the following traffic signal components being installed, modified or being affected in other ways by this contract:

- All Mast Arm Poles and Posts
- Traffic Signal Wood Poles
- Rail Road Bungalow
- UPS
- Handholes
- Conduit roadway crossings
- Controller Cabinets
- Communication Cabinets
- Electric Service Disconnect locations
- CCTV Camera installations
- Fiber Optic Splice Locations
- Conduit Crossings

Datum to be used shall be North American 1983.

Data shall be provided electronically and in print form. The electronic format shall be compatible with MS Excel. Latitude and Longitude shall be in decimal degrees with a minimum of 6 decimal places. Each coordinate shall have the following information:

- File shall be named: TSXXX-YY-MM-DD (i.e. TS22157_15-01-01)
- Each intersection shall have its own file
- Row 1 should have the location name (i.e. IL 31 @ Klausen)
- Row 2 is blank
- Row 3 is the headers for the columns
- Row 4 starts the data
- Column A (Date) – should be in the following format: MM/DD/YYYY
- Column B (Item) – as shown in the table below
- Column C (Description) – as shown in the table below
- Column D and E (GPS Data) – should be in decimal form, per the IDOT special provisions

Examples:

Date	Item	Description	Latitude	Longitude
01/01/2015	MP (Mast Arm Pole)	NEQ, NB, Dual, Combination Pole	41.580493	- 87.793378

01/01/2015	HH (Handhole)	Heavy Duty, Fiber, Intersection, Double	41.558532	- 87.792571
01/01/2015	ES (Electrical Service)	Ground mount, Pole mount	41.765532	- 87.543571
01/01/2015	CC (Controller Cabinet)		41.602248	- 87.794053
01/01/2015	RSC (Rigid Steel Crossing)	IL 31 east side crossing south leg to center HH at Klausen	41.611111	- 87.790222
01/01/2015	PTZ (PTZ)	NEQ extension pole	41.593434	- 87.769876
01/01/2015	POST (Post)		41.651848	- 87.762053
01/01/2015	MCC (Master Controller Cabinet)		41.584593	- 87.793378
01/01/2015	COMC (Communication Cabinet)		41.584600	- 87.793432
01/01/2015	BBS (Battery Backup System)		41.558532	- 87.792571
01/01/2015	CNCR (Conduit Crossing)	4-inch IL 31 n/o of Klausen	41.588888	- 87.794440

Prior to the collection of data, the contractor shall provide a sample data collection of at least six data points of known locations to be reviewed and verified by the Engineer to be accurate within 1 foot. Upon verification, data collection can begin. Data collection can be made as construction progresses, or can be collected after all items are installed. If the data is unacceptable the contractor shall make corrections to the data collection equipment and or process and submit the data for review and approval as specified.

Accuracy. Data collected is to be mapping grade. A handheld mapping grade GPS device shall be used for the data collection. The receiver shall support differential correction and data shall have a minimum 1 foot accuracy after post processing.

GPS receivers integrated into cellular communication devices, recreational and automotive GPS devices are not acceptable.

The GPS shall be the product of an established major GPS manufacturer having been in the business for a minimum of 6 years.”

Delete the last sentence of the 3rd paragraph of Article 801.16.

Locating Underground Facilities.

Revise Section 803 to the Standard Specifications to read:

IDOT traffic signal facilities are not part of any of the one-call locating service such as J.U.L.I.E or Digger. If this Contract requires the services of an Electrical Contractor, the Contractor shall be

responsible at his/her own expense for locating existing IDOT electrical facilities prior to performing any work. If this Contract does not require the services of an Electrical Contractor, the Contractor may request one free locate for existing IDOT electrical facilities from the District One Electrical Maintenance Contractor prior to the start of any work. Additional requests may be at the expense of the Contractor. The location of underground traffic facilities does not relieve the Contractor of their responsibility to repair any facilities damaged during construction at their expense.

The exact location of all utilities shall be field verified by the Contractor before the installation of any components of the traffic signal system. For locations of utilities, locally owned equipment, and leased enforcement camera system facilities, the local Counties or Municipalities may need to be contacted: in the City of Chicago contact Digger at (312) 744-7000 and for all other locations contact J.U.L.I.E. at 1-800-892-0123 or 811.

Restoration of Work Area.

Add the following article to Section 801 of the Standard Specifications:

801.17 Restoration of work area. Restoration of the traffic signal work area shall be included in the related pay items such as foundation, conduit, handhole, underground raceways, etc. All roadway surfaces such as shoulders, medians, sidewalks, pavement, etc. shall be replaced in kind. All damage to mowed lawns shall be replaced with an approved sod, and all damage to unmowed fields shall be seeded. All brick pavers disturbed in the work area shall be restored to their original configuration as directed by the Engineer. All damaged brick pavers shall be replaced with a comparable material approved by the Engineer. Restoration of the work area shall be included in the contract without any extra compensation allowed to the Contractor.

Bagging Signal Heads.

Light tan colored traffic and pedestrian signal reusable covers shall be used to cover dark/un-energized signal sections and visors. Covers shall be made of outdoor fabric with urethane coating for repelling water, have elastic fully sewn around the cover ends for a tight fit over the visor, and have a minimum of two straps with buckles to secure the cover to the backplate. A center mesh strip allows viewing without removal for signal status testing purposes. Covers shall include a message indicating the signal is not in service.

OPTIMIZE TRAFFIC SIGNAL SYSTEM

Effective: May 22, 2002

Revised: July 1, 2015

800.02TS

Description.

This work shall consist of optimizing a closed loop traffic signal system.

OPTIMIZE TRAFFIC SIGNAL SYSTEM applies when a new or existing closed loop traffic signal system is to be optimized and a formal Signal Coordination and Timing (SCAT) Report is to be prepared. The purpose of this work is to improve system performance by optimizing traffic signal timings, developing a time of day program and a traffic responsive program.

After the signal improvements are completed, the signal system shall be optimized as specified by an approved Consultant who has previous experience in optimizing Closed Loop Traffic Signal Systems for District One of the Illinois Department of Transportation. The Contractor shall contact the Traffic Signal Engineer at (847) 705-4424 for a listing of approved Consultants. Traffic signal system optimization work, including fine-tuning adjustments of the optimized system, shall follow the requirements stated in the most recent IDOT District 1 SCAT Guidelines, except as noted herein.

A listing of existing signal equipment, interconnect information, phasing data, and timing patterns may be obtained from the Department, if available and as appropriate. The existing SCAT Report is available for review at the District One office and if the Consultant provides blank a CD, copies of computer simulation files for the existing optimized system and a timing database that includes intersection displays will be made for the Consultant. The Consultant shall confer with the Traffic Signal Engineer prior to optimizing the system to determine if any extraordinary conditions exist that would affect traffic flows in the vicinity of the system, in which case, the Consultant may be instructed to wait until the conditions return to normal or to follow specific instructions regarding the optimization.

(a) The following tasks are associated with OPTIMIZE TRAFFIC SIGNAL SYSTEM.

1. Appropriate signal timings and offsets shall be developed for each intersection and appropriate cycle lengths shall be developed for the closed loop signal system.
2. Traffic counts shall be taken at all intersections after the permanent traffic signals are approved for operation by the Area Traffic Signal Operations Engineer. Manual turning movement counts shall be conducted from 6:30 a.m. to 9:30 a.m., 11:00 a.m. to 1:00 p.m., and 3:30 p.m. to 6:30 p.m. on a typical weekday from midday Monday to midday Friday and on a Saturday or Sunday, as directed by the Engineer, to account for special traffic generators such as shopping centers, educational institutes and special event facilities. The turning movement counts shall identify cars, and single-unit and multi-unit heavy vehicles.
3. As necessary, the intersections shall be re-addressed and all system detectors reassigned in the master controller according to the current standard of District One.
4. A traffic responsive program shall be developed, which considers both volume and occupancy. A time-of-day program shall be developed for used as a back-up system.
5. Proposed signal timing plan for the new or modified intersection shall be forwarded to IDOT for review prior to implementation.
6. Consultant shall conduct on-site implementation of the timings and make fine-tuning adjustments to the timings in the field to alleviate observed adverse operating conditions and to enhance operations. The consultant shall respond to IDOT comments and public complaints for a minimum period of 90 days from date of timing plan implementation.
7. Speed and delay studies shall be conducted during each of the count periods along the system corridor in the field before and after implementation of the proposed timing plans for comparative evaluations. These studies should utilize specialized electronic timing and measuring devices.

(b) The following deliverables shall be provided for OPTIMIZE TRAFFIC SIGNAL SYSTEM.

1. Consultant shall furnish to IDOT one (1) copy of a SCAT Report for the optimized system.
 The SCAT Report shall include the following elements:

Cover Page in color showing a System Map
Figures <ol style="list-style-type: none"> 1. System overview map – showing system number, system schematic map with numbered system detectors, oversaturated movements, master location, system phone number, cycle lengths, and date of completion. 2. General location map in color – showing signal system location in the metropolitan area. 3. Detail system location map in color – showing cross street names and local controller addresses. 4. Controller sequence – showing controller phase sequence diagrams.
Table of Contents
Tab 1: Final Report <ol style="list-style-type: none"> 1. Project Overview 2. System and Location Description (Project specific) 3. Methodology 4. Data Collection 5. Data Analysis and Timing Plan Development 6. Implementation <ol style="list-style-type: none"> a. Traffic Responsive Programming (Table of TRP vs. TOD Operation) with am, md, and pm cycle lengths 7. Evaluation <ol style="list-style-type: none"> a. Speed and Delay runs
Tab 2: Turning Movement Counts <ol style="list-style-type: none"> 1. Turning Movement Counts (Showing turning movement counts in the intersection diagram for each period, including truck percentage)
Tab 3: Synchro Analysis <ol style="list-style-type: none"> 1. AM: Time-Space diagram in color, followed by intersection Synchro report (Timing report) summarizing the implemented timings. 2. Midday: same as AM 3. PM: same as AM 4. Special weekend or off-peak traffic generators (shopping centers, educational facilities, arenas, etc.): same as AM
Tab 4: Speed, Delay Studies <ol style="list-style-type: none"> 1. Summary of before and after runs results in two (2) tables showing travel time and delay time. 2. Plot of the before and after runs diagram for each direction and time period.
Tab 5: Environmental Report <ol style="list-style-type: none"> 1. Environmental impact report including gas consumption, NO2, HCCO, improvements.
Tab 6: Electronic Files <ol style="list-style-type: none"> 1. Two (2) CDs for the optimized system. The CDs shall include the following elements: <ol style="list-style-type: none"> a. Electronic copy of the SCAT Report in PDF format b. Copies of the Synchro files for the optimized system c. Traffic counts for the optimized system d. New or updated intersection graphic display files for each of the system intersections and the system graphic display file including system detector locations and addresses.

Basis of Payment.

The work shall be paid for at the contract unit each for OPTIMIZE TRAFFIC SIGNAL SYSTEM, which price shall be payment in full for performing all work described herein for the entire traffic signal system. Following the completion of traffic counts, 25 percent of the bid price will be paid. Following the completion of the Synchro analysis, 25 percent of the bid price will be paid. Following the setup and fine tuning of the timings, the speed-delay study, and the TRP programming, 25 percent of the bid price will be paid. The remaining 25 percent will be paid when the system is working to the satisfaction of the engineer and an approved report and CD have been submitted.

RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM

Effective: May 22, 2002

Revised: July 1, 2015

800.03TS

Description.

This work shall consist of re-optimizing a closed loop traffic signal system according to the following Levels of work.

LEVEL I applies when improvements are made to an existing signalized intersection within an existing closed loop traffic signal system. The purpose of this work is to integrate the improvements to the subject intersection into the signal system while minimizing the impacts to the existing system operation. This type of work would be commonly associated with the addition of signal phases, pedestrian phases, or improvements that do not affect the capacity at an intersection.

LEVEL II applies when improvements are made to an existing signalized intersection within an existing closed loop traffic signal system and detailed analysis of the intersection operation is desired by the engineer, or when a new signalized or existing signalized intersection is being added to an existing system, but optimization of the entire system is not required. The purpose of this work is to optimize the subject intersection, while integrating it into the existing signal system with limited impact to the system operations. This item also includes an evaluation of the overall system operation, including the traffic responsive program.

For the purposes of re-optimization work, an intersection shall include all traffic movements operated by the subject controller and cabinet.

After the signal improvements are completed, the signal shall be re-optimized as specified by an approved Consultant who has previous experience in optimizing Closed Loop Traffic Signal Systems for District One of the Illinois Department of Transportation. The Contractor shall contact the Traffic Signal Engineer at (847) 705-4424 for a listing of approved Consultants. Traffic signal system optimization work, including fine-tuning adjustments of the optimized system, shall follow the requirements stated in the most recent IDOT District 1 SCAT Guidelines, except as note herein.

A listing of existing signal equipment, interconnect information, phasing data, and timing patterns may be obtained from the Department, if available and as appropriate. The existing SCAT Report is available for review at the District One office and if the Consultant provides blank computer discs, copies of computer simulation files for the existing optimized system and a timing database will be made for the Consultant. The Consultant shall confer with the Traffic Signal Engineer prior to optimizing the system to determine if any extraordinary conditions exist that would affect traffic flows

in the vicinity of the system, in which case, the Consultant may be instructed to wait until the conditions return to normal or to follow specific instructions regarding the optimization.

(a) LEVEL I Re-Optimization

1. The following tasks are associated with LEVEL I Re-Optimization.
 - a. Appropriate signal timings shall be developed for the subject intersection and existing timings shall be utilized for the rest of the intersections in the system.
 - b. Proposed signal timing plan for the modified intersection(s) shall be forwarded to IDOT for review prior to implementation.
 - c. Consultant shall conduct on-site implementation of the timings at the turn-on and make fine-tuning adjustments to the timings of the subject intersection in the field to alleviate observed adverse operating conditions and to enhance operations. The consultant shall respond to IDOT comments and public complaints for a minimum period of 60 days from date of timing plan implementation.
2. The following deliverables shall be provided for LEVEL I Re-Optimization.
 - a. Consultant shall furnish to IDOT a cover letter describing the extent of the re-optimization work performed.
 - b. Consultant shall furnish an updated intersection graphic display for the subject intersection to IDOT and to IDOT's Traffic Signal Maintenance Contractor.

(b) LEVEL II Re-Optimization

1. In addition to the requirements described in the LEVEL I Re-Optimization above, the following tasks are associated with LEVEL II Re-Optimization.
 - a. Traffic counts shall be taken at the subject intersection(s) after the traffic signals are approved for operation by the Area Traffic Signal Operations Engineer. Manual turning movement counts shall be conducted from 6:30 a.m. to 9:30 a.m., 11:00 a.m. to 1:00 p.m., and 3:30 p.m. to 6:30 p.m. on a typical weekday from midday Monday to midday Friday and on a Saturday and/or Sunday, as directed by the Engineer, to account for special traffic generators such as shopping centers, educational institutes and special event facilities. The turning movement counts shall identify cars, and single-unit, multi-unit heavy vehicles, and transit buses.
 - b. As necessary, the intersection(s) shall be re-addressed and all system detectors reassigned in the master controller according to the current standard of District One.
 - c. Traffic responsive program operation shall be evaluated to verify proper pattern selection and lack of oscillation and a report of the operation shall be provided to IDOT.
2. The following deliverables shall be provided for LEVEL II Re-Optimization.
 - a. Consultant shall furnish to IDOT one (1) copy of a technical memorandum for the optimized system. The technical memorandum shall include the following elements:
 - (1) Brief description of the project
 - (2) Printed copies of the analysis output from Synchro (or other appropriate, approved optimization software file)
 - (3) Printed copies of the traffic counts conducted at the subject intersection
 - b. Consultant shall furnish to IDOT two (2) CDs for the optimized system. The CDs shall include the following elements:
 - (1) Electronic copy of the technical memorandum in PDF format

- (2) Revised Synchro files (or other appropriate, approved optimization software file) including the new signal and the rest of the signals in the closed loop system
- (3) Traffic counts conducted at the subject intersection(s)
- (4) New or updated intersection(s) graphic display file for the subject intersection(s)
- (5) The CD shall be labeled with the IDOT system number and master location, as well as the submittal date and the consultant logo. The CD case shall include a clearly readable label displaying the same information securely affixed to the side and front.

Basis of Payment.

This work shall be paid for at the contract unit price each for RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM – LEVEL I or RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM – LEVEL II, which price shall be payment in full for performing all work described herein per intersection. Following completion of the timings and submittal of specified deliverables, 100 percent of the bid price will be paid. Each intersection will be paid for separately.

ELECTRIC SERVICE INSTALLATION

Effective: May 22, 2002

Revised: July 1, 2015

805.01TS

Revise Section 805 of the Standard Specifications to read:

Description.

This work shall consist of all materials and labor required to install, modify, or extend the electric service installation. All installations shall meet the requirements of the “District One Standard Traffic Signal Design Details”.

General.

The electric service installation shall be the electric service disconnecting means and it shall be identified as suitable for use as service equipment.

The electric utility contact information is noted on the plans and represents the current information at the time of contract preparation. The Contractor must request in writing for service and/or service modification within 10 days of contract award and must follow-up with the electric utility to assure all necessary documents and payment are received by the utility. The Contractor shall forward copies of all correspondence between the contractor and utility company to the Engineer and Area Traffic Signal Maintenance and Operations Engineer. The service agreement and sketch shall be submitted for signature to the IDOT’s Traffic Operations Programs Engineer.

Materials.

- a. General. The completed control panel shall be constructed in accordance with UL Std. 508A, Industrial Control Panel, and carry the UL label. Wire terminations shall be UL listed.
- b. Enclosures.

1. Pole Mounted Cabinet. The cabinet shall be UL 50, NEMA Type 4X, unfinished single door design, fabricated from minimum 0.080-inch (2.03 mm) thick Type 5052 H-32 aluminum. Seams shall be continuous welded and ground smooth. Stainless steel screws and clamps shall secure the cover and assure a watertight seal. The cover shall be removable by pulling the continuous stainless steel hinge pin. The cabinet shall have an oil-resistant gasket and a lock kit shall be provided with an internal O-ring in the locking mechanism assuring a watertight and dust-tight seal. The cabinet shall be sized to adequately house all required components with extra space for arrangement and termination of wiring. A minimum size of 14-inches (350 mm) high, 9-inches (225 mm) wide and 8-inches (200 mm) in depth is required. The cabinet shall be channel mounted to a wooden utility pole using assemblies recommended by the vendor.
 2. Ground Mounted Cabinet. The cabinet shall be UL 50, NEMA Type 3R unfinished single door design with back panel. The cabinet shall be fabricated from Type 5052 H-32 aluminum with the frame and door 0.125-inch (3.175 mm) thick, the top 0.250-inch (6.350 mm) thick and the bottom 0.500-inch (12.70 mm) thick. Seams shall be continuous welded and ground smooth. The door and door opening shall be double flanged. The door shall be approximately 80% of the front surface, with a full length tamperproof stainless steel .075-inch (1.91 mm) thick hinge bolted to the cabinet with stainless steel carriage bolts and nylocks nuts. The locking mechanism shall be slam-latch type with a keyhole cover. The cabinet shall be sized to adequately house all required components with extra space for arrangement and termination of wiring. A minimum size of 40-inches (1000 mm) high, 16-inches (400 mm) wide and 15-inches (375 mm) in depth is required. The cabinet shall be mounted upon a square Type A concrete foundation as indicated on the plans. The foundation is paid for separately.
 3. All enclosures shall include a green external power indicator LED light with circuitry as shown in the Electrical Service-Panel Diagram detail sheet. For pole mounted service enclosures, the power indicator light shall be mounted as shown in the detail. For ground mounted enclosures, the power indicator light shall be mounted on the side of the enclosure most visible from the major roadway.
- c. Electric Utility Meter Housing and Riser. The electric meter housing and meter socket shall be supplied and installed by the contractor. Electric utility required risers, weather/service head and any other materials necessary for connection shall also be included in the pay item. Materials shall be in accordance with the electric utility's requirements. For ground-mounted service, the electric utility meter housing shall be mounted to the enclosure. Metered service shall not be used unless specified in the plans.
 - d. Surge Protector. Overvoltage protection, with LED indicator, shall be provided for the 120 volt load circuit by the means MOV and thermal fusing technology. The response time shall be <5n seconds and operate within a range of -40C to +85C. The surge protector shall be UL 1449 Listed.
 - e. Circuit Breakers. Circuit breakers shall be standard UL listed molded case, thermal-magnetic bolt-on type circuit breakers with trip free indicating handles. 120 volt circuit breakers shall have an interrupting rating of not less than 65,000 rms symmetrical amperes.

Unless otherwise indicated, the main disconnect circuit breaker for the traffic signal controller shall be rated 60 amperes, 120 V and the auxiliary circuit breakers shall be rated 10 amperes, 120 V.

- f. Fuses, Fuseholders and Power Indicating Light. Fuses shall be small-dimensional cylindrical fuses of the dual element time-delay type. The fuses shall be rated for 600 V AC and shall have a UL listed interrupting rating of not less than 10,000 rms symmetrical amperes at rated voltage. The power indicating light shall be LED type with a green colored lens and shall be energized when electric utility power is present.
- g. Ground and Neutral Bus Bars. A single copper ground and neutral bus bar, mounted on the equipment panel shall be provided. Ground and neutral conductors shall be separated on the bus bar. Compression lugs, plus 2 spare lugs, shall be sized to accommodate the cables with the heads of the connector screws painted green for ground connections and white for neutral connections.
- h. Utility Services Connection. The Contractor shall notify the Utility Company marketing representative a minimum of 30 working days prior to the anticipated date of hook-up. This 30 day advance notification will begin only after the Utility Company marketing representative has received service charge payments from the Contractor. Prior to contacting the Utility Company marketing representative for service connection, the service installation controller cabinet and cable must be installed for inspection by the Utility Company.
- i. Ground Rod. Ground rods shall be copper-clad steel, a minimum of 10 feet (3.0m) in length, and 3/4 inch (20mm) in diameter. Ground rod resistance measurements to ground shall be 25 ohms or less. If necessary additional rods shall be installed to meet resistance requirements at no additional cost to the contract.

Installation.

- a. General. The Contractor shall confirm the orientation of the traffic service installation and its door side with the engineer, prior to installation. All conduit entrances into the service installation shall be sealed with a pliable waterproof material.
- b. Pole Mounted. Brackets designed for pole mounting shall be used. All mounting hardware shall be stainless steel. Mounting height shall be as noted on the plans or as directed by the Engineer.
- c. Ground Mounted. The service installation shall be mounted plumb and level on the foundation and fastened to the anchor bolts with hot-dipped galvanized or stainless steel nuts and washers. The space between the bottom of the enclosure and the top of the foundation shall be caulked at the base with silicone.

Basis of Payment.

The service installation shall be paid for at the contract unit price each for SERVICE INSTALLATION of the type specified which shall be payment in full for furnishing and installing the service installation complete. The CONCRETE FOUNDATION, TYPE A, which includes the ground rod, shall be paid for separately. SERVICE INSTALLATION, POLE MOUNTED shall include the 3/4 inch (20mm) grounding conduit, ground rod, and pole mount assembly. Any charges by the utility companies shall

be approved by the engineer and paid for as an addition to the contract according to Article 109.05 of the Standard Specifications.

GROUNDING OF TRAFFIC SIGNAL SYSTEMS

Effective: May 22, 2002

Revised: July 1, 2015

806.01TS

Revise Section 806 of the Standard Specifications to read:

General.

All traffic signal systems, equipment and appurtenances shall be properly grounded in strict conformance with the NEC. This work shall be in accordance with IDOT's District One Traffic Signal Design Details.

The grounding electrode system shall include a ground rod installed with each traffic signal controller concrete foundation and all mast arm and post concrete foundations. An additional ground rod will be required at locations where measured resistance exceeds 25 ohms. Ground rods are included in the applicable concrete foundation or service installation pay item and will not be paid for separately.

Testing shall be according to Article 801.13 (a) (4) and (5).

- a) The grounded conductor (neutral conductor) shall be white color coded. This conductor shall be bonded to the equipment grounding conductor only at the Electric Service Installation. All power cables shall include one neutral conductor of the same size.
- b) The equipment grounding conductor shall be green color coded. The following is in addition to Article 801.04 of the Standard Specifications.
 - 1) Equipment grounding conductors shall be bonded to the grounded conductor (neutral conductor) only at the Electric Service Installation. The equipment grounding conductor is paid for separately and shall be continuous. The Earth shall not be used as the equipment grounding conductor.
 - 2) Equipment grounding conductors shall be bonded, using a UL Listed grounding connector, to all traffic signal mast arm poles, traffic signal posts, pedestrian posts, pull boxes, handhole frames and covers, conduits, and other metallic enclosures throughout the traffic signal wiring system, except where noted herein. Bonding shall be made with a splice and pigtail connection, using a sized compression type copper sleeve, sealant tape, and heat-shrinkable cap. A UL listed electrical joint compound shall be applied to all conductors' terminations, connector threads and contact points. Conduit grounding bushings shall be installed at all conduit terminations including spare or empty conduits.
 - 3) All metallic and non-metallic raceways shall have a continuous equipment grounding conductor, except raceways containing only detector loop lead-in circuits, circuits under 50 volts and/or fiber optic cable will not be required to include an equipment grounding conductor.

4. Individual conductor splices in handholes shall be soldered and sealed with heat shrink. When necessary to maintain effective equipment grounding, a full cable heat shrink shall be provided over individual conductor heat shrinks.
- c) The grounding electrode conductor shall be similar to the equipment grounding conductor in color coding (green) and size. The grounding electrode conductor is used to connect the ground rod to the equipment grounding conductor and is bonded to ground rods via exothermic welding, UL listed pressure connectors, and UL listed clamps .

COILABLE NON-METALLIC CONDUIT

Effective: May 22, 2002

Revised: July 1, 2015

810.01TS

Description.

This work shall consist of furnishing and installing empty coilable non-metallic conduit (CNC).

General.

The CNC installation shall be in accordance with Sections 810 and 811 of the Standard Specifications except for the following:

Add the following to Article 810.03 of the Standard Specifications:

CNC meeting the requirements of NEC Article 353 shall be used for detector loop raceways to the handholes.

Add the following to Article 811.03 of the Standard Specifications:

On temporary traffic signal installations with detector loops, CNC meeting the requirements of NEC Article 353 shall be used for detector loop raceways from the saw-cut to 10 feet (3m) up the wood pole, unless otherwise shown on the plans

Basis of Payment.

All installations of CNC for loop detection shall be included in the contract and not paid for separately.

UNDERGROUND RACEWAYS

Effective: May 22, 2002

Revised: July 1, 2015

810.02TS

Revise Article 810.04 of the Standard Specifications to read:

“Installation. All underground conduits shall have a minimum depth of 30-inches (700 mm) below the finished grade.”

Add the following to Article 810.04 of the Standard Specifications:

“All metal conduit installed underground shall be Rigid Steel Conduit unless otherwise indicated on the plans.”

Add the following to Article 810.04 of the Standard Specifications:

“All raceways which extend outside of a structure or duct bank but are not terminated in a cabinet, junction box, pull box, handhole, post, pole, or pedestal shall extend a minimum of 300 mm (12”) or the length shown on the plans beyond the structure or duct bank. The end of this extension shall be capped and sealed with a cap designed for the conduit to be capped.

The ends of rigid metal conduit to be capped shall be threaded, the threads protected with full galvanizing, and capped with a threaded galvanized steel cap.

The ends of rigid nonmetallic conduit and coilaible nonmetallic conduit shall be capped with a rigid PVC cap of not less than 3 mm (0.125”) thick. The cap shall be sealed to the conduit using a room-temperature-vulcanizing (RTV) sealant compatible with the material of both the cap and the conduit. A washer or similar metal ring shall be glued to the inside center of the cap with epoxy, and the pull cord shall be tied to this ring.”

HANDHOLES

Effective: January 01, 2002

Revised: July 1, 2015

814.01TS

Description

Add the following to Section 814 of the Standard Specifications:

All conduits shall enter the handhole at a depth of 30 inches (762 mm) except for the conduits for detector loops when the handhole is less than 5 feet (1.52 m) from the detector loop. All conduit ends should be sealed with a waterproof sealant to prevent the entrance of contaminants into the handhole.

Steel cable hooks shall be coated with hot-dipped galvanization in accordance with AASHTO Specification M111. Hooks shall be a minimum of 1/2 inch (13 mm) diameter with two 90 degree bends and extend into the handhole at least 6 inches (152 mm). Hooks shall be placed a minimum of 12 inches (305 mm) below the lid or lower if additional space is required.

Precast round handholes shall not be used unless called out on the plans.

The cover of the handhole frame shall be labeled “Traffic Signals” with legible raised letters.

Revise the third paragraph of Article 814.03 of the Standard Specifications to read:

“Handholes shall be constructed as shown on the plans and shall be cast-in-place, or precast concrete units. Heavy duty handholes shall be either cast-in-place or precast concrete units.”

Add the following to Article 814.03 of the Standard Specifications:

“(c) Precast Concrete. Precast concrete handholes shall be fabricated according to Article 1042.17. Where a handhole is contiguous to a sidewalk, preformed joint filler of 1/2 inch (13 mm) thickness shall be placed between the handhole and the sidewalk.”

Cast-In-Place Handholes.

All cast-in-place handholes shall be concrete, with inside dimensions of 21-1/2 inches (546 mm) minimum. Frames and lid openings shall match this dimension.

For grounding purposes the handhole frame shall have provisions for a 7/16 inch (11 mm) diameter stainless steel bolt cast into the frame. The covers shall have a stainless steel threaded stint extended from the eye hook assembly for the purpose of attaching the grounding conductor to the handhole cover.

The minimum wall thickness for heavy duty hand holes shall be 12 inches (305mm).

Precast Round Handholes.

All precast handholes shall be concrete, with inside dimensions of 30 inches (762mm) diameter. Frames and covers shall have a minimum opening of 26 inches (660mm) and no larger than the inside diameter of the handhole.

For grounding purposes the handhole frame shall have provisions for a 7/16 inch (11 mm) diameter stainless steel bolt cast into the frame. For the purpose of attaching the grounding conductor to the handhole cover, the covers shall either have a 7/16 inch (11 mm) diameter stainless steel bolt cast into the cover or a stainless steel threaded stint extended from an eye hook assembly. A hole may be drilled for the bolt if one cannot be cast into the frame or cover. The head of the bolt shall be flush or lower than the top surface of the cover.

The minimum wall thickness for precast heavy duty hand holes shall be 6 inches (152 mm).

Precast round handholes shall be only produced by an approved precast vendor.

Materials.

Add the following to Section 1042 of the Standard Specifications:

“1042.17 Precast Concrete Handholes. Precast concrete handholes shall be according to Articles 1042.03(a)(c)(d)(e).”

GROUNDING CABLE

Effective: May 22, 2002
Revised: July 1, 2015
817.01TS

The cable shall meet the requirements of Section 817 of the "Standard Specifications," except for the following:

Add the following to Article 817.02 (b) of the Standard Specifications:

Unless otherwise noted on the Plans, traffic signal grounding conductor shall be one conductor, #6 gauge copper, with a green color coded XLP jacket.

The traffic signal grounding conductor shall be bonded, using a UL Listed grounding connector to all proposed and existing traffic signal mast arm poles and traffic/pedestrian signal posts, including push button posts. The grounding conductor shall be bonded to all proposed and existing pull boxes, handhole frames and covers and other metallic enclosures throughout the traffic signal wiring system and noted herein and detailed on the plans. The grounding conductor shall be bonded to conduit terminations using rated grounding bushings. Bonding to existing handhole frames and covers shall be paid for separately.

Add the following to Article 817.05 of the Standard Specifications:

Basis of Payment.

Grounding cable shall be measured in place for payment in foot (meter). Payment shall be at the contract unit price for ELECTRIC CABLE IN CONDUIT, EQUIPMENT GROUNDING CONDUCTOR, NO. 6 1C, which price includes all associated labor and material including grounding clamps, splicing, exothermic welds, grounding connectors, conduit grounding bushings, and other hardware.

FIBER OPTIC TRACER CABLE

Effective: May 22, 2002

Revised: July 1, 2015

817.02TS

The cable shall meet the requirements of Section 817 of the Standard Specifications, except for the following:

Add the following to Article 817.03 of the Standard Specifications:

In order to trace the fiber optic cable after installation, the tracer cable shall be installed in the same conduit as the fiber optic cable in locations shown on the plans. The tracer cable shall be continuous, extended into the controller cabinet and terminated on a barrier type terminal strip mounted on the side wall of the controller cabinet. The barrier type terminal strip and tracer cable shall be clearly marked and identified. All tracer cable splices shall be kept to a minimum and shall incorporate maximum lengths of cable supplied by the manufacturer. The tracer cable will be allowed to be spliced at handholes only. The tracer cable splice shall use a Western Union Splice soldered with resin core flux and shall be soldered using a soldering iron. Blow torches or other devices which oxidize copper cable shall not be allowed for soldering operations. All exposed surfaces of the solder shall be smooth. The splice shall be covered with a black shrink tube meeting UL 224 guidelines, Type V and rated 600V, minimum length 4 inches (100 mm) and with a minimum 1 inch (25 mm) coverage over the XLP insulation, underwater grade.

Add the following to Article 817.05 of the Standard Specifications:

Basis of Payment.

The tracer cable shall be paid for separately as ELECTRIC CABLE IN CONDUIT, TRACER, NO. 14 1C per foot (meter), which price shall include all associated labor and material for installation.

FULL-ACTUATED CONTROLLER AND CABINET

Effective: January 1, 2002

Revised: July 1, 2015

857.02TS

Description.

This work shall consist of furnishing and installing a traffic actuated solid state digital controller in the controller cabinet of the type specified, meeting the requirements of Section 857 of the Standard Specifications, as modified herein, including malfunction management unit, load switches and flasher relays, with all necessary connections for proper operation.

If the intersection is part of an existing system and/or when specified in the plans, this work shall consist of furnishing and installing a(n) " _____ " brand traffic actuated solid state controller.

Materials.

Add the following to Article 857.02 of the Standard Specifications:

For installation as a stand-alone traffic signal, connected to a closed loop system or integrated into an advance traffic management system (ATMS), controllers shall be Econolite ASC/3S-1000 or Eagle/Siemens M52 unless specified otherwise on the plans or elsewhere on these specifications. Only controllers supplied by one of the District One approved closed loop equipment suppliers will be allowed. Unless specified otherwise on the plans or these specifications, the controller shall be of the most recent model and software version supplied by the equipment supplier at the time of the traffic signal TURN-ON. A removable controller data key shall also be provided. Individual load switches shall be provided for each vehicle, pedestrian, and right turn over lap phase. The controller shall prevent phases from being skipped during program changes and after all preemption events and shall inhibit simultaneous display of circular yellow and yellow arrow indications.

For integration into an ATMS such as Centrac, Tactics, or TransSuite, the controller shall have the latest version of NTCIP software installed. For operation prior to integration into an ATMS, the controller shall maintain existing close loop management communications.

Add the following to Article 1074.03 of the Standard Specifications:

- (a) (6) Cabinets shall be designed for NEMA TS2 Type 1 operation. All cabinets shall be pre-wired for a minimum of eight (8) phases of vehicular, four (4) phases of pedestrian and four (4) phases of overlap operation.
- (b) (1) Revise "conflict monitor" to read "Malfunction Management Unit"
- (b) (5) Cabinets – Provide 1/8" (3.2 mm) thick unpainted aluminum alloy 5052-H32. The surface shall be smooth, free of marks and scratches. All external hardware shall be stainless steel.
- (b) (6) Controller Harness – Provide a TS2 Type 2 "A" wired harness in addition to the TS2 Type 1 harness.

- (b) (7) Surge Protection – Shall be a 120VAC Single phase Modular filter Plug-in type, supplied from an approved vendor.
- (b) (8) BIU – shall be secured by mechanical means.
- (b) (9) Transfer Relays – Solid state or mechanical flash relays are acceptable.
- (b) (10) Switch Guards – All switches shall be guarded.
- (b) (11) Heating – One (1) 200 watt, thermostatically-controlled, electric heater.
- (b) (12) Lighting – One (1) LED Panel shall be placed inside the cabinet top panel and one (1) LED Panel shall be placed on each side of the pull-out drawer/shelf assembly located beneath the controller support shelf. The LED Panels shall be controlled by a door switch. The LED Panels shall be provided from an approved vendor.
- (b) (13) The cabinet shall be equipped with a pull-out drawer/shelf assembly. A 1 ½ inch (38mm) deep drawer shall be provided in the cabinet, mounted directly beneath the controller support shelf. The drawer shall have a hinged top cover and shall be capable of accommodating one (1) complete set of cabinet prints and manuals. This drawer shall support 50 lbs. (23 kg) in weight when fully extended. The drawer shall open and close smoothly. Drawer dimensions shall make maximum use of available depth offered by the controller shelf and be a minimum of 18 inches (610mm) wide.
- (b) (14) Plan & Wiring Diagrams – 12” x 15” (305mm x 406mm) moisture sealed container attached to door.
- (b) (15) Detector Racks – Fully wired and labeled for four (4) channels of emergency vehicle pre-emption and sixteen channels (16) of vehicular operation.
- (b) (16) Field Wiring Labels – All field wiring shall be labeled.
- (b) (17) Field Wiring Termination – Approved channel lugs required.
- (b) (18) Power Panel – Provide a nonconductive shield.
- (b) (19) Circuit Breaker – The circuit breaker shall be sized for the proposed load but shall not be rated less than 30 amps.
- (b) (20) Police Door – Provide wiring and termination for plug in manual phase advance switch.

Basis of Payment.

This work will be paid for at the contract unit price each for FULL-ACTUATED CONTROLLER AND TYPE IV CABINET; FULL-ACTUATED CONTROLLER AND TYPE V CABINET; FULL-ACTUATED CONTROLLER AND TYPE SUPER P CABINET; FULL-ACTUATED CONTROLLER AND TYPE SUPER R CABINET; FULL-ACTUATED CONTROLLER AND TYPE IV CABINET, SPECIAL; FULL-ACTUATED CONTROLLER AND TYPE V CABINET, SPECIAL; FULL-ACTUATED CONTROLLER AND TYPE SUPER P CABINET (SPECIAL); FULL-ACTUATED CONTROLLER AND TYPE SUPER R CABINET (SPECIAL).

MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION.

Revise Articles 850.02 and 850.03 of the Standard Specifications to read:

Procedure.

The energy charges for the operation of the traffic signal installation shall be paid for by others. Full maintenance responsibility shall start as soon as the Contractor begins any physical work on the Contract or any portion thereof.

The Contractor shall have electricians with IMSA Level II certification on staff to provide signal maintenance.

This item shall include maintenance of all traffic signal equipment at the intersection, including emergency vehicle pre-emption equipment, master controllers, uninterruptible power supply (UPS and batteries), telephone service installations, communication cables, conduits to adjacent intersections, and other traffic signal equipment, but shall not include Automatic Traffic Enforcement equipment, such as Red Light Enforcement cameras, detectors, or peripheral equipment, not owned by the State.

Maintenance.

The maintenance shall be according to MAINTENANCE AND RESPONSIBILITY in Division 800 of these specifications and the following:

The Contractor shall check all controllers every two (2) weeks, which will include visually inspecting all timing intervals, relays, detectors, and pre-emption equipment to ensure that they are functioning properly. This item includes, as routine maintenance, all portions of emergency vehicle pre-emption equipment. The Contractor shall maintain in stock at all times a sufficient amount of materials and equipment to provide effective temporary and permanent repairs.

The Contractor shall provide immediate corrective action when any part or parts of the system fail to function properly. Two far side heads facing each approach shall be considered the minimum acceptable signal operation pending permanent repairs. When repairs at a signalized intersection require that the controller be disconnected or otherwise removed from normal operation, and power is available, the Contractor shall place the traffic signal installation on flashing operation. The signals shall flash RED for all directions unless a different indication has been specified by the Engineer. The Contractor shall be required to place stop signs (R1-1-36) at each approach of the intersection as a temporary means of regulating traffic. When the signals operate in flash, the Contractor shall furnish and equip all their vehicles assigned to the maintenance of traffic signal installations with a sufficient number of stop signs as specified herein. The Contractor shall maintain a sufficient number of spare stop signs in stock at all times to replace stop signs which may be damaged or stolen.

The Contractor shall provide the Engineer with a 24 hour telephone number for the maintenance of the traffic signal installation and for emergency calls by the Engineer.

Traffic signal equipment which is lost or not returned to the Department for any reason shall be replaced with new equipment meeting the requirements of the Standard Specifications and these special provisions.

The Contractor shall respond to all emergency calls from the Department or others within one hour after notification and provide immediate corrective action. When equipment has been damaged or becomes faulty beyond repair, the Contractor shall replace it with new and identical equipment. The cost of furnishing and installing the replaced equipment shall be borne by the Contractor at no additional charge to the contract. The Contractor may institute action to recover damages from a responsible third party. If at any time the Contractor fails to perform all work as specified herein to keep the traffic signal installation in proper operating condition or if the Engineer cannot contact the Contractor's designated personnel, the Engineer shall have the State's Electrical Maintenance Contractor perform the maintenance work required. The State's Electrical Maintenance Contractor shall bill the Contractor for the total cost of the work. The Contractor shall pay this bill within thirty (30) days of the date of receipt of the invoice or the cost of such work will be deducted from the amount due the Contractor. The Contractor shall allow the Electrical Maintenance Contractor to make reviews of the Existing Traffic Signal Installation that has been transferred to the Contractor for Maintenance.

UNINTERRUPTABLE POWER SUPPLY, SPECIAL

Effective: January 1, 2013

Revised: July 1, 2015

862.01TS

This work shall be in accordance with section 862 of the Standard Specification except as modified herein

Add the following to Article 862.01 of the Standard Specifications:

The UPS shall have the power capacity to provide normal operation of a signalized intersection that utilizes all LED type signal head optics, for a minimum of 10 (ten) hours.

Add the following to Article 862.02 of the Standard Specifications:

Materials shall be according to Article 1074.04 as modified in UNINTERRUPTABLE POWER SUPPLY, SPECIAL.

Add the following to Article 862.03 of the Standard Specifications:

The UPS shall additionally include, but not be limited to, a battery cabinet, where applicable. For Super-P (Type IV) and Super-R (Type V) cabinets, the battery cabinet is integrated to the traffic signal cabinet, and shall be included in the cost for the traffic signal cabinet of the size and type indicated on the plans.

The UPS shall provide reliable emergency power to the traffic signals in the event of a power failure or interruption.

Revise Article 862.04 of the Standard Specifications to read:

Installation.

When a UPS is installed at an existing traffic signal cabinet, the UPS cabinet shall partially rest on the lip of the existing controller cabinet foundation and be secured to the existing controller cabinet by means of at least four (4) stainless steel bolts. The UPS cabinet shall be completely enclosed with the bottom and back constructed of the same material as the cabinet.

When a UPS is installed at a new signal cabinet and foundation, it shall be mounted as shown on the plans.

At locations where UPS is installed and an Emergency Vehicle Priority System is in use, any existing incandescent confirmation beacons shall be replaced with LED lamps in accordance with the District One Emergency Vehicle Priority System specification at no additional cost to the contract. A concrete apron shall be provided and be in accordance with Articles 424 and 202 of the Standard Specifications. The concrete apron shall also, follow the District 1 Standard Traffic Signal Design Detail, Type D for Ground Mounted Controller Cabinet and UPS Battery Cabinet.

This item shall include any required modifications to an existing traffic signal controller as a result of the addition of the UPS including the addition of alarms.

Materials.

Revise Article 1074.04(a)(1) of the Standard Specifications to read:

The UPS shall be line interactive or double conversion and provide voltage regulation and power conditioning when utilizing utility power. The UPS shall be sized appropriately for the intersection(s) normal traffic signal operating load. The UPS must be able to maintain the intersection's normal operating load plus 20 percent (20%) of the intersection's normal operating load. When installed at a railroad-interconnected intersection the UPS must maintain the railroad pre-emption load, plus 20 percent (20%) of the railroad preemption-operating load. The total connected traffic signal load shall not exceed the published ratings for the UPS.

The UPS shall provide a minimum of 10 (ten) hours of normal operation run-time for signalized intersections with LED type signal head optics at 77 °F (25 °C) (minimum 1000 W active output capacity, with 86 percent minimum inverter efficiency).

Revise the first paragraph of Article 1074.04(a)(3) of the Standard Specifications to read:

The UPS shall have a minimum of four (4) sets of normally open (NO) and normally closed (NC) single-pole double-throw (SPDT) relay contact closures, available on a panel mounted terminal block or locking circular connectors, rated at a minimum 120 V/1 A, and labeled so as to identify each contact according to the plans.

Revise Article 1074.04(a)(10) of the Standard Specifications to read:

The UPS shall be compatible with the District's approved traffic controller assemblies utilizing NEMA TS 1 or NEMA TS 2 controllers and cabinet components for full time operation.

Revise Article 1074.04(a)(17) of the Standard Specifications to read:

When the intersection is in battery backup mode, the UPS shall bypass all internal cabinet lights, ventilation fans, cabinet heaters, service receptacles, any lighted street name signs, any automated enforcement equipment and any other devices directed by the Engineer.

Revise Article 1074.04(b)(2)b of the Standard Specifications to read:

Batteries, inverter/charger and power transfer relay shall be housed in a separate NEMA Type 3R cabinet. The cabinet shall be Aluminum alloy, 5052-H32, 0.125-inch thick and have a natural mill finish.

Revise Article 1074.04(b)(2)c of the Standard Specifications to read:

No more than three batteries shall be mounted on individual shelves for a cabinet housing six batteries and no more than four batteries per shelf for a cabinet housing eight batteries.

Revise Article 1074.04(b)(2)e of the Standard Specifications to read:

The battery cabinet housing shall have the following nominal outside dimensions: a width of 25 in. (785 mm), a depth of 16 in. (440 mm), and a height of 41 to 48 in. (1.1 to 1.3 m). Clearance between shelves shall be a minimum of 10 in. (250 mm).

End of paragraph 1074.04(b)(2)e

The door shall be equipped with a two position doorstop, one a 90° and one at 120°.

Revise Article 1074.04(b)(2)g of the Standard Specifications to read:

The door shall open to the entire cabinet, have a neoprene gasket, an Aluminum continuous piano hinge with stainless steel pin, and a three point locking system. The cabinet shall be provided with a main door lock which shall operate with a traffic industry conventional No. 2 key. Provisions for padlocking the door shall be provided.

Add the following to Article 1074.04(b)(2) of the Standard Specifications:

j. The battery cabinet shall have provisions for an external generator connection.

Add the following to Article 1074.04(c) of the Standard Specifications:

- (8) The UPS shall include a tip or kill switch installed in the battery cabinet, which shall completely disconnect power from the UPS when the switch is manually activated.
- (9) The UPS shall include standard RS-232 and internal Ethernet interface.
- (10) The UPS shall incorporate a flanged electric generator inlet for charging the batteries and operating the UPS. The generator connector shall be male type, twist-lock, rated as 15A, 125VAC with a NEMA L5-15P configuration and weatherproof lift cover plate. Access to the generator inlet shall be from a secured weatherproof lift cover plate or behind a locked battery cabinet police panel.
- (11) The bypass switch shall include an internal power transfer relay that allows removal of the battery back-up unit, while the traffic signal is connected to utility power, without impacting normal traffic signal operation.

Revise Article 1074.04(d)(3) of the Standard Specifications to read:

All batteries supplied in the UPS shall be either gel cell or AGM type, deep cycle, completely sealed, prismatic lead calcium based, silver alloy, valve regulated lead acid (VRLA) requiring no maintenance. All batteries in a UPS installation shall be the same type; mixing of gel cell and AGM types within a UPS installation is not permitted.

Revise Article 1074.04(d)(4) of the Standard Specifications to read:

Batteries shall be certified by the manufacturer to operate over a temperature range of -13 to 160 °F (-25 to + 71 °C) for gel cell batteries and -40 to 140 °F (-40 to + 60 °C) for AGM type batteries.

Add the following to Article 1074.04(d) of the Standard Specifications:

(9) The UPS shall consist of an even number of batteries that are capable of maintaining normal operation of the signalized intersection for a minimum of 10 (ten) hours. Calculations shall be provided showing the number of batteries of the type supplied that are needed to satisfy this requirement. A minimum of four batteries shall be provided.

(10) Battery Heater mats shall be provided, when gel cell type batteries are supplied.

Add the following to the Article 1074.04 of the Standard Specifications:

(e) Warranty. The warranty for an uninterruptable power supply (UPS) and batteries (full replacement) shall cover a minimum of 5 years from date the equipment is placed in operation.

(f) Installation. Bypass switch shall completely disconnect the traffic signal cabinet from the utility provider.

(g) The UPS shall be set-up to run the traffic signal continuously, without going to a red flashing condition, when switched to battery power unless otherwise directed by the Engineer. The Contractor shall confirm set-up with the Engineer. The continuous operation mode when switched to battery may require modification to unit connections and these modifications are included in the unit price for this item.

Revise Article 862.05 of the Standard Specifications to read:

Basis of Payment.

This work will be paid for at the contract unit price per each for UNINTERRUPTABLE POWER SUPPLY, SPECIAL or UNINTERRUPTABLE POWER SUPPLY AND CABINET, SPECIAL. Replacement of Emergency Vehicle Priority System confirmation beacons and any required modifications to the traffic signal controller shall be included in the cost of the UNINTERRUPTABLE POWER SUPPLY, SPECIAL or UNINTERRUPTABLE POWER SUPPLY AND CABINET, SPECIAL item. The concrete apron and earth excavation required shall be included in the cost of the UNINTERRUPTABLE POWER SUPPLY AND CABINET, SPECIAL item.

ELECTRIC CABLE

Effective: May 22, 2002

Revised: July 1, 2015

873.01TS

Delete “or stranded, and No. 12 or” from the last sentence of Article 1076.04 (a) of the Standard Specifications.

Add the following to the Article 1076.04(d) of the Standard Specifications:

Service cable may be single or multiple conductor cable.

EMERGENCY VEHICLE PRIORITY SYSTEM LINE SENSOR CABLE, NO. 20 3/C

Effective: January 1, 2013

Revised: July 1, 2015

873.03TS

This work shall consist of furnishing and installing lead-in cable for light detectors installed at existing and/or proposed traffic signal installations as part of an emergency vehicle priority system. The work includes installation of the lead-in cables in existing and/or new conduit. The electric cable shall be shielded and have (3) stranded conductors, colored blue, orange, and yellow with a stranded tinned copper drain wire. The cable shall meet the requirements of the vendor of the Emergency Vehicle Priority System Equipment.

Basis of Payment.

This work will be paid for at the contract unit price per foot for EMERGENCY VEHICLE PRIORITY SYSTEM LINE SENSOR CABLE, NO. 20 3/C, which price shall be payment in full for furnishing, installing and making all electrical connections necessary for proper operations.

TRAFFIC SIGNAL POST

Effective: May 22, 2002

Revised: July 01, 2015

875.01TS

Add the following to Article 1077.01 (c) of the Standard Specifications:

Washers for post bases shall be the same size or larger than the nut.

Revise the first sentence of Article 1077.01 (d) of the Standard Specifications to read:

All posts and bases shall be steel and hot dipped galvanized according to AASHTO M 111. If the Department approves painting, powder coating by the manufacturer will be required over the galvanization in accordance with 851.01TS TRAFFIC SIGNAL PAINTING Special Provisions.

MAST ARM ASSEMBLY AND POLE

Effective: May 22, 2002

Revised: July 01, 2015

877.01TS

Revise the second sentence of Article 1077.03 (a)(3) of the Standard Specifications to read:

Traffic signal mast arms shall be one piece construction, unless otherwise approved by the Engineer.

Add the following to Article 1077.03 (a)(3) of the Standard Specifications:

If the Department approves painting, powder coating by the manufacturer will be required over the galvanization in accordance with 851.01TS TRAFFIC SIGNAL PAINTING Special Provisions.

CONCRETE FOUNDATIONS

Effective: May 22, 2002

Revised: July 01, 2015

878.01TS

Add the following to Article 878.03 of the Standard Specifications:

All anchor bolts shall be according to Article 1006.09, with all anchor bolts hot dipped galvanized a minimum of 12 in. (300 mm) at the threaded end.

Foundations used for Combination Mast Arm Poles shall provide an extra 2-1/2 inch (65 mm) raceway.

No foundation is to be poured until the Resident Engineer gives his/her approval as to the depth of the foundation.

Add the following to the first paragraph of Article 878.05 of the Standard Specifications:

The price shall include a concrete apron in front of the cabinet and UPS as shown in the plans or as directed by the engineer.

LIGHT EMITTING DIODE (LED) SIGNAL HEAD AND OPTICALLY PROGRAMMED LED SIGNAL HEAD

Effective: May 22, 2002

Revised: July 1, 2015

880.01TS

Materials.

Add the following to Section 1078 of the Standard Specifications:

1. LED modules proposed for use and not previously approved by IDOT District One will require independent testing for compliance to current VTCSH-ITE standards for the product and be Intertek ETL Verified. This would include modules from new vendors and new models from IDOT District One approved vendors.
2. The proposed independent testing facility shall be approved by IDOT District One. Independent testing must include a minimum of two (2) randomly selected modules of each type of module (i.e. ball, arrow, pedestrian, etc.) used in the District and include as a minimum Luminous Intensity and Chromaticity tests. However, complete module performance verification testing may be required by the Engineer to assure the accuracy of the vendor's published data and previous test results. An IDOT representative will select sample modules from the local warehouse and mark the modules for testing. Independent test results shall meet current ITE standards and vendor's published data. Any module failures shall require retesting of the module type. All costs associated with the selection of sample modules, testing, reporting, and retesting, if applicable, shall be the responsibility of the LED module vendor and not be a cost to this contract.
3. All signal heads shall provide 12" (300 mm) displays with glossy yellow or black polycarbonate housings. All head housings shall be the same color (yellow or black) at the

intersection. For new signalized intersections and existing signalized intersections where all signals heads are being replaced, the proposed head housings shall be black. Where only selected heads are being replaced, the proposed head housing color (yellow or black) shall match existing head housings. Connecting hardware and mounting brackets shall be polycarbonate (black). A corrosion resistant anti-seize lubricant shall be applied to all metallic mounting bracket joints, and shall be visible to the inspector at the signal turn-on. Post top mounting collars are required on all posts, and shall be constructed of the same material as the brackets.

4. The LED signal modules shall be replaced or repaired if an LED signal module fails to function as intended due to workmanship or material defects within the first 7 years from the date of traffic signal TURN-ON. LED signal modules which exhibit luminous intensities less than the minimum values specified in Table 1 of the ITE Vehicle Traffic Control Signal Heads: Light Emitting Diode (LED) Circular Signal Supplement (June 27, 2005) [VTSCH], or applicable successor ITE specifications, or show signs of entrance of moisture or contaminants within the first 7 years of the date of traffic signal TURN-ON shall be replaced or repaired. The vendor's written warranty for the LED signal modules shall be dated, signed by a vendor's representative and included in the product submittal to the State.

(a) Physical and Mechanical Requirements

1. Modules can be manufactured under this specification for the following faces:
 - a. 12 inch (300 mm) circular, multi-section
 - b. 12 inch (300 mm) arrow, multi-section
2. The maximum weight of a module shall be 4 lbs. (1.8 kg).
3. Each module shall be a sealed unit to include all parts necessary for operation (a printed circuit board, power supply, a lens and gasket, etc.), and shall be weather proof after installation and connection.
5. The lens of the module shall be tinted with a wavelength-matched color to reduce sun phantom effect and enhance on/off contrast. The tinting shall be uniform across the lens face. Polymeric lens shall provide a surface coating or chemical surface treatment applied to provide abrasion resistance. The lens of the module shall be integral to the unit, convex with a smooth outer surface and made of plastic. The lens shall have a textured surface to reduce glare.
6. The use of tinting or other materials to enhance ON/OFF contrasts shall not affect chromaticity and shall be uniform across the face of the lens.
7. Each module shall have a symbol of the type of module (i.e. circle, arrow, etc.) in the color of the module. The symbol shall be 1 inch (25.4 mm) in diameter. Additionally, the color shall be written out in 1/2 inch (12.7mm) letters next to the symbol.

(b) Photometric Requirements

4. The LEDs utilized in the modules shall be AlInGaP technology for red and InGaN for green and amber indications, and shall be the ultra bright type rated for 100,000 hours of continuous operation from -40 °C to +74 °C.

(c) Electrical

1. Maximum power consumption for LED modules is per Table 2.
2. Operating voltage of the modules shall be 120 VAC. All parameters shall be measured at this voltage.
3. The modules shall be operationally compatible with currently used controller assemblies (solid state load switches, flashers, and conflict monitors).
4. When a current of 20 mA AC (or less) is applied to the unit, the voltage read across the two leads shall be 15 VAC or less.
5. The LED modules shall provide constant light output under power. Modules with dimming capabilities shall have the option disabled or set on a non-dimming operation.
6. LED arrows shall be wired such that a catastrophic loss or the failure of one or more LED will not result in the loss of the entire module.

(d) Retrofit Traffic Signal Module

1. The following specification requirements apply to the Retrofit module only. All general specifications apply unless specifically superseded in this section.
2. Retrofit modules can be manufactured under this specification for the following faces:
 - a. 12 inch (300 mm) circular, multi-section
 - b. 12 inch (300 mm) arrow, multi-section
3. Each Retrofit module shall be designed to be installed in the doorframe of a standard traffic signal housing. The Retrofit module shall be sealed in the doorframe with a one-piece EPDM (ethylene propylene rubber) gasket.
4. The maximum weight of a Retrofit module shall be 4 lbs. (1.8 kg).
5. Each Retrofit module shall be a sealed unit to include all parts necessary for operation (a printed circuit board, power supply, a lens and gasket, etc.), and shall be weather proof after installation and connection.
6. Electrical conductors for modules, including Retrofit modules, shall be 39.4 inches (1m) in length, with quick disconnect terminals attached.
7. The lens of the Retrofit module shall be integral to the unit, shall be convex with a smooth outer surface and made of plastic or of glass.

- (e) The following specification requirements apply to the 12 inch (300 mm) arrow module only. All general specifications apply unless specifically superseded in this section.
 - 1. The arrow module shall meet specifications stated in Section 9.01 of the Equipment and Material Standards of the Institute of Transportation Engineers (November 1998) [ITE Standards], Chapter 2 (Vehicle Traffic Control Signal Heads) or applicable successor ITE specifications for arrow indications.
 - 2. The LEDs arrow indication shall be a solid display with a minimum of three (3) outlining rows of LEDs and at least one (1) fill row of LEDs.
- (f) The following specification requirement applies to the 12 inch (300 mm) programmed visibility (PV) module only. All general specifications apply unless specifically superseded in this section.
 - 1. The LED module shall be a module designed and constructed to be installed in a programmed visibility (PV) signal housing without modification to the housing.

Basis of Payment.

Add the following to the first paragraph of Article 880.04 of the Standard Specifications:

The price shall include furnishing the equipment described above, all mounting hardware and installing them in satisfactory operating condition.

Revise the second paragraph of Article 880.04 of the Standard Specifications to read:

If the work consists of retrofitting an existing polycarbonate traffic signal head with light emitting diodes (LEDs), it will be paid for as a SIGNAL HEAD, LED, RETROFIT, of the type specified, and of the particular kind of material, when specified. Price shall be payment in full for removal of the existing module, furnishing the equipment described above including LED modules, all mounting hardware, and installing them in satisfactory operating condition. The type specified will indicate the number of signal faces, the number of signal sections in each signal face and the method of mounting.

LIGHT EMITTING DIODE (LED) PEDESTRIAN SIGNAL HEAD

Effective: May 22, 2002

Revised: July 1, 2015

881.01TS

Add the following to the third paragraph of Article 881.03 of the Standard Specifications:

No mixing of different types of pedestrian traffic signals or displays will be permitted.

Add the following to Article 881.03 of the Standard Specifications:

(a) Pedestrian Countdown Signal Heads.

- (1) Pedestrian Countdown Signal Heads shall not be installed at signalized intersections where traffic signals and railroad warning devices are interconnected.

- (2) Pedestrian Countdown Signal Heads shall be 16 inch (406mm) x 18 inch (457mm), for single units with glossy yellow or black polycarbonate housings. All pedestrian head housings shall be the same color (yellow or black) at the intersection. For new signalized intersections and existing signalized intersections where all pedestrian heads are being replaced, the proposed head housings shall be black. Where only selected heads are being replaced, the proposed head housing color (yellow or black) shall match existing head housings. Connecting hardware and mounting brackets shall be polycarbonate (black). A corrosion resistant anti-seize lubricant shall be applied to all metallic mounting bracket joints, and shall be visible to the inspector at the signal turn-on.
- (3) Each pedestrian signal LED module shall be fully MUTCD compliant and shall consist of double overlay message combining full LED symbols of an Upraised Hand and a Walking Person. "Egg Crate" type sun shields are not permitted. Numerals shall measure 9 inches (229mm) in height and easily identified from a distance of 120 feet (36.6m).

Materials.

Add the following to Article 1078.02 of the Standard Specifications:

General.

1. The module shall operate in one mode: Clearance Cycle Countdown Mode Only. The countdown module shall display actual controller programmed clearance cycle and shall start counting when the flashing clearance signal turns on and shall countdown to "0" and turn off when the steady Upraised Hand (symbolizing Don't Walk) signal turns on. Module shall not have user accessible switches or controls for modification of cycle.
2. At power on, the module shall enter a single automatic learning cycle. During the automatic learning cycle, the countdown display shall remain dark.
3. The module shall re-program itself if it detects any increase or decrease of Pedestrian Timing. The counting unit will go blank once a change is detected and then take one complete pedestrian cycle (with no counter during this cycle) to adjust its buffer timer.
4. If the controller preempts during the Walking Person (symbolizing Walk), the countdown will follow the controller's directions and will adjust from Walking Person to flashing Upraised Hand. It will start to count down during the flashing Upraised Hand.
5. If the controller preempts during the flashing Upraised Hand, the countdown will continue to count down without interruption.
6. The next cycle, following the preemption event, shall use the correct, initially programmed values.
7. If the controller output displays Upraised Hand steady condition and the unit has not arrived to zero or if both the Upraised Hand and Walking Person are dark for some reason, the unit suspends any timing and the digits will go dark.
8. The digits will go dark for one pedestrian cycle after loss of power of more than 1.5 seconds.

9. The countdown numerals shall be two (2) “7 segment” digits forming the time display utilizing two rows of LEDs.
10. The LED module shall meet the requirements of the Institute of Transportation Engineers (ITE) LED purchase specification, “Pedestrian Traffic Control Signal Indications - Part 2: LED Pedestrian Traffic Signal Modules,” or applicable successor ITE specifications, except as modified herein.
11. The LED modules shall provide constant light output under power. Modules with dimming capabilities shall have the option disabled or set on a non-dimming operation.
12. In the event of a power outage, light output from the LED modules shall cease instantaneously.
13. The LEDs utilized in the modules shall be AlInGaP technology for Portland Orange (Countdown Numerals and Upraised Hand) and GaN technology for Lunar White (Walking Person) indications.
14. The individual LEDs shall be wired such that a catastrophic loss or the failure of one or more LED will not result in the loss of the entire module.

Basis of Payment.

Add the following to the first paragraph of Article 881.04 of the Standard Specifications:

The price shall include furnishing the equipment described above, all mounting hardware and installing them in satisfactory operating condition.

Add the following to Article 881.04 of the Standard Specifications:

If the work consists of retrofitting an existing polycarbonate pedestrian signal head and pedestrian countdown signal head with light emitting diodes (LEDs), it will be paid for as a PEDESTRIAN SIGNAL HEAD, LED, RETROFIT, of the type specified, and of the particular kind of material, when specified. Price shall be payment in full for furnishing the equipment described above including LED modules, all mounting hardware, and installing them in satisfactory operating condition.

TRAFFIC SIGNAL BACKPLATE

Effective: May 22, 2002

Revised: July 1, 2015

882.01TS

Delete 1st sentence of Article 1078.03 of the Standard Specifications and add “All backplates shall be louvered, formed ABS plastic”.

Add the following to the third paragraph of Article 1078.03 of the Standard Specifications. The retroreflective backplate shall not contain louvers.

Delete second sentence of the fourth paragraph of Article 1078.03 the Standard Specifications.

Add the following to the fourth paragraph of Article 1078.03 of the Standard Specifications:

When retro reflective sheeting is specified, it shall be Type ZZ sheeting according to Article 1091.03 and applied in preferred orientation for the maximum angularity according to the vendor's recommendations. The retroreflective sheeting shall be installed under a controlled environment at the vendor/equipment supplier before shipment to the contractor. The formed plastic backplate shall be prepared and cleaned, following recommendations of the retroreflective sheeting manufacturer.

DETECTOR LOOP

Effective: May 22, 2002

Revised: July 1, 2015

886.01TS

Procedure.

A minimum of seven (7) working days prior to the Contractor cutting loops, the Contractor shall mark the proposed loop locations and contact the Area Traffic Signal Maintenance and Operations Engineer (847) 705-4424 to inspect and approve the layout. When preformed detector loops are installed, the Contractor shall have them inspected and approved prior to the pouring of the Portland cement concrete surface, using the same notification process as above.

Installation.

Revise Article 886.04 of the Standard Specifications to read:

Loop detectors shall be installed according to the requirements of the "District One Standard Traffic Signal Design Details." Saw-cuts (homeruns on preformed detector loops) from the loop to the edge of pavement shall be made perpendicular to the edge of pavement when possible in order to minimize the length of the saw-cut (homerun on preformed detector loops) unless directed otherwise by the Engineer or as shown on the plan.

The detector loop cable insulation shall be labeled with the cable specifications.

Each loop detector lead-in wire shall be labeled in the handhole using a water proof tag, from an approved vender, secured to each wire with nylon ties.

Resistance to ground shall be a minimum of 100 mega-ohms under any conditions of weather or moisture. Inductance shall be more than 50 and less than 700 microhenries. Quality readings shall be more than 5.

- (a) Type I. All loops installed in new asphalt pavement shall be installed in the binder course and not in the surface course. The edge of pavement, curb and handhole shall be cut with a 1/4 inch (6.3 mm) deep x 4 inches (100 mm) saw cut to mark location of each loop cable.
- (b) Loop sealant shall be two-component thixotropic chemically cured polyurethane from an approved vender. The sealant shall be installed 1/8 inch (3 mm) below the pavement surface. If installed above the surface the excess shall be removed immediately.
- (c) Preformed. This work shall consist of furnishing and installing a rubberized or cross linked polyethylene heat resistant preformed traffic signal loop in accordance with the Standard Specifications, except for the following:

- (d) Preformed detector loops shall be installed in new pavement constructed of Portland cement concrete using mounting chairs or tied to re-bar or the preformed detector loops may be placed in the sub-base. Loop lead-ins shall be extended to a temporary protective enclosure near the proposed handhole location. The protective enclosure shall provide sufficient protection from other construction activities and may be buried for additional protection.
- (e) Handholes shall be placed next to the shoulder or back of curb when preformed detector loops enter the handhole. CNC, included in this pay item, shall be used to protect the preformed lead-ins from back of curb to the handhole.
- (f) Preformed detector loops shall be factory assembled with ends capped and sealed against moisture and other contaminants. The loop configurations and homerun lengths shall be assembled for the specific application. The loop and homerun shall be constructed using 11/16 inch (17.2 mm) outside diameter (minimum), 3/8 inch (9.5 mm) inside diameter (minimum) Class A oil resistant synthetic cord reinforced hydraulic hose with 250 psi (1,720 kPa) internal pressure rating or a similarly sized XLPE cable jacket. Hose for the loop and homerun assembly shall be one continuous piece. No joints or splices shall be allowed in the hose except where necessary to connect homeruns to the loops. This will provide maximum wire protection and loop system strength. Hose tee connections shall be heavy duty high temperature synthetic rubber. The tee shall be of proper size to attach directly to the hose, minimizing glue joints. The tee shall have the same flexible properties as the hose to insure that the whole assembly can conform to pavement movement and shifting without cracking or breaking. For XLPE jacketed preformed loops, all splice connections shall be soldered, sealed, and tested before being sealed in a high impact glass impregnated plastic splice enclosure. The wire used shall be #16 THWN stranded copper. The number of turns in the loop shall be application specific. Homerun wire pairs shall be twisted a minimum of four turns per foot. No wire splices will be allowed in the preformed loop assembly. The loop and homeruns shall be filled and sealed with a flexible sealant to insure complete moisture blockage and further protect the wire. The preformed loops shall be constructed to allow a minimum of 6.5 feet of extra cable in the handhole.

Method of Measurement.

Add the following to Article 886.05 of the Standard Specifications:

Preformed detector loops will be measured along the detector loop embedded in the pavement, rather than the actual length of the wire. Detector loop measurements shall include the saw cut and the length of the detector loop wire to the edge of pavement. The detector loop wire, including all necessary connections for proper operations, from the edge of pavement to the handhole, shall be included in the price of the detector loop. Unit duct, trench and backfill, and drilling of pavement or handholes shall be included in detector loop quantities.

Basis of Payment.

This work shall be paid for at the contract unit price per foot (meter) for DETECTOR LOOP, TYPE I or PREFORMED DETECTOR LOOP as specified in the plans, which price shall be payment in full for furnishing and installing the detector loop and all related connections for proper operation.

EMERGENCY VEHICLE PRIORITY SYSTEM

Effective: May 22, 2002

Revised: July 1, 2015

887.01TS

Revise Section 887 of the Standard Specifications to read:

It shall be the Contractor's responsibility to contact the municipality or fire district to verify the brand of emergency vehicle pre-emption equipment to be installed prior to the contract bidding. The equipment must be completely compatible with all components of the equipment currently in use by the Agency.

All new installations shall be equipped with Confirmation Beacons as shown on the "District One Standard Traffic Signal Design Details." The Confirmation Beacon shall consist of a 6 watt Par 38 LED flood lamp with a 30 degree light spread, or a 7 watt Par 30 LED flood lamp with a 15 degree or greater spread, maximum 7 watt energy consumption at 120V, and a 2,000 hour warranty for each direction of pre-emption. The lamp shall have an adjustable mount with a weatherproof enclosure for cable splicing. All hardware shall be cast aluminum or stainless steel. Holes drilled into signal poles, mast arms, or posts shall require rubber grommets. In order to maintain uniformity between communities, the confirmation beacons shall indicate when the control equipment receives the pre-emption signal. The pre-emption movement shall be signalized by a flashing indication at the rate specified by Section 4L.01 of the "Manual on Uniform Traffic Control Devices," and other applicable sections of future editions. The stopped pre-empted movements shall be signalized by a continuous indication.

All light operated systems shall include security and transit preemption software and operate at a uniform rate of 14.035 Hz \pm 0.002, or as otherwise required by the Engineer, and provide compatible operation with other light systems currently being operated in the District.

This item shall include any required modifications to an existing traffic signal controller as a result of the addition of the EMERGENCY VEHICLE PRIORITY SYSTEM.

Basis of Payment.

The work shall be paid for at the contract unit price each for furnishing and installing LIGHT DETECTOR and LIGHT DETECTOR AMPLIFIER. Furnishing and installing the confirmation beacon shall be included in the cost of the Light Detector. Any required modifications to the traffic signal controller shall be included in the cost of the LIGHT DETECTOR AMPLIFIER. The preemption detector amplifier shall be paid for on a basis of (1) one each per intersection controller and shall provide operation for all movements required in the pre-emption phase sequence.

PEDESTRIAN PUSH-BUTTON

Effective: May 22, 2002

Revised: July 1, 2015

888.01TS

Description.

Revise Article 888.01 of the Standard Specifications to read:

This work shall consist of furnishing and installing a latching (single call) or non-latching (dual call) pedestrian push-button and a regulatory pedestrian instruction sign according to MUTCD, sign series R10-3e 9" x 15" sign with arrow(s) for a count-down pedestrian signal. The pedestrian station sign size

without count-down pedestrian signals shall accommodate a MUTCD sign series R10-3b or R10-3d 9" x 12" sign with arrow(s).

Installation.

Add the following to Article 888.03 of the Standard Specifications:

A mounting bracket and/or extension shall be used to assure proper orientation when two pedestrian push buttons are required for one post. The price of the bracket and/or extension shall be included in the cost of the pedestrian push button. The contractor is not allowed to install a push-button assembly with the sign below the push-button in order to meet mounting requirements.

Materials.

Revise Article 1074.02(a) of the Standard Specifications to read:

The pedestrian push-button housing shall be constructed of aluminum alloy according to ASTM B 308 6061-T6 and powder coated yellow, unless otherwise noted on the plans. The housing shall be furnished with suitable mounting hardware.

Revise Article 1074.02(e) of the Standard Specifications to read:

Stations shall be designed to be mounted to a post, mast arm pole or wood pole. The station shall be aluminum and shall accept a 3 inch (75mm) round push-button assembly and a regulatory pedestrian instruction sign according to MUTCD, sign series R10-3e 9" x 15" sign with arrow(s) for a count-down pedestrian signal. The pedestrian station size without count-down pedestrian signals shall accommodate a MUTCD sign series R10-3b or R10-3d 9" x 12" sign with arrow(s).

Add the following to Article 1074.02 of the Standard Specifications:

- (f) Location. Pedestrian push-buttons and stations shall be mounted to a post, mast arm pole or wood pole as shown on the plans and shall be fully ADA accessible from a paved or concrete surface. See the District's Detail sheets for orientation and mounting details.

Basis of Payment.

Revise Article 888.04 of the Standard Specifications to read:

This work will be paid for at the contract unit price per each for PEDESTRIAN PUSH-BUTTON or PEDESTRIAN PUSH-BUTTON, NON-LATCHING.

ACCESSIBLE PEDESTRIAN SIGNALS

Effective: April 1, 2003

Revised: July 1, 2015

888.02TS

Description.

This work shall consist of furnishing and installing pedestrian push button accessible pedestrian signals (APS) type. Each APS shall consist of an interactive vibrotactile pedestrian pushbutton with speaker, an informational sign, a light emitting diode (LED) indicator light, a solid state electronic

control board, a power supply, wiring, and mounting hardware. The APS shall meet the requirements of the MUTCD and Sections 801 and 888 of the Standard Specifications, except as modified herein.

Electrical Requirements.

The APS shall operate with systems providing 95 to 130 VAC, 60 Hz and throughout an ambient air temperature range of -29 to +160 °F (-34 to +70 °C).

The APS shall contain a power protection circuit consisting of both fuse and transient protection.

Audible Indications.

A pushbutton locator tone shall sound at each pushbutton with volume settings a maximum of 5 dBA louder than ambient sound.

If two accessible pedestrian pushbuttons are placed less than 10 ft (3 m) apart or placed on the same pole, the audible walk indication shall be a speech walk message.

A clear, verbal message shall be used to communicate the pedestrian walk interval. This message shall sound throughout the WALK interval only. The verbal message shall be modeled after: “Street Name.” Walk Sign is on to cross “Street Name.” No other messages shall be used to denote the WALK interval.

Where two accessible pedestrian pushbuttons are separated by at least 10 ft (3 m), the walk indication shall be an audible percussive tone. It shall repeat at 8 to 10 ticks per second with a dominant frequency of 880 Hz.

Automatic volume adjustments in response to ambient traffic sound level shall be provided up to a maximum volume of 100 dBA. Locator tone and verbal messages shall be no more than 5 dB louder than ambient sound.

Pedestrian Pushbutton.

Pedestrian pushbuttons shall be at least 2 in. (50 mm) in diameter or width. The force required to activate the pushbutton shall be no greater than 3.5 lb (15.5 N).

A red LED indicator shall be located on or near the pushbutton which, when activated, acknowledges the pedestrians request to cross the street. The recorded messages and roadway designations shall be confirmed with the engineer and included with submitted product data.

Signage.

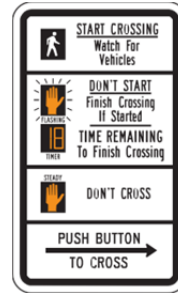
A sign shall be located immediately above the pedestrian pushbutton and parallel to the crosswalk controlled by the pushbutton. The sign shall be one of the following standard MUTCD designs: R10-3b, R10-3d, or R10-3e.



R10-3b



R10-3d



R10-3e

Tactile Arrow.

A tactile arrow, pointing in the direction of travel controlled by a pushbutton, shall be provided either on the pushbutton or its sign.

Vibrotactile Feature.

The pushbutton shall pulse when depressed and shall vibrate continuously throughout the WALK interval.

Training.

The Contractor shall provide APS onsite training for Department personnel and person(s) or group that requested the installation of the APS. APS features and operation shall be demonstrated during the training. The training shall be presented by the APS equipment supplier. Time, date, and location of the training and demonstration shall be coordinated with the Engineer.

Basis of Payment.

This work will be paid for at the contract unit price each for a pedestrian push button, ACCESSIBLE PEDESTRIAN SIGNALS type and shall include furnishing, installation, mounting hardware, message programming, and training.

State of Illinois
Department of Transportation
Bureau of Local Roads and Streets

SPECIAL PROVISION
FOR
INSURANCE

Effective: February 1, 2007
Revised: August 1, 2007

All references to Sections or Articles in this specification shall be construed to mean specific Section or Article of the Standard Specifications for Road and Bridge Construction, adopted by the Department of Transportation.

The Contractor shall name the following entities as additional insured under the Contractor's general liability insurance policy in accordance with Article 107.27:

The entities listed above and their officers, employees, and agents shall be indemnified and held harmless in accordance with Article 107.26.

IDOT TRAINING PROGRAM GRADUATE ON-THE-JOB TRAINING SPECIAL PROVISION (TPG)

Effective: August 1, 2012

Revised: February 1, 2014

In addition to the Contractor's equal employment opportunity affirmative action efforts undertaken as elsewhere required by this Contract, the Contractor is encouraged to participate in the incentive program to provide additional on-the-job training to certified graduates of IDOT funded pre-apprenticeship training programs outlined by this Special Provision.

It is the policy of IDOT to fund IDOT pre-apprenticeship training programs throughout Illinois to provide training and skill-improvement opportunities to assure the increased participation of minority groups, disadvantaged persons and women in all phases of the highway construction industry. The intent of this IDOT Training Program Graduate (TPG) Special Provision is to place certified graduates of these IDOT funded pre-apprentice training programs on IDOT project sites when feasible, and provide the graduates with meaningful on-the-job training intended to lead to journey-level employment. IDOT and its sub-recipients, in carrying out the responsibilities of a state contract, shall determine which construction contracts shall include

Training Program Graduate Special Provisions. To benefit from the incentives to encourage

the participation in the additional on-the-job training under this Training Program Graduate Special Provision, the Contractor shall make every reasonable effort to employ certified graduates of IDOT funded Pre-apprenticeship Training Programs to the extent such persons are available within a reasonable recruitment area.

Participation pursuant to IDOT's requirements by the Contractor or subcontractor in this Training Program Graduate (TPG) Special Provision entitles the Contractor or subcontractor to be reimbursed at \$15.00 per hour for training given a certified TPG on this contract. As approved by the Department, reimbursement will be made for training persons as specified herein. This reimbursement will be made even though the Contractor or subcontractor may receive additional training program funds from other sources for other trainees, provided such other source does not specifically prohibit the Contractor or subcontractor from receiving other reimbursement. For purposes of this Special Provision the Contractor is not relieved of requirements under applicable federal law, the Illinois Prevailing Wage Act, and is not eligible for other training fund reimbursements in addition to the Training Program Graduate (TPG) Special Provision reimbursement.

No payment shall be made to the Contractor if the Contractor or subcontractor fails to provide the required training. It is normally expected that a TPG will begin training on the project as soon as feasible after start of work utilizing the skill involved and remain on the project through completion of the contract, so long as training opportunities exist in his work classification or until he has completed his training program. Should the TPG's employment end in advance of the completion of the contract, the Contractor shall promptly notify the designated IDOT staff member under this Special Provision that the TPG's involvement in the contract has ended and supply a written report of the reason for the end of the involvement, the hours completed by the TPG under the Contract and the number of hours for which the incentive payment provided under this Special Provision will be or has been claimed for the TPG.

The Contractor will provide for the maintenance of records and furnish periodic reports documenting its performance under this Special Provision.

METHOD OF MEASUREMENT: The unit of measurement is in hours.

BASIS OF PAYMENT: This work will be paid for at the contract unit price of \$15.00 per hour for certified TRAINEES TRAINING PROGRAM GRADUATE. The estimated total number of hours, unit price, and total price have been included in the schedule of prices.

The Contractor shall provide training opportunities aimed at developing full journeyworker in the type of trade or job classification involved. The initial number of TPGs for which the incentive is available under this contract is two (2). During the course of performance of the Contract the Contractor may seek approval from the Department for additional incentive eligible TPGs. In the event the Contractor subcontracts a portion of the contract work, it shall determine how many, if any, of the TPGs are to be trained by the subcontractor, provided however, that the Contractor shall retain the primary responsibility for meeting the training requirements imposed by this Special Provision. The Contractor shall also insure that this Training Program Graduate Special Provision is made applicable to such subcontract if the TPGs are to be trained by a subcontractor and that the incentive payment is passed on to each subcontractor.

For the Contractor to meet the obligations for participation in this TPG incentive program under this Special Provision, the Department has contracted with several entities to provide screening, tutoring and pre-training to individuals interested in working in the applicable construction classification and has certified those students who have successfully completed the program and are eligible to be TPGs. A designated IDOT staff member, the Director of the Office of Business and Workforce Diversity (OBWD), will be responsible for providing assistance and referrals to the Contractor for the applicable TPGs. For this contract, the Director of OBWD is designated as the responsible IDOT staff member to provide the assistance and referral services related to the placement for this Special Provision. For purposes of this Contract, contacting the Director of OBWD and interviewing each candidate he/she recommends constitutes reasonable recruitment.

Prior to commencing construction, the Contractor shall submit to the Department for approval the TPGs to be trained in each selected classification. Furthermore, the Contractor shall specify the starting time for training in each of the classifications. No employee shall be employed as a TPG in any classification in which he/she has successfully completed a training course leading to journeyman status or in which he/she has been employed as a journeyman. Notwithstanding the on-the-job training purpose of this TPG Special Provision, some offsite training is permissible as long as the offsite training is an integral part of the work of the contract and does not comprise a significant part of the overall training.

Training and upgrading of TPGs of IDOT pre-apprentice training programs is intended to move said TPGs toward journeyman status and is the primary objective of this Training Program Graduate Special Provision. Accordingly, the Contractor shall make every effort to enroll TPGs by recruitment through the IDOT funded TPG programs to the extent such persons are available within a reasonable area of recruitment. The Contractor will be responsible for demonstrating the steps that it has taken in pursuance thereof, prior to a determination as to whether the Contractor is in compliance and entitled to the Training Program Graduate Special Provision \$15.00 an hour incentive.

The Contractor or subcontractor shall provide each TPG with a certificate showing the type and length of training satisfactorily completed.

ACCESSIBLE PEDESTRIAN SIGNALS (APS) (BDE)

Effective: April 1, 2003

Revised: January 1, 2014

Description. This work shall consist of furnishing and installing accessible pedestrian signals (APS). Each APS shall consist of an interactive vibrotactile pedestrian pushbutton with speaker, an informational sign, a light emitting diode (LED) indicator light, a solid state electronic control board, a power supply, wiring, and mounting hardware. The APS shall meet the requirements of the MUTCD and Sections 801 and 888 of the Standard Specifications, except as modified herein.

Electrical Requirements. The APS shall operate with systems providing 95 to 130 VAC, 60 Hz and throughout an ambient air temperature range of -29 to +160 °F (-34 to +70 °C).

The APS shall contain a power protection circuit consisting of both fuse and transient protection.

Audible Indications. A pushbutton locator tone shall sound at each pushbutton.

If two accessible pedestrian pushbuttons are placed less than 10 ft (3 m) apart or placed on the same pole, the audible walk indication shall be a speech walk message.

A clear, verbal message shall be used to communicate the pedestrian walk interval. This message shall sound throughout the WALK interval only. The verbal message shall be modeled after: “Street Name.’ Walk Sign is on to cross “Street Name.” No other messages shall be used to denote the WALK interval.

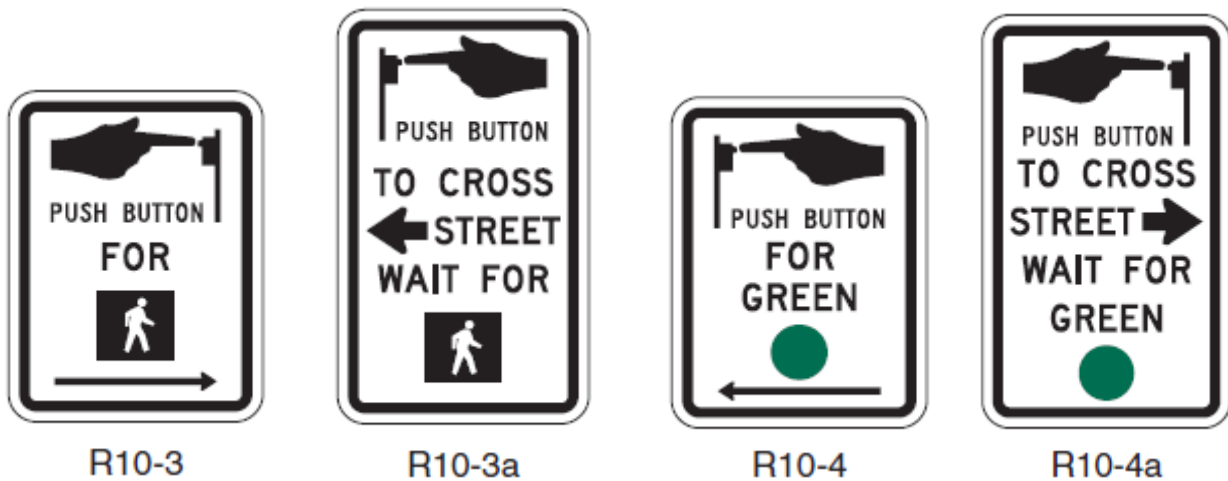
Where two accessible pedestrian pushbuttons are separated by at least 10 ft (3 m), the walk indication shall be an audible percussive tone. It shall repeat at 8 to 10 ticks per second with a dominant frequency of 880 Hz.

Automatic volume adjustments in response to ambient traffic sound level shall be provided up to a maximum volume of 100 dBA. Locator tone and verbal messages shall be no more than 5 dB louder than ambient sound.

Pedestrian Pushbutton. Pedestrian pushbuttons shall be at least 2 in. (50 mm) in diameter or width. The force required to activate the pushbutton shall be no greater than 3.5 lb (15.5 N).

A red LED shall be located on or near the pushbutton which, when activated, acknowledges the pedestrians request to cross the street.

Signage. A sign shall be located immediately above the pedestrian pushbutton and parallel to the crosswalk controlled by the pushbutton. The sign shall be one of the following standard MUTCD designs:



Tactile Arrow. A tactile arrow, pointing in the direction of travel controlled by a pushbutton, shall be provided either on the pushbutton or its sign.

Vibrotactile Feature. The pushbutton shall pulse when depressed and shall vibrate continuously throughout the WALK interval.

Method of Measurement. This work will be measured for payment as each, per pushbutton.

Basis of Payment. This work will be paid for at the contract unit price per each for ACCESSIBLE PEDESTRIAN SIGNALS.

AGGREGATE SUBGRADE IMPROVEMENT (BDE)

Effective: April 1, 2012

Revised: January 1, 2016

Add the following Section to the Standard Specifications:

“SECTION 303. AGGREGATE SUBGRADE IMPROVEMENT

303.01 Description. This work shall consist of constructing an aggregate subgrade improvement.

303.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Coarse Aggregate	1004.06
(b) Reclaimed Asphalt Pavement (RAP) (Notes 1, 2, and 3)	1031

Note 1. Crushed RAP, from either full depth or single lift removal, may be mechanically blended with aggregate gradations CS 01, CS 02, and RR 01 but shall not exceed 40 percent of the total product. The top size of the RAP shall be less than 4 in. (100 mm) and well graded.

Note 2. RAP having 100 percent passing the 1 1/2 in. (37.5 mm) sieve and being well graded, may be used as capping aggregate in the top 3 in. (75 mm) when aggregate gradations CS 01, CS 02, or RR 01 are used in lower lifts.

Note 3. The RAP used for aggregate subgrade improvement shall be according to the current Bureau of Materials and Physical Research Policy Memorandum, “Reclaimed Asphalt Pavement (RAP) for Aggregate Applications”.

303.03 Equipment. The vibratory machine shall be according to Article 1101.01, or as approved by the Engineer.

303.04 Soil Preparation. The stability of the soil shall be according to the Department’s Subgrade Stability Manual for the aggregate thickness specified.

303.05 Placing Aggregate. The maximum nominal lift thickness of aggregate gradations CA 02, CA 06, or CA 10 shall be 12 in. (300 mm). The maximum nominal lift thickness of aggregate gradations CS 01, CS 02, and RR 01 shall be 24 in. (600 mm).

303.06 Capping Aggregate. The top surface of the aggregate subgrade shall consist of a minimum 3 in. (75 mm) of aggregate gradations CA 06 or CA 10. When the contract specifies that a granular subbase is to be placed on the aggregate subgrade improvement, the 3 in. (75 mm) of capping aggregate shall be the same gradation and may be placed with the underlying aggregate subgrade improvement material.

303.07 Compaction. All aggregate lifts shall be compacted to the satisfaction of the Engineer. If the moisture content of the material is such that compaction cannot be obtained, sufficient water shall be added so that satisfactory compaction can be obtained.

303.08 Finishing and Maintenance of Aggregate Subgrade Improvement. The aggregate subgrade improvement shall be finished to the lines, grades, and cross sections shown on the plans, or as directed by the Engineer. The aggregate subgrade improvement shall be maintained in a smooth and compacted condition.

303.09 Method of Measurement. This work will be measured for payment according to Article 311.08.

303.10 Basis of Payment. This work will be paid for at the contract unit price per cubic yard (cubic meter) or ton (metric ton) for AGGREGATE SUBGRADE IMPROVEMENT or at the contract unit price per square yard (square meter) for AGGREGATE SUBGRADE IMPROVEMENT, of the thickness specified.”

Add the following to Section 1004 of the Standard Specifications:

“1004.06 Coarse Aggregate for Aggregate Subgrade Improvement. The aggregate shall be according to Article 1004.01 and the following.

(a) Description. The coarse aggregate shall be crushed gravel, crushed stone, or crushed concrete. In applications where greater than 24 in. (600 mm) of subgrade material is required, gravel may be used below the first 12 in (300 mm) of subgrade.

(b) Quality. The coarse aggregate shall consist of sound durable particles reasonably free of deleterious materials.

(c) Gradation.

(1) The coarse aggregate gradation for total subgrade thickness less than or equal to 12 in. (300 mm) shall be CA 2, CA 6, CA 10, or CS 01.

The coarse aggregate gradation for total subgrade thickness more than 12 in. (300 mm) shall be CS 01, CS 02 or RR 01(see Article 1005.01(c)).

COARSE AGGREGATE SUBGRADE GRADATIONS					
Grad No.	Sieve Size and Percent Passing				
	8"	6"	4"	2"	#4
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20
CS 02		100	80 ± 10	25 ± 15	

COARSE AGGREGATE SUBGRADE GRADATIONS (Metric)	
Grad No.	Sieve Size and Percent Passing

	200 mm	150 mm	100 mm	50 mm	4.75 mm
CS 01	100	97 ± 3	90 ± 10	45 ± 25	20 ± 20
CS 02		100	80 ± 10	25 ± 15	

(2) The 3 in. (75 mm) capping aggregate shall be gradation CA 6 or CA 10.”

80274

BITUMINOUS MATERIALS COST ADJUSTMENTS (BDE) (RETURN FORM WITH BID)

Effective: November 2, 2006

Revised: July 1, 2015

Description. Bituminous material cost adjustments will be made to provide additional compensation to the Contractor, or credit to the Department, for fluctuations in the cost of bituminous materials when optioned by the Contractor. The bidder shall indicate on the attached form whether or not this special provision will be part of the contract and submit the completed form with his/her bid. Failure to submit the form, or failure to fill out the form completely, shall make this contract exempt of bituminous materials cost adjustments.

The adjustments shall apply to permanent and temporary hot-mix asphalt (HMA) mixtures, bituminous surface treatments (cover and seal coats), and preventative maintenance type surface treatments that are part of the original proposed construction, or added as extra work and paid for by agreed unit prices. The adjustments shall not apply to bituminous prime coats, tack coats, crack filling/sealing, joint filling/sealing, or extra work paid for at a lump sum price or by force account.

Method of Adjustment. Bituminous materials cost adjustments will be computed as follows.

$$CA = (BPI_P - BPI_L) \times (\%AC_V / 100) \times Q$$

Where: CA = Cost Adjustment, \$.

BPI_P = Bituminous Price Index, as published by the Department for the month the work is performed, \$/ton (\$/metric ton).

BPI_L = Bituminous Price Index, as published by the Department for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price, \$/ton (\$/metric ton).

%AC_V = Percent of virgin Asphalt Cement in the Quantity being adjusted. For HMA mixtures, the % AC_V will be determined from the adjusted job mix formula. For bituminous materials applied, a performance graded or cutback asphalt will be considered to be 100% AC_V and undiluted emulsified asphalt will be considered to be 65% AC_V.

Q = Authorized construction Quantity, tons (metric tons) (see below).

For HMA mixtures measured in square yards: $Q, \text{ tons} = A \times D \times (G_{mb} \times 46.8) / 2000$. For HMA mixtures measured in square meters: $Q, \text{ metric tons} = A \times D \times (G_{mb} \times 1) / 1000$. When computing adjustments for full-depth HMA pavement, separate calculations will be made for the binder and surface courses to account for their different G_{mb} and % AC_V.

For bituminous materials measured in gallons: $Q, \text{ tons} = V \times 8.33 \text{ lb/gal} \times SG / 2000$

For bituminous materials measured in liters: $Q, \text{ metric tons} = V \times 1.0 \text{ kg/L} \times SG / 1000$

Where: A = Area of the HMA mixture, sq yd (sq m).

- D = Depth of the HMA mixture, in. (mm).
G_{mb} = Average bulk specific gravity of the mixture, from the approved mix design.
V = Volume of the bituminous material, gal (L).
SG = Specific Gravity of bituminous material as shown on the bill of lading.

Basis of Payment. Bituminous materials cost adjustments may be positive or negative but will only be made when there is a difference between the BPI_L and BPI_P in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(BPI_L - BPI_P) \div BPI_L\} \times 100$$

Bituminous materials cost adjustments will be calculated for each calendar month in which applicable bituminous material is placed; and will be paid or deducted when all other contract requirements for the work placed during the month are satisfied. The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

Return With Bid

**ILLINOIS DEPARTMENT
OF TRANSPORTATION**

**OPTION FOR
BITUMINOUS MATERIALS COST ADJUSTMENTS**

The bidder shall submit this completed form with his/her bid. Failure to submit the form, or failure to fill out the form completely, shall make this contract exempt of bituminous materials cost adjustments. After award, this form, when submitted, shall become part of the contract.

Contract No.: _____

Company Name: _____

Contractor's Option:

Is your company opting to include this special provision as part of the contract?

Yes ☐ No ☐

Signature: _____ **Date:** _____

80173

COATED GALVANIZED STEEL CONDUIT (BDE)

Effective: January 1, 2013

| Revised: January 1, 2015

Revise Article 811.03(b) of the Standard Specifications to read:

“(b) Coated Galvanized Steel Conduit. In addition to the methods described in Article 810.05(a) the following methods shall be observed when installing coated conduit.

Coated conduit pipe vise jaw adapters shall be used when the conduit is being clamped to avoid damaging the coating.

Coated conduit shall be cut with a roller cutter or by other means approved by the conduit manufacturer.

After any cutting or threading operations are completed, the bare steel shall be touched up with the conduit manufacturer’s touch up compound.”

|
80310

COILABLE NONMETALLIC CONDUIT (BDE)

Effective: August 1, 2014

| Revised: January 1, 2015

Revise Article 1088.01(c) of the Standard Specifications to read:

“(c) Coilable Nonmetallic Conduit. The conduit shall be a high density polyethylene duct which is intended for underground use can be manufactured and coiled or reeled in continuous transportable lengths and uncoiled for further processing and/or installation without adversely affecting its properties or performance. The conduit and its manufacture shall be according to UL 651A for Schedule 40 conduit, except Schedule 80 shall be used under pavement, stabilized shoulder, paved median, paved driveway, curb and/or gutter and sidewalk.

| Performance Tests. Testing procedures and test results shall meet the requirements of UL 651A. Certified copies of the test report shall be submitted to the Engineer prior to the installation of the conduit.”

80341

**CONCRETE BOX CULVERTS WITH SKEWS \leq 30 DEGREES REGARDLESS OF DESIGN
FILL AND SKEWS > 30 DEGREES WITH DESIGN FILLS > 5 FEET (BDE)**

Effective: April 1, 2012

| Revised: April 1, 2014

Revise the second paragraph of Article 540.04 of the Standard Specifications to read:

“Unless otherwise noted on the plans, the Contractor shall have the option, when a cast-in-place concrete box culvert is specified, of constructing the box culvert using precast box culvert sections when the design cover is 6 in. (150 mm) minimum. The precast box culvert sections shall be designed for the same design cover shown on the plans for cast-in-place box culvert; shall be of equal or larger size opening, and shall satisfy the design requirements of ASTM C 1577.”

|
80294

CONCRETE GUTTER, CURB, MEDIAN, AND PAVED DITCH (BDE)

Effective: April 1, 2014

| Revised: August 1, 2014

Add the following to Article 606.02 of the Standard Specifications:

“(i) Polyurethane Joint Sealant 1050.04”

Revise the fifth paragraph of Article 606.07 of the Standard Specifications to read:

“Transverse contraction and longitudinal construction joints shall be sealed according to Article 420.12, except transverse joints in concrete curb and gutter shall be sealed with polysulfide or polyurethane joint sealant.”

Add the following to Section 1050 of the Standard Specifications:

| “**1050.04 Polyurethane Joint Sealant.** The joint sealant shall be a polyurethane sealant, Type S, Grade NS, Class 25 or better, Use T (T₁ or T₂), according to ASTM C 920.”

80334

CONSTRUCTION AIR QUALITY – DIESEL RETROFIT (BDE)

Effective: June 1, 2010

Revised: November 1, 2014

The reduction of emissions of particulate matter (PM) for off-road equipment shall be accomplished by installing retrofit emission control devices. The term “equipment” refers to diesel fuel powered devices rated at 50 hp and above, to be used on the jobsite in excess of seven calendar days over the course of the construction period on the jobsite (including rental equipment).

Contractor and subcontractor diesel powered off-road equipment assigned to the contract shall be retrofitted using the phased in approach shown below. Equipment that is of a model year older than the year given for that equipment’s respective horsepower range shall be retrofitted:

Effective Dates	Horsepower Range	Model Year
June 1, 2010 ^{1/}	600-749	2002
	750 and up	2006
June 1, 2011 ^{2/}	100-299	2003
	300-599	2001
	600-749	2002
	750 and up	2006
June 1, 2012 ^{2/}	50-99	2004
	100-299	2003
	300-599	2001
	600-749	2002
	750 and up	2006

1/ Effective dates apply to Contractor diesel powered off-road equipment assigned to the contract.

2/ Effective dates apply to Contractor and subcontractor diesel powered off-road equipment assigned to the contract.

The retrofit emission control devices shall achieve a minimum PM emission reduction of 50 percent and shall be:

- a) Included on the U.S. Environmental Protection Agency (USEPA) *Verified Retrofit Technology List* (<http://www.epa.gov/cleandiesel/verification/verif-list.htm>), or verified by the California Air Resources Board (CARB) (<http://www.arb.ca.gov/diesel/verdev/vt/cvt.htm>); or
- b) Retrofitted with a non-verified diesel retrofit emission control device if verified retrofit emission control devices are not available for equipment proposed to be used on the project, and if the Contractor has obtained a performance certification from the retrofit

device manufacturer that the emission control device provides a minimum PM emission reduction of 50 percent.

Note: Large cranes (Crawler mounted cranes) which are responsible for critical lift operations are exempt from installing retrofit emission control devices if such devices adversely affect equipment operation.

Diesel powered off-road equipment with engine ratings of 50 hp and above, which are unable to be retrofitted with verified emission control devices or if performance certifications are not available which will achieve a minimum 50 percent PM reduction, may be granted a waiver by the Department if documentation is provided showing good faith efforts were made by the Contractor to retrofit the equipment.

Construction shall not proceed until the Contractor submits a certified list of the diesel powered off-road equipment that will be used, and as necessary, retrofitted with emission control devices. The list(s) shall include (1) the equipment number, type, make, Contractor/rental company name; and (2) the emission control devices make, model, USEPA or CARB verification number, or performance certification from the retrofit device manufacturer. Equipment reported as fitted with emissions control devices shall be made available to the Engineer for visual inspection of the device installation, prior to being used on the jobsite.

The Contractor shall submit an updated list of retrofitted off-road construction equipment as retrofitted equipment changes or comes on to the jobsite. The addition or deletion of any diesel powered equipment shall be included on the updated list.

If any diesel powered off-road equipment is found to be in non-compliance with any portion of this special provision, the Engineer will issue the Contractor a diesel retrofit deficiency deduction.

Any costs associated with retrofitting any diesel powered off-road equipment with emission control devices shall be considered as included in the contract unit prices bid for the various items of work involved and no additional compensation will be allowed. The Contractor's compliance with this notice and any associated regulations shall not be grounds for a claim.

Diesel Retrofit Deficiency Deduction

When the Engineer determines that a diesel retrofit deficiency exists, a daily monetary deduction will be imposed for each calendar day or fraction thereof the deficiency continues to exist. The calendar day(s) will begin when the time period for correction is exceeded and end with the Engineer's written acceptance of the correction. The daily monetary deduction will be \$1,000.00 for each deficiency identified.

The deficiency will be based on lack of diesel retrofit emissions control.

If a Contractor accumulates three diesel retrofit deficiency deductions for the same piece of equipment in a contract period, the Contractor will be shutdown until the deficiency is corrected.

Such a shutdown will not be grounds for any extension of the contract time, waiver of penalties, or be grounds for any claim.

80261

CONTRACT CLAIMS (BDE)

Effective: April 1, 2014

Revise the first paragraph of Article 109.09(a) of the Standard Specifications to read:

“(a) Submission of Claim. All claims filed by the Contractor shall be in writing and in sufficient detail to enable the Department to ascertain the basis and amount of the claim. As a minimum, the following information must accompany each claim submitted.”

Revise Article 109.09(e) of the Standard Specifications to read:

“(e) Procedure. The Department provides two administrative levels for claims review.

Level I Engineer of Construction

Level II Chief Engineer/Director of Highways or Designee

- (1) Level I. All claims shall first be submitted at Level I. Two copies each of the claim and supporting documentation shall be submitted simultaneously to the District and the Engineer of Construction. The Engineer of Construction, in consultation with the District, will consider all information submitted with the claim and render a decision on the claim within 90 days after receipt by the Engineer of Construction. Claims not conforming to this Article will be returned without consideration. The Engineer of Construction may schedule a claim presentation meeting if in the Engineer of Construction's judgment such a meeting would aid in resolution of the claim, otherwise a decision will be made based on the claim documentation submitted. If a Level I decision is not rendered within 90 days of receipt of the claim, or if the Contractor disputes the decision, an appeal to Level II may be made by the Contractor.
- (2) Level II. An appeal to Level II shall be made in writing to the Engineer of Construction within 45 days after the date of the Level I decision. Review of the claim at Level II shall be conducted as a full evaluation of the claim. A claim presentation meeting may be scheduled if the Chief Engineer/Director of Highways determines that such a meeting would aid in resolution of the claim, otherwise a decision will be made based on the claim documentation submitted. A Level II final decision will be rendered within 90 days of receipt of the written request for appeal.

Full compliance by the Contractor with the provisions specified in this Article is a contractual condition precedent to the Contractor's right to seek relief in the Court of Claims. The Director's written decision shall be the final administrative action of the Department. Unless the Contractor files a claim for adjudication by the Court of Claims within 60 days after the date of the written decision, the failure to file shall constitute a release and waiver of the claim.”

DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION (BDE)

Effective: September 1, 2000

Revised: January 2, 2015

FEDERAL OBLIGATION. The Department of Transportation, as a recipient of federal financial assistance, is required to take all necessary and reasonable steps to ensure nondiscrimination in the award and administration of contracts. Consequently, the federal regulatory provisions of 49 CFR Part 26 apply to this contract concerning the utilization of disadvantaged business enterprises. For the purposes of this Special Provision, a disadvantaged business enterprise (DBE) means a business certified by the Department in accordance with the requirements of 49 CFR Part 26 and listed in the Illinois Unified Certification Program (IL UCP) DBE Directory.

STATE OBLIGATION. This Special Provision will also be used by the Department to satisfy the requirements of the Business Enterprise for Minorities, Females, and Persons with Disabilities Act, 30 ILCS 575. When this Special Provision is used to satisfy state law requirements on 100 percent state-funded contracts, the federal government has no involvement in such contracts (not a federal-aid contract) and no responsibility to oversee the implementation of this Special Provision by the Department on those contracts. DBE participation on 100 percent state-funded contracts will not be credited toward fulfilling the Department's annual overall DBE goal required by the US Department of Transportation to comply with the federal DBE program requirements.

CONTRACTOR ASSURANCE. The Contractor makes the following assurance and agrees to include the assurance in each subcontract that the Contractor signs with a subcontractor.

The Contractor, subrecipient, or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The Contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of contracts funded in whole or in part with federal or state funds. Failure by the Contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the recipient deems appropriate, which may include, but is not limited to:

- (a) Withholding progress payments;
- (b) Assessing sanctions;
- (c) Liquidated damages; and/or
- (d) Disqualifying the Contractor from future bidding as non-responsible.

OVERALL GOAL SET FOR THE DEPARTMENT. As a requirement of compliance with 49 CFR Part 26, the Department has set an overall goal for DBE participation in its federally assisted contracts. That goal applies to all federal-aid funds the Department will expend in its federally assisted contracts for the subject reporting fiscal year. The Department is required to make a

good faith effort to achieve the overall goal. The dollar amount paid to all approved DBE companies performing work called for in this contract is eligible to be credited toward fulfillment of the Department's overall goal.

CONTRACT GOAL TO BE ACHIEVED BY THE CONTRACTOR. This contract includes a specific DBE utilization goal established by the Department. The goal has been included because the Department has determined that the work of this contract has subcontracting opportunities that may be suitable for performance by DBE companies. The determination is based on an assessment of the type of work, the location of the work, and the availability of DBE companies to do a part of the work. The assessment indicates that, in the absence of unlawful discrimination, and in an arena of fair and open competition, DBE companies can be expected to perform _____% of the work. This percentage is set as the DBE participation goal for this contract. Consequently, in addition to the other award criteria established for this contract, the Department will only award this contract to a bidder who makes a good faith effort to meet this goal of DBE participation in the performance of the work. A bidder makes a good faith effort for award consideration if either of the following is done in accordance with the procedures set for in this Special Provision:

- (a) The bidder documents that enough DBE participation has been obtained to meet the goal or,
- (b) The bidder documents that a good faith effort has been made to meet the goal, even though the effort did not succeed in obtaining enough DBE participation to meet the goal.

DBE LOCATOR REFERENCES. Bidders shall consult the IL UCP DBE Directory as a reference source for DBE-certified companies. In addition, the Department maintains a letting and item specific DBE locator information system whereby DBE companies can register their interest in providing quotes on particular bid items advertised for letting. Information concerning DBE companies willing to quote work for particular contracts may be obtained by contacting the Department's Bureau of Small Business Enterprises at telephone number (217) 785-4611, or by visiting the Department's website at www.dot.il.gov.

BIDDING PROCEDURES. Compliance with this Special Provision is a material bidding requirement. The failure of the bidder to comply will render the bid not responsive.

- (a) The bidder shall submit a Disadvantaged Business Utilization Plan on Department forms SBE 2025 and 2026 with the bid.
- (b) The Utilization Plan shall indicate that the bidder either has obtained sufficient DBE participation commitments to meet the contract goal or has not obtained enough DBE participation commitments in spite of a good faith effort to meet the goal. The Utilization Plan shall further provide the name, telephone number, and telefax number of a responsible official of the bidder designated for purposes of notification of plan approval or disapproval under the procedures of this Special Provision.

(c) The Utilization Plan shall include a DBE Participation Commitment Statement, Department form SBE 2025, for each DBE proposed for the performance of work to achieve the contract goal. For bidding purposes, submission of the completed SBE 2025 forms, signed by the DBEs and faxed to the bidder will be acceptable as long as the original is available and provided upon request. All elements of information indicated on the said form shall be provided, including but not limited to the following:

- (1) The names and addresses of DBE firms that will participate in the contract;
- (2) A description, including pay item numbers, of the work each DBE will perform;
- (3) The dollar amount of the participation of each DBE firm participating. The dollar amount of participation for identified work shall specifically state the quantity, unit price, and total subcontract price for the work to be completed by the DBE. If partial pay items are to be performed by the DBE, indicate the portion of each item, a unit price where appropriate and the subcontract price amount;
- (4) DBE Participation Commitment Statements, form SBE 2025, signed by the bidder and each participating DBE firm documenting the commitment to use the DBE subcontractors whose participation is submitted to meet the contract goal;
- (5) If the bidder is a joint venture comprised of DBE companies and non-DBE companies, the plan must also include a clear identification of the portion of the work to be performed by the DBE partner(s); and,
- (6) If the contract goal is not met, evidence of good faith efforts; the documentation of good faith efforts must include copies of each DBE and non-DBE subcontractor quote submitted to the bidder when a non-DBE subcontractor is selected over a DBE for work on the contract.

GOOD FAITH EFFORT PROCEDURES. The contract will not be awarded until the Utilization Plan submitted by the apparent successful bidder is approved. All information submitted by the bidder must be complete, accurate and adequately document that enough DBE participation has been obtained or document that good faith efforts of the bidder, in the event enough DBE participation has not been obtained, before the Department will commit to the performance of the contract by the bidder. The Utilization Plan will be approved by the Department if the Utilization Plan documents sufficient commercially useful DBE work performance to meet the contract goal or the bidder submits sufficient documentation of a good faith effort to meet the contract goal pursuant to 49 CFR Part 26, Appendix A. The Utilization Plan will not be approved by the Department if the Utilization Plan does not document sufficient DBE participation to meet the contract goal unless the apparent successful bidder documented in the Utilization Plan that it made a good faith effort to meet the goal. This means that the bidder must show that all necessary and reasonable steps were taken to achieve the contract goal. Necessary and reasonable steps are those which, by their scope, intensity and appropriateness to the objective, could reasonably be expected to obtain sufficient DBE participation, even if they were not successful. The Department will consider the quality, quantity, and intensity of the kinds of

efforts that the bidder has made. Mere *pro forma* efforts, in other words, efforts done as a matter of form, are not good faith efforts; rather, the bidder is expected to have taken genuine efforts that would be reasonably expected of a bidder actively and aggressively trying to obtain DBE participation sufficient to meet the contract goal.

(a) The following is a list of types of action that the Department will consider as part of the evaluation of the bidder's good faith efforts to obtain participation. These listed factors are not intended to be a mandatory checklist and are not intended to be exhaustive. Other factors or efforts brought to the attention of the Department may be relevant in appropriate cases, and will be considered by the Department.

(1) Soliciting through all reasonable and available means (e.g. attendance at pre-bid meetings, advertising and/or written notices) the interest of all certified DBE companies that have the capability to perform the work of the contract. The bidder must solicit this interest within sufficient time to allow the DBE companies to respond to the solicitation. The bidder must determine with certainty if the DBE companies are interested by taking appropriate steps to follow up initial solicitations.

(2) Selecting portions of the work to be performed by DBE companies in order to increase the likelihood that the DBE goals will be achieved. This includes, where appropriate, breaking out contract work items into economically feasible units to facilitate DBE participation, even when the prime Contractor might otherwise prefer to perform these work items with its own forces.

(3) Providing interested DBE companies with adequate information about the plans, specifications, and requirements of the contract in a timely manner to assist them in responding to a solicitation.

(4) a. Negotiating in good faith with interested DBE companies. It is the bidder's responsibility to make a portion of the work available to DBE subcontractors and suppliers and to select those portions of the work or material needs consistent with the available DBE subcontractors and suppliers, so as to facilitate DBE participation. Evidence of such negotiation includes the names, addresses, and telephone numbers of DBE companies that were considered; a description of the information provided regarding the plans and specifications for the work selected for subcontracting; and evidence as to why additional agreements could not be reached for DBE companies to perform the work.

b. A bidder using good business judgment would consider a number of factors in negotiating with subcontractors, including DBE subcontractors, and would take a firm's price and capabilities as well as contract goals into consideration. However, the fact that there may be some additional costs involved in finding and using DBE companies is not in itself sufficient reason for a bidder's failure to meet the contract DBE goal, as long as such costs are reasonable. Also the ability or desire of a bidder to perform the work of a contract with its own organization does not relieve the bidder of the responsibility to make good faith

efforts. Bidders are not, however, required to accept higher quotes from DBE companies if the price difference is excessive or unreasonable. In accordance with Section 6 of the above Bidding Procedures, the documentation of good faith efforts must include copies of each DBE and non-DBE subcontractor quote submitted to the bidder when a non-DBE subcontractor was selected over a DBE for work on the contract.

- (5) Not rejecting DBE companies as being unqualified without sound reasons based on a thorough investigation of their capabilities. The bidder's standing within its industry, membership in specific groups, organizations, or associations and political or social affiliations (for example union vs. non-union employee status) are not legitimate causes for the rejection or non-solicitation of bids in the bidder's efforts to meet the project goal.
 - (6) Making efforts to assist interested DBE companies in obtaining bonding, lines of credit, or insurance as required by the recipient or Contractor.
 - (7) Making efforts to assist interested DBE companies in obtaining necessary equipment, supplies, materials, or related assistance or services.
 - (8) Effectively using the services of available minority/women community organizations; minority/women contractors' groups; local, state, and federal minority/women business assistance offices; and other organizations as allowed on a case-by-case basis to provide assistance in the recruitment and placement of DBE companies.
- (b) If the Department determines that the apparent successful bidder has made a good faith effort to secure the work commitment of DBE companies to meet the contract goal, the Department will award the contract provided that it is otherwise eligible for award. If the Department determines that the bidder has failed to meet the requirements of this Special Provision or that a good faith effort has not been made, the Department will notify the responsible company official designated in the Utilization Plan that the bid is not responsive. The notification shall include a statement of reasons for the determination.
- (c) The bidder may request administrative reconsideration of a determination adverse to the bidder within the five working days after the receipt of the notification date of the determination by delivering the request to the Department of Transportation, Bureau of Small Business Enterprises, Contract Compliance Section, 2300 South Dirksen Parkway, Room 319, Springfield, Illinois 62764 (Telefax: (217) 785-1524). Deposit of the request in the United States mail on or before the fifth business day shall not be deemed delivery. The determination shall become final if a request is not made and delivered. A request may provide additional written documentation or argument concerning the issues raised in the determination statement of reasons, provided the documentation and arguments address efforts made prior to submitting the bid. The request will be forwarded to the Department's Reconsideration Officer. The Reconsideration Officer will extend an opportunity to the bidder to meet in person in

order to consider all issues of documentation and whether the bidder made a good faith effort to meet the goal. After the review by the Reconsideration Officer, the bidder will be sent a written decision within ten working days after receipt of the request for consideration, explaining the basis for finding that the bidder did or did not meet the goal or make adequate good faith efforts to do so. A final decision by the Reconsideration Officer that a good faith effort was made shall approve the Utilization Plan submitted by the bidder and shall clear the contract for award. A final decision that a good faith effort was not made shall render the bid not responsive.

CALCULATING DBE PARTICIPATION. The Utilization Plan values represent work anticipated to be performed and paid for upon satisfactory completion. The Department is only able to count toward the achievement of the overall goal and the contract goal the value of payments made for the work actually performed by DBE companies. In addition, a DBE must perform a commercially useful function on the contract to be counted. A commercially useful function is generally performed when the DBE is responsible for the work and is carrying out its responsibilities by actually performing, managing, and supervising the work involved. The Department and Contractor are governed by the provisions of 49 CFR Part 26.55(c) on questions of commercially useful functions as it affects the work. Specific counting guidelines are provided in 49 CFR Part 26.55, the provisions of which govern over the summary contained herein.

- (a) DBE as the Contractor: 100 percent goal credit for that portion of the work performed by the DBE's own forces, including the cost of materials and supplies. Work that a DBE subcontracts to a non-DBE does not count toward the DBE goals.
- (b) DBE as a joint venture Contractor: 100 percent goal credit for that portion of the total dollar value of the contract equal to the distinct, clearly defined portion of the work performed by the DBE's own forces.
- (c) DBE as a subcontractor: 100 percent goal credit for the work of the subcontract performed by the DBE's own forces, including the cost of materials and supplies, excluding the purchase of materials and supplies or the lease of equipment by the DBE subcontractor from the prime Contractor or its affiliates. Work that a DBE subcontractor in turn subcontracts to a non-DBE does not count toward the DBE goal.
- (d) DBE as a trucker: 100 percent goal credit for trucking participation provided the DBE is responsible for the management and supervision of the entire trucking operation for which it is responsible. At least one truck owned, operated, licensed, and insured by the DBE must be used on the contract. Credit will be given for the following:
 - (1) The DBE may lease trucks from another DBE firm, including an owner-operator who is certified as a DBE. The DBE who leases trucks from another DBE receives credit for the total value of the transportation services the lessee DBE provides on the contract.

- (2) The DBE may also lease trucks from a non-DBE firm, including from an owner-operator. The DBE who leases trucks from a non-DBE is entitled to credit only for the fee or commission it receives as a result of the lease arrangement.

(e) DBE as a material supplier:

- (1) 60 percent goal credit for the cost of the materials or supplies purchased from a DBE regular dealer.
- (2) 100 percent goal credit for the cost of materials or supplies obtained from a DBE manufacturer.
- (3) 100 percent credit for the value of reasonable fees and commissions for the procurement of materials and supplies if not a regular dealer or manufacturer.

CONTRACT COMPLIANCE. Compliance with this Special Provision is an essential part of the contract. The Department is prohibited by federal regulations from crediting the participation of a DBE included in the Utilization Plan toward either the contract goal or the Department's overall goal until the amount to be applied toward the goals has been paid to the DBE. The following administrative procedures and remedies govern the compliance by the Contractor with the contractual obligations established by the Utilization Plan. After approval of the Utilization Plan and award of the contract, the Utilization Plan and individual DBE Participation Statements become part of the contract. If the Contractor did not succeed in obtaining enough DBE participation to achieve the advertised contract goal, and the Utilization Plan was approved and contract awarded based upon a determination of good faith, the total dollar value of DBE work calculated in the approved Utilization Plan as a percentage of the awarded contract value shall become the amended contract goal. All work indicated for performance by an approved DBE shall be performed, managed, and supervised by the DBE executing the Participation Statement.

- (a) NO AMENDMENT. No amendment to the Utilization Plan may be made without prior written approval from the Department's Bureau of Small Business Enterprises. All requests for amendment to the Utilization Plan shall be submitted to the Department of Transportation, Bureau of Small Business Enterprises, Contract Compliance Section, 2300 South Dirksen Parkway, Room 319, Springfield, Illinois 62764. Telephone number (217) 785-4611. Telefax number (217) 785-1524.

- (b) CHANGES TO WORK. Any deviation from the DBE condition-of-award or contract plans, specifications, or special provisions must be approved, in writing, by the Department as provided elsewhere in the Contract. The Contractor shall notify affected DBEs in writing of any changes in the scope of work which result in a reduction in the dollar amount condition-of-award to the contract. Where the revision includes work committed to a new DBE subcontractor, not previously involved in the project, then a Request for Approval of Subcontractor, Department form BC 260A, must be signed and submitted. If the commitment of work is in the form of additional tasks assigned to an existing subcontract, then a new Request for Approval of Subcontractor shall not be

required. However, the Contractor must document efforts to assure that the existing DBE subcontractor is capable of performing the additional work and has agreed in writing to the change.

(c) SUBCONTRACT. The Contractor must provide DBE subcontracts to IDOT upon request. Subcontractors shall ensure that all lower tier subcontracts or agreements with DBEs to supply labor or materials be performed in accordance with this Special Provision.

(d) ALTERNATIVE WORK METHODS. In addition to the above requirements for reductions in the condition of award, additional requirements apply to the two cases of Contractor-initiated work substitution proposals. Where the contract allows alternate work methods which serve to delete or create underruns in condition of award DBE work, and the Contractor selects that alternate method or, where the Contractor proposes a substitute work method or material that serves to diminish or delete work committed to a DBE and replace it with other work, then the Contractor must demonstrate one of the following:

(1) That the replacement work will be performed by the same DBE (as long as the DBE is certified in the respective item of work) in a modification of the condition of award; or

(2) That the DBE is aware that its work will be deleted or will experience underruns and has agreed in writing to the change. If this occurs, the Contractor shall substitute other work of equivalent value to a certified DBE or provide documentation of good faith efforts to do so; or

(3) That the DBE is not capable of performing the replacement work or has declined to perform the work at a reasonable competitive price. If this occurs, the Contractor shall substitute other work of equivalent value to a certified DBE or provide documentation of good faith efforts to do so.

(e) TERMINATION AND REPLACEMENT PROCEDURES. The Contractor shall not terminate or replace a DBE listed on the approved Utilization Plan, or perform with other forces work designated for a listed DBE except as provided in this Special Provision. The Contractor shall utilize the specific DBEs listed to perform the work and supply the materials for which each is listed unless the Contractor obtains the Department's written consent as provided in subsection (a). Unless Department consent is provided for termination of a DBE subcontractor, the Contractor shall not be entitled to any payment for work or material unless it is performed or supplied by the DBE listed in the Utilization Plan.

As stated above, the Contractor shall not terminate or replace a DBE subcontractor listed in the approved Utilization Plan without prior written consent. This includes, but is not limited to, instances in which the Contractor seeks to perform work originally designated for a DBE subcontractor with its own forces or those of an affiliate, a non-DBE firm, or with another DBE firm. Written consent will be granted only if the Bureau of

Small Business Enterprises agrees, for reasons stated in its concurrence document, that the Contractor has good cause to terminate or replace the DBE firm. Before transmitting to the Bureau of Small Business Enterprises any request to terminate and/or substitute a DBE subcontractor, the Contractor shall give notice in writing to the DBE subcontractor, with a copy to the Bureau, of its intent to request to terminate and/or substitute, and the reason for the request. The Contractor shall give the DBE five days to respond to the Contractor's notice. The DBE so notified shall advise the Bureau and the Contractor of the reasons, if any, why it objects to the proposed termination of its subcontract and why the Bureau should not approve the Contractor's action. If required in a particular case as a matter of public necessity, the Bureau may provide a response period shorter than five days.

For purposes of this paragraph, good cause includes the following circumstances:

- (1) The listed DBE subcontractor fails or refuses to execute a written contract;
- (2) The listed DBE subcontractor fails or refuses to perform the work of its subcontract in a way consistent with normal industry standards. Provided, however, that good cause does not exist if the failure or refusal of the DBE subcontractor to perform its work on the subcontract results from the bad faith or discriminatory action of the prime contractor;
- (3) The listed DBE subcontractor fails or refuses to meet the prime Contractor's reasonable, nondiscriminatory bond requirements;
- (4) The listed DBE subcontractor becomes bankrupt, insolvent, or exhibits credit unworthiness;
- (5) The listed DBE subcontractor is ineligible to work on public works projects because of suspension and debarment proceedings pursuant 2 CFR Parts 180, 215 and 1,200 or applicable state law.
- (6) You have determined that the listed DBE subcontractor is not a responsible contractor;
- (7) The listed DBE subcontractor voluntarily withdraws from the projects and provides to you written notice of its withdrawal;
- (8) The listed DBE is ineligible to receive DBE credit for the type of work required;
- (9) A DBE owner dies or becomes disabled with the result that the listed DBE contractor is unable to complete its work on the contract;
- (10) Other documented good cause that compels the termination of the DBE subcontractor. Provided, that good cause does not exist if the prime Contractor seeks to terminate a DBE it relied upon to obtain the contract so that the prime

Contractor can self-perform the work for which the DBE contractor was engaged or so that the prime Contractor can substitute another DBE or non-DBE contractor after contract award.

When a DBE is terminated, or fails to complete its work on the Contract for any reason the Contractor shall make a good faith effort to find another DBE to substitute for the original DBE to perform at least the same amount of work under the contract as the terminated DBE to the extent needed to meet the established Contract goal. The good faith efforts shall be documented by the Contractor. If the Department requests documentation under this provision, the Contractor shall submit the documentation within seven days, which may be extended for an additional seven days if necessary at the request of the Contractor. The Department shall provide a written determination to the Contractor stating whether or not good faith efforts have been demonstrated.

- (f) PAYMENT RECORDS. The Contractor shall maintain a record of payments for work performed to the DBE participants. The records shall be made available to the Department for inspection upon request. After the performance of the final item of work or delivery of material by a DBE and final payment therefore to the DBE by the Contractor, but not later than thirty calendar days after payment has been made by the Department to the Contractor for such work or material, the Contractor shall submit a DBE Payment Agreement on Department form SBE 2115 to the Regional Engineer. If full and final payment has not been made to the DBE, the DBE Payment Agreement shall indicate whether a disagreement as to the payment required exists between the Contractor and the DBE or if the Contractor believes that the work has not been satisfactorily completed. If the Contractor does not have the full amount of work indicated in the Utilization Plan performed by the DBE companies indicated in the Utilization Plan and after good faith efforts are reviewed, the Department may deduct from contract payments to the Contractor the amount of the goal not achieved as liquidated and ascertained damages. The Contractor may request an administrative reconsideration of any amount deducted as damages pursuant to subsection (h) of this part.
- (g) ENFORCEMENT. The Department reserves the right to withhold payment to the Contractor to enforce the provisions of this Special Provision. Final payment shall not be made on the contract until such time as the Contractor submits sufficient documentation demonstrating achievement of the goal in accordance with this Special Provision or after liquidated damages have been determined and collected.
- (h) RECONSIDERATION. Notwithstanding any other provision of the contract, including but not limited to Article 109.09 of the Standard Specifications, the Contractor may request administrative reconsideration of a decision to deduct the amount of the goal not achieved as liquidated damages. A request to reconsider shall be delivered to the Contract Compliance Section and shall be handled and considered in the same manner as set forth in paragraph (c) of "Good Faith Effort Procedures" of this Special Provision, except a final decision that a good faith effort was not made during contract performance

to achieve the goal agreed to in the Utilization Plan shall be the final administrative decision of the Department.

80029

EQUAL EMPLOYMENT OPPORTUNITY (BDE)

Effective: April 1, 2015

FEDERAL AID CONTRACTS. Revise the following section of Check Sheet #1 of the Recurring Special Provisions to read:

"EQUAL EMPLOYMENT OPPORTUNITY

In the event of the Contractor's noncompliance with the provisions of this Equal Employment Opportunity Clause, the Illinois Human Rights Act, or the Illinois Department of Human Rights Rules and Regulations, the Contractor may be declared ineligible for future contracts or subcontracts with the State of Illinois or any of its political sub-divisions or municipal corporations, and the contract may be cancelled or voided in whole or in part, and such other sanctions or penalties may be imposed or remedies invoked as provided by statute or regulation.

During the performance of this Contract, the Contractor agrees as follows:

- (1) That it will not discriminate against any employee or applicant for employment because of race, color, religion, sex, sexual orientation, marital status, order of protection status, national origin or ancestry, citizenship status, age, physical or mental disability unrelated to ability, military status, or an unfavorable discharge from military service; and further that it will examine all job classifications to determine if minority persons or women are underutilized and will take appropriate affirmative action to rectify any such underutilization.
- (2) That, if it hires additional employees in order to perform this contract or any portion hereof, it will determine the availability (according to the Illinois Department of Human Rights Rules and Regulations) of minorities and women in the area(s) from which it may reasonably recruit and it will hire for each job classification for which employees are hired in such a way that minorities and women are not underutilized.
- (3) That, in all solicitations or advertisements for employees placed by it or on its behalf, it will state that all applicants will be afforded equal opportunity without discrimination because of race, color, religion, sex, sexual orientation, marital status, order of protection status, national origin or ancestry, citizenship status, age, physical or mental disability unrelated to ability, military status or an unfavorable discharge from military service.
- (4) That it will send to each labor organization or representative of workers with which it has or is bound by a collective bargaining or other agreement or understanding, a notice advising such labor organization or representative of the Contractor's obligations under the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations. If any labor organization or representative fails or refuses to cooperate with the Contractor in its efforts to comply with such Act and Rules and Regulations, the

Contractor will promptly so notify the Illinois Department of Human Rights and IDOT and will recruit employees from other sources when necessary to fulfill its obligations thereunder.

- (5) That it will submit reports as required by the Illinois Department of Human Rights Rules and Regulations, furnish all relevant information as may from time to time be requested by the Illinois Department of Human Rights or IDOT, and in all respects comply with the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations.
- (6) That it will permit access to all relevant books, records, accounts, and work sites by personnel of IDOT and the Illinois Department of Human Rights for purposes of investigation to ascertain compliance with the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations.
- (7) That it will include verbatim or by reference the provisions of this clause in every subcontract it awards under which any portion of the contract obligations are undertaken or assumed, so that the provisions will be binding upon the subcontractor. In the same manner as with other provisions of this contract, the Contractor will be liable for compliance with applicable provisions of this clause by subcontractors; and further it will promptly notify IDOT and the Illinois Department of Human Rights in the event any subcontractor fails or refuses to comply with these provisions. In addition, the Contractor will not utilize any subcontractor declared by the Illinois Human Rights Commission to be ineligible for contracts or subcontracts with the State of Illinois or any of its political subdivisions or municipal corporations."

STATE CONTRACTS. Revise Section II of Check Sheet #5 of the Recurring Special Provisions to read:

"II. EQUAL EMPLOYMENT OPPORTUNITY

In the event of the Contractor's noncompliance with the provisions of this Equal Employment Opportunity Clause, the Illinois Human Rights Act or the Illinois Department of Human Rights Rules and Regulations, the Contractor may be declared ineligible for future contracts or subcontracts with the State of Illinois or any of its political sub-divisions or municipal corporations, and the contract may be cancelled or voided in whole or in part, and such other sanctions or penalties may be imposed or remedies invoked as provided by statute or regulation.

During the performance of this Contract, the Contractor agrees as follows:

1. That it will not discriminate against any employee or applicant for employment because of race, color, religion, sex, sexual orientation, marital status, order of protection status, national origin or ancestry, citizenship status, age, physical or mental disability unrelated to ability, military status, or an unfavorable discharge from military service; and further

that it will examine all job classifications to determine if minority persons or women are underutilized and will take appropriate affirmative action to rectify any such underutilization.

2. That, if it hires additional employees in order to perform this contract or any portion hereof, it will determine the availability (according to the Illinois Department of Human Rights Rules and Regulations) of minorities and women in the area(s) from which it may reasonably recruit and it will hire for each job classification for which employees are hired in such a way that minorities and women are not underutilized.
3. That, in all solicitations or advertisements for employees placed by it or on its behalf, it will state that all applicants will be afforded equal opportunity without discrimination because of race, color, religion, sex, sexual orientation, marital status, order of protection status, national origin or ancestry, citizenship status, age, physical or mental disability unrelated to ability, military status, or an unfavorable discharge from military service.
4. That it will send to each labor organization or representative of workers with which it has or is bound by a collective bargaining or other agreement or understanding, a notice advising such labor organization or representative of the Contractor's obligations under the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations. If any labor organization or representative fails or refuses to cooperate with the Contractor in its efforts to comply with such Act and Rules and Regulations, the Contractor will promptly so notify the Illinois Department of Human Rights and IDOT and will recruit employees from other sources when necessary to fulfill its obligations thereunder.
5. That it will submit reports as required by the Illinois Department of Human Rights Rules and Regulations, furnish all relevant information as may from time to time be requested by the Illinois Department of Human Rights or IDOT, and in all respects comply with the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations.
6. That it will permit access to all relevant books, records, accounts and work sites by personnel of IDOT and the Illinois Department of Human Rights for purposes of investigation to ascertain compliance with the Illinois Human Rights Act and the Illinois Department of Human Rights Rules and Regulations.
7. That it will include verbatim or by reference the provisions of this clause in every subcontract it awards under which any portion of the contract obligations are undertaken or assumed, so that the provisions will be binding upon the subcontractor. In the same manner as with other provisions of this contract, the Contractor will be liable for compliance with applicable provisions of this clause by subcontractors; and further it will promptly notify IDOT and the Illinois Department of Human Rights in the event any subcontractor fails or refuses to comply with these provisions. In addition, the Contractor will not utilize any subcontractor declared by the Illinois Human Rights

Commission to be ineligible for contracts or subcontracts with the State of Illinois or any of its political subdivisions or municipal corporations.”

80358

FRICTION AGGREGATE (BDE)

Effective: January 1, 2011

Revised: November 1, 2014

Revise Article 1004.01(a)(4) of the Standard Specifications to read:

“(4) Crushed Stone. Crushed stone shall be the angular fragments resulting from crushing undisturbed, consolidated deposits of rock by mechanical means. Crushed stone shall be divided into the following, when specified.

- a. Carbonate Crushed Stone. Carbonate crushed stone shall be either dolomite or limestone. Dolomite shall contain 11.0 percent or more magnesium oxide (MgO). Limestone shall contain less than 11.0 percent magnesium oxide (MgO).
- b. Crystalline Crushed Stone. Crystalline crushed stone shall be either metamorphic or igneous stone, including but is not limited to, quartzite, granite, rhyolite and diabase.”

Revise Article 1004.03(a) of the Standard Specifications to read:

“**1004.03 Coarse Aggregate for Hot-Mix Asphalt (HMA).** The aggregate shall be according to Article 1004.01 and the following.

(a) Description. The coarse aggregate for HMA shall be according to the following table.

Use	Mixture	Aggregates Allowed
Class A	Seal or Cover	<u>Allowed Alone or in Combination</u> ^{5/} : Gravel Crushed Gravel Carbonate Crushed Stone Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag Crushed Concrete

Use	Mixture	Aggregates Allowed	
HMA Low ESAL	Stabilized Subbase or Shoulders	<u>Allowed Alone or in Combination</u> ^{5/} : Gravel Crushed Gravel Carbonate Crushed Stone Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{1/} Crushed Concrete	
HMA High ESAL Low ESAL	Binder IL-19.0 or IL-19.0L SMA Binder	<u>Allowed Alone or in Combination</u> ^{5/} : Crushed Gravel Carbonate Crushed Stone ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Concrete ^{3/}	
HMA High ESAL Low ESAL	C Surface and Leveling Binder IL-9.5 or IL-9.5L SMA Ndesign 50 Surface	<u>Allowed Alone or in Combination</u> ^{5/} : Crushed Gravel Carbonate Crushed Stone ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{4/} Crushed Concrete ^{3/}	
HMA High ESAL	D Surface and Leveling Binder IL-9.5 SMA Ndesign 50 Surface	<u>Allowed Alone or in Combination</u> ^{5/} : Crushed Gravel Carbonate Crushed Stone (other than Limestone) ^{2/} Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag ^{4/} Crushed Concrete ^{3/}	
		<u>Other Combinations Allowed:</u>	
		<i>Up to...</i>	<i>With...</i>
		25% Limestone	Dolomite

Use	Mixture	Aggregates Allowed	
		50% Limestone	Any Mixture D aggregate other than Dolomite
		75% Limestone	Crushed Slag (ACBF) or Crushed Sandstone
HMA High ESAL	E Surface IL-9.5 SMA Ndesign 80 Surface	<u>Allowed Alone or in Combination</u> ^{5/} :	
		Crushed Gravel Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag Crushed Concrete ^{3/} No Limestone.	
		<u>Other Combinations Allowed:</u>	
		<i>Up to...</i>	<i>With...</i>
		50% Dolomite ^{2/}	Any Mixture E aggregate
		75% Dolomite ^{2/}	Crushed Sandstone, Crushed Slag (ACBF), Crushed Steel Slag, or Crystalline Crushed Stone
HMA High ESAL	F Surface IL-9.5 SMA Ndesign 80 Surface	75% Crushed Gravel or Crushed Concrete ^{3/}	Crushed Sandstone, Crystalline Crushed Stone, Crushed Slag (ACBF), or Crushed Steel Slag
		<u>Allowed Alone or in Combination</u> ^{5/} :	
		Crystalline Crushed Stone Crushed Sandstone Crushed Slag (ACBF) Crushed Steel Slag No Limestone.	
		<u>Other Combinations Allowed:</u>	

Use	Mixture	Aggregates Allowed	
		<i>Up to...</i>	<i>With...</i>
		50% Crushed Gravel, Crushed Concrete ^{3/} , or Dolomite ^{2/}	Crushed Sandstone, Crushed Slag (ACBF), Crushed Steel Slag, or Crystalline Crushed Stone

- 1/ Crushed steel slag allowed in shoulder surface only.
- 2/ Carbonate crushed stone shall not be used in SMA Ndesign 80. In SMA Ndesign 50, carbonate crushed stone shall not be blended with any of the other aggregates allowed alone in Ndesign 50 SMA binder or Ndesign 50 SMA surface.
- 3/ Crushed concrete will not be permitted in SMA mixes.
- 4/ Crushed steel slag shall not be used as leveling binder.
- 5/ When combinations of aggregates are used, the blend percent measurements shall be by volume."

FUEL COST ADJUSTMENT (BDE) (RETURN FORM WITH BID)

Effective: April 1, 2009

Revised: July 1, 2015

Description. Fuel cost adjustments will be made to provide additional compensation to the Contractor, or a credit to the Department, for fluctuations in fuel prices when optioned by the Contractor. The bidder shall indicate on the attached form whether or not this special provision will be part of the contract and submit the completed form with his/her bid. Failure to submit the form or failure to indicate contract number, company name and sign and date the form shall make this contract exempt of fuel cost adjustments for all categories of work. Failure to indicate "Yes" for any category of work will make that category of work exempt from fuel cost adjustment.

General. The fuel cost adjustment shall apply to contract pay items as grouped by category. The adjustment shall only apply to those categories of work checked "Yes", and only when the cumulative plan quantities for a category exceed the required threshold. Adjustments to work items in a category, either up or down, and extra work paid for by agreed unit price will be subject to fuel cost adjustment only when the category representing the added work was subject to the fuel cost adjustment. Extra work paid for at a lump sum price or by force account will not be subject to fuel cost adjustment. Category descriptions and thresholds for application and the fuel usage factors which are applicable to each are as follows:

(a) Categories of Work.

- (1) Category A: Earthwork. Contract pay items performed under Sections 202, 204, and 206 including any modified standard or nonstandard items where the character of the work to be performed is considered earthwork. The cumulative total of all applicable item plan quantities shall exceed 25,000 cu yd (20,000 cu m). Included in the fuel usage factor is a weighted average 0.10 gal/cu yd (0.50 liters/cu m) factor for trucking.
- (2) Category B: Subbases and Aggregate Base Courses. Contract pay items constructed under Sections 311, 312 and 351 including any modified standard or nonstandard items where the character of the work to be performed is considered construction of a subbase or aggregate, stabilized or modified base course. The cumulative total of all applicable item plan quantities shall exceed 5000 tons (4500 metric tons). Included in the fuel usage factor is a 0.60 gal/ton (2.50 liters/metric ton) factor for trucking.
- (3) Category C: Hot-Mix Asphalt (HMA) Bases, Pavements and Shoulders. Contract pay items constructed under Sections 355, 406, 407 and 482 including any modified standard or nonstandard items where the character of the work to be performed is considered HMA bases, pavements and shoulders. The cumulative total of all applicable item plan quantities shall exceed 5000 tons (4500 metric tons). Included in the fuel usage factor is 0.60 gal/ton (2.50 liters/metric ton) factor for trucking.

- (4) Category D: Portland Cement Concrete (PCC) Bases, Pavements and Shoulders. Contract pay items constructed under Sections 353, 420, 421 and 483 including any modified standard or nonstandard items where the character of the work to be performed is considered PCC base, pavement or shoulder. The cumulative total of all applicable item plan quantities shall exceed 7500 sq yd (6000 sq m). Included in the fuel usage factor is 1.20 gal/cu yd (5.94 liters/cu m) factor for trucking.
- (5) Category E: Structures. Structure items having a cumulative bid price that exceeds \$250,000 for pay items constructed under Sections 502, 503, 504, 505, 512, 516 and 540 including any modified standard or nonstandard items where the character of the work to be performed is considered structure work when similar to that performed under these sections and not included in categories A through D.

(b) Fuel Usage Factors.

English Units		
Category	Factor	Units
A - Earthwork	0.34	gal / cu yd
B – Subbase and Aggregate Base courses	0.62	gal / ton
C – HMA Bases, Pavements and Shoulders	1.05	gal / ton
D – PCC Bases, Pavements and Shoulders	2.53	gal / cu yd
E – Structures	8.00	gal / \$1000

Metric Units		
Category	Factor	Units
A - Earthwork	1.68	liters / cu m
B – Subbase and Aggregate Base courses	2.58	liters / metric ton
C – HMA Bases, Pavements and Shoulders	4.37	liters / metric ton
D – PCC Bases, Pavements and Shoulders	12.52	liters / cu m
E – Structures	30.28	liters / \$1000

(c) Quantity Conversion Factors.

Category	Conversion	Factor
B	sq yd to ton	0.057 ton / sq yd / in depth
	sq m to metric ton	0.00243 metric ton / sq m / mm depth
C	sq yd to ton	0.056 ton / sq yd / in depth
	sq m to metric ton	0.00239 m ton / sq m / mm depth
D	sq yd to cu yd	0.028 cu yd / sq yd / in depth
	sq m to cu m	0.001 cu m / sq m / mm depth

Method of Adjustment. Fuel cost adjustments will be computed as follows.

$$CA = (FPI_P - FPI_L) \times FUF \times Q$$

Where: CA = Cost Adjustment, \$
 FPI_P = Fuel Price Index, as published by the Department for the month the work is performed, \$/gal (\$/liter)
 FPI_L = Fuel Price Index, as published by the Department for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price, \$/gal (\$/liter)
 FUF = Fuel Usage Factor in the pay item(s) being adjusted
 Q = Authorized construction Quantity, tons (metric tons) or cu yd (cu m)

The entire FUF indicated in paragraph (b) will be used regardless of use of trucking to perform the work.

Basis of Payment. Fuel cost adjustments may be positive or negative but will only be made when there is a difference between the FPI_L and FPI_P in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(FPI_L - FPI_P) \div FPI_L\} \times 100$$

Fuel cost adjustments will be calculated for each calendar month in which applicable work is performed; and will be paid or deducted when all other contract requirements for the items of work are satisfied. The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

Return With Bid

**ILLINOIS DEPARTMENT
OF TRANSPORTATION**

**OPTION FOR
FUEL COST ADJUSTMENT**

The bidder shall submit this completed form with his/her bid. Failure to submit the form or properly complete contract number, company name, and sign and date the form shall make this contract exempt of fuel cost adjustments in all categories. Failure to indicate "Yes" for any category of work at the time of bid will make that category of work exempt from fuel cost adjustment. After award, this form, when submitted shall become part of the contract.

Contract No.: _____

Company Name: _____

Contractor's Option:

Is your company opting to include this special provision as part of the contract plans for the following categories of work?

Category A Earthwork. Yes ☐

Category B Subbases and Aggregate Base Courses Yes ☐

Category C HMA Bases, Pavements and Shoulders Yes ☐

Category D PCC Bases, Pavements and Shoulders Yes ☐

Category E Structures Yes ☐

Signature: _____ **Date:** _____

HOT-MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS (BDE)

Effective: January 1, 2010

Revised: April 1, 2012

Description. This work shall consist of testing the density of longitudinal joints as part of the quality control/quality assurance (QC/QA) of hot-mix asphalt (HMA). Work shall be according to Section 1030 of the Standard Specifications except as follows.

Quality Control/Quality Assurance (QC/QA). Delete the second and third sentence of the third paragraph of Article 1030.05(d)(3) of the Standard Specifications.

Add the following paragraphs to the end of Article 1030.05(d)(3) of the Standard Specifications:

“Longitudinal joint density testing shall be performed at each random density test location. Longitudinal joint testing shall be located at a distance equal to the lift thickness or a minimum of 4 in. (100 mm), from each pavement edge. (i.e. for a 5 in. (125 mm) lift the near edge of the density gauge or core barrel shall be within 5 in. (125 mm) from the edge of pavement.) Longitudinal joint density testing shall be performed using either a correlated nuclear gauge or cores.

- a. Confined Edge. Each confined edge density shall be represented by a one-minute nuclear density reading or a core density and shall be included in the average of density readings or core densities taken across the mat which represents the Individual Test.
- b. Unconfined Edge. Each unconfined edge joint density shall be represented by an average of three one-minute density readings or a single core density at the given density test location and shall meet the density requirements specified herein. The three one-minute readings shall be spaced ten feet apart longitudinally along the unconfined pavement edge and centered at the random density test location.”

Revise the Density Control Limits table in Article 1030.05(d)(4) of the Standard Specifications to read:

“Mixture Composition	Parameter	Individual Test (includes confined edges)	Unconfined Edge Joint Density Minimum
IL-4.75	Ndesign = 50	93.0 – 97.4%	91.0%
IL-9.5, IL-12.5	Ndesign ≥ 90	92.0 – 96.0%	90.0%
IL-9.5, IL-9.5L, IL-12.5	Ndesign < 90	92.5 – 97.4%	90.0%
IL-19.0, IL-25.0	Ndesign ≥ 90	93.0 – 96.0%	90.0%
IL-19.0, IL-19.0L, IL-25.0	Ndesign < 90	93.0 – 97.4%	90.0%

SMA	Ndesign = 50 & 80	93.5 – 97.4%	91.0%
All Other	Ndesign = 30	93.0 - 97.4%	90.0%”

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HOT MIX ASPHALT – PRIME COAT (BDE)

Effective: November 1, 2014

Revise Note 1 of Article 406.02 of the Standard Specifications to read:

“Note 1. The bituminous material used for prime coat shall be one of the types listed in the following table.

When emulsified asphalts are used, any dilution with water shall be performed by the emulsion producer. The emulsified asphalt shall be thoroughly agitated within 24 hours of application and show no separation of water and emulsion.

Application	Bituminous Material Types
Prime Coat on Brick, Concrete, or HMA Bases	SS-1, SS-1h, SS-1hP, SS-1vh, RS-1, RS-2, CSS-1, CSS-1h, CSS-1hp, CRS-1, CRS-2, HFE-90, RC-70
Prime Coat on Aggregate Bases	MC-30, PEP”

Add the following to Article 406.03 of the Standard Specifications.

- “(i) Vacuum Sweeper 1101.19
(j) Spray Paver 1102.06”

Revise Article 406.05(b) of the Standard Specifications to read:

“(b) Prime Coat. The bituminous material shall be prepared according to Article 403.05 and applied according to Article 403.10. The use of RC-70 shall be limited to air temperatures less than 60 °F (15 °C).

- (1) Brick, Concrete or HMA Bases. The base shall be cleaned of all dust, debris and any substance that will prevent the prime coat from adhering to the base. Cleaning shall be accomplished by sweeping to remove all large particles and air blasting to remove dust. As an alternative to air blasting, a vacuum sweeper may be used to accomplish the dust removal. The base shall be free of standing water at the time of application. The prime coat shall be applied uniformly and at a rate that will provide a residual asphalt rate on the prepared surface as specified in the following table.

Type of Surface to be Primed	Residual Asphalt Rate lb/sq ft (kg/sq m)
Milled HMA, Aged Non-Milled HMA, Milled Concrete, Non-Milled Concrete & Tined Concrete	0.05 (0.244)
Fog Coat between HMA Lifts, IL-4.75 & Brick	0.025 (0.122)

The bituminous material for the prime coat shall be placed one lane at a time. If a spray paver is not used, the primed lane shall remain closed until the prime coat is

fully cured and does not pickup under traffic. When placing prime coat through an intersection where it is not possible to keep the lane closed, the prime coat may be covered immediately following its application with fine aggregate mechanically spread at a uniform rate of 2 to 4 lb/sq yd (1 to 2 kg/sq m).

- (2) Aggregate Bases. The prime coat shall be applied uniformly and at a rate that will provide a residual asphalt rate on the prepared surface of 0.25 lb/sq ft \pm 0.01 (1.21 kg/sq m \pm 0.05).

The prime coat shall be permitted to cure until the penetration has been approved by the Engineer, but at no time shall the curing period be less than 24 hours for MC-30 or four hours for PEP. Pools of prime occurring in the depressions shall be broomed or squeegeed over the surrounding surface the same day the prime coat is applied.

The base shall be primed 1/2 width at a time. The prime coat on the second half/width shall not be applied until the prime coat on the first half/width has cured so that it will not pickup under traffic.

The residual asphalt rate will be verified a minimum of once per type of surface to be primed as specified herein for which at least 2000 tons (1800 metric tons) of HMA will be placed. The test will be according to the "Determination of Residual Asphalt in Prime and Tack Coat Materials" test procedure.

Prime coat shall be fully cured prior to placement of HMA to prevent pickup by haul trucks or paving equipment. If pickup occurs, paving shall cease in order to provide additional cure time, and all areas where the pickup occurred shall be repaired.

If after five days, loss of prime coat is evident prior to covering with HMA, additional prime coat shall be placed as determined by the Engineer at no additional cost to the Department."

Revise the last sentence of the first paragraph of Article 406.13(b) of the Standard Specifications to read:

"Water added to emulsified asphalt, as allowed in Article 406.02, will not be included in the quantities measured for payment."

Revise the second paragraph of Article 406.13(b) of the Standard Specifications to read:

"Aggregate for covering prime coat will not be measured for payment."

Revise the first paragraph of Article 406.14 of the Standard Specifications to read:

"406.14 Basis of Payment. Prime Coat will be paid for at the contract unit price per pound (kilogram) of residual asphalt applied for BITUMINOUS MATERIALS (PRIME COAT), or POLYMERIZED BITUMINOUS MATERIALS (PRIME COAT)."

Revise Article 407.02 of the Standard Specifications to read:

“407.02 Materials. Materials shall be according to Article 406.02, except as follows.

Item	Article/Section
(a) Packaged Rapid Hardening Mortar or Concrete	1018”

Revise Article 407.06(b) of the Standard Specifications to read:

“(b) A bituminous prime coat shall be applied between each lift of HMA according to Article 406.05(b).”

Delete the second paragraph of Article 407.12 of the Standard Specifications.

Revise the first paragraph of Article 408.04 of the Standard Specifications to read:

“408.04 Method of Measurement. Bituminous priming material will be measured for payment according to Article 406.13.”

Revise the first paragraph of Article 408.05 of the Standard Specifications to read:

“408.05 Basis of Payment. This work will be paid for at the contract unit price per pound (kilogram) of residual asphalt applied for BITUMINOUS MATERIALS (PRIME COAT) or POLYMERIZED BITUMINOUS MATERIALS (PRIME COAT) and at the contract unit price per ton (metric ton) for INCIDENTAL HOT-MIX ASPHALT SURFACING.”

Revise Article 1032.02 of the Standard Specifications to read:

“1032.02 Measurement. Asphalt binders, emulsified asphalts, rapid curing liquid asphalt, medium curing liquid asphalts, slow curing liquid asphalts, asphalt fillers, and road oils will be measured by weight.

A weight ticket for each truck load shall be furnished to the inspector. The truck shall be weighed at a location approved by the Engineer. The ticket shall show the weight of the empty truck (the truck being weighed each time before it is loaded), the weight of the loaded truck, and the net weight of the bituminous material.

When an emulsion or cutback is used for prime coat, the percentage of asphalt residue of the actual certified product shall be shown on the producer’s bill of lading or attached certificate of analysis. If the producer adds extra water to an emulsion at the request of the purchaser, the amount of water shall also be shown on the bill of lading.

Payment will not be made for bituminous materials in excess of 105 percent of the amount specified by the Engineer.”

Add the following to the table in Article 1032.04 of the Standard Specifications.

"SS-1vh	160-180	70-80
RS-1, CRS-1	75-130	25-55"

Add the following to Article 1032.06 of the Standard Specifications.

"(g) Non Tracking Emulsified Asphalt SS-1vh shall be according to the following.

Requirements for SS-1vh			
Test		SPEC	AASHTO Test Method
Saybolt Viscosity @ 25C,	SFS	20-200	T 72
Storage Stability, 24hr.,	%	1 max.	T 59
Residue by Evaporation,	%	50 min.	T 59
Sieve Test,	%	0.3 max.	T 59
Tests on Residue from Evaporation			
Penetration @25°C, 100g., 5 sec., dmm		20 max.	T 49
Softening Point,	°C	65 min.	T 53
Solubility,	%	97.5 min.	T 44
Orig. DSR @ 82°C,	kPa	1.00 min.	T 315"

Revise the last table in Article 1032.06(f)(2)d. of the Standard Specifications to read:

"Grade	Use
SS-1, SS-1h, RS-1, RS-2, CSS-1, CRS-1, CRS-2, CSS-1h, HFE-90, SS-1hP, CSS-1hP, SS-1vh	Prime or fog seal
PEP	Bituminous surface treatment prime
RS-2, HFE-90, HFE-150, HFE- 300, CRSP, HFP, CRS-2, HFRS-2	Bituminous surface treatment
CSS-1h Latex Modified	Microsurfacing"

Add the following to Article 1101 of the Standard Specifications.

"1101.19 Vacuum Sweeper. The vacuum sweeper shall have a minimum sweeping path of 52 in. (1.3 m) and a minimum blower rating of 20,000 cu ft per minute (566 cu m per minute)."

Add the following to Article 1102 of the Standard Specifications:

"1102.06 Spray Paver. The spreading and finishing machine shall be capable of spraying a rapid setting emulsion tack coat, paving a layer of HMA, and providing a smooth HMA mat in one pass. The HMA shall be spread over the tack coat in less than five seconds after the

application of the tack coat during normal paving speeds. No wheel or other part of the paving machine shall come into contact with the tack coat before the HMA is applied. In addition to meeting the requirements of Article 1102.03, the spray paver shall also meet the requirements of Article 1102.05 for the tank, heating system, pump, thermometer, tachometer or synchronizer, and calibration. The spray bar shall be equipped with properly sized and spaced nozzles to apply a uniform application of tack coat at the specified rate for the full width of the mat being placed.”

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LRFD PIPE CULVERT BURIAL TABLES (BDE)

Effective: November 1, 2013

Revised: April 1, 2015

Revise Article 542.02 of the Standard Specifications to read as follows:

"Item	Article/Section
(a) Galvanized Corrugated Steel Pipe	1006.01
(b) Galvanized Corrugated Steel Pipe Arch	1006.01
(c) Bituminous Coated Corrugated Steel Pipe	1006.01
(d) Bituminous Coated Corrugated Steel Pipe Arch	1006.01
(e) Reserved	
(f) Aluminized Steel Type 2 Corrugated Pipe	1006.01
(g) Aluminized Steel Type 2 Corrugated Pipe Arch	1006.01
(h) Precoated Galvanized Corrugated Steel Pipe	1006.01
(i) Precoated Galvanized Corrugated Steel Pipe Arch	1006.01
(j) Corrugated Aluminum Alloy Pipe	1006.03
(k) Corrugated Aluminum Alloy Pipe Arch	1006.03
(l) Extra Strength Clay Pipe	1040.02
(m) Concrete Sewer, Storm Drain, and Culvert Pipe	1042
(n) Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe	1042
(o) Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe.....	1042
(p) Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe	1042
(q) Polyvinyl Chloride (PVC) Pipe	1040.03
(r) Corrugated Polyvinyl Chloride (PVC) Pipe with a Smooth Interior	1040.03
(s) Corrugated Polypropylene (CPP) pipe with smooth Interior	1040.08
(t) Corrugated Polyethylene (PE) Pipe with a Smooth Interior	1040.04
(u) Polyethylene (PE) Pipe with a Smooth Interior	1040.04
(v) Rubber Gaskets and Preformed Flexible Joint Sealants for Concrete Pipe	1056
(w) Mastic Joint Sealer for Pipe	1055
(x) External Sealing Band	1057
(y) Fine Aggregate (Note 1)	1003.04
(z) Coarse Aggregate (Note 2)	1004.05
(aa) Packaged Rapid Hardening Mortar or Concrete	1018
(bb) Nonshrink Grout	1024.02
(cc) Reinforcement Bars and Welded Wire Fabric	1006.10
(dd) Handling Hole Plugs	1042.16

Note 1. The fine aggregate shall be moist.

Note 2. The coarse aggregate shall be wet."

Revise the table for permitted materials in Article 542.03 of the Standard Specifications as follows:

"Class	Materials
A	Rigid Pipes: Extra Strength Clay Pipe Concrete Sewer Storm Drain and Culvert Pipe, Class 3 Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe
C	Rigid Pipes: Extra Strength Clay Pipe Concrete Sewer Storm Drain and Culvert Pipe, Class 3 Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe Flexible Pipes: Aluminized Steel Type 2 Corrugated Pipe Aluminized Steel Type 2 Corrugated Pipe Arch Precoated Galvanized Corrugated Steel Pipe Precoated Galvanized Corrugated Steel Pipe Arch Corrugated Aluminum Alloy Pipe Corrugated Aluminum Alloy Pipe Arch Polyvinyl Chloride (PVC) Pipe Corrugated Polyvinyl Chloride (PVC) Pipe with a Smooth Interior Polyethylene (PE) Pipe with a Smooth Interior Corrugated Polypropylene (CPP) Pipe with Smooth Interior
D	Rigid Pipes: Extra Strength Clay Pipe Concrete Sewer Storm Drain and Culvert Pipe, Class 3 Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe Flexible Pipes: Galvanized Corrugated Steel Pipe Galvanized Corrugated Steel Pipe Arch Bituminous Coated Corrugated Steel Pipe Bituminous Coated Corrugated Steel Pipe Arch Aluminized Steel Type 2 Corrugated Pipe Aluminized Steel Type 2 Corrugated Pipe Arch Precoated Galvanized Corrugated Steel Pipe Precoated Galvanized Corrugated Steel Pipe Arch Corrugated Aluminum Alloy Pipe Corrugated Aluminum Alloy Pipe Arch Polyvinyl Chloride (PVC) Pipe Corrugated Polyvinyl Chloride (PVC) Pipe with a Smooth Interior Corrugated Polyethylene (PE) Pipe with a Smooth Interior Polyethylene (PE) Pipe with a Smooth Interior" Corrugated Polypropylene (CPP) Pipe with Smooth Interior

Revise Articles 542.03(b) and (c) of the Standard Specifications to read:

“(b) Extra strength clay pipe will only be permitted for pipe culverts Type 1, for 10 in., 12 in., 42 in. and 48 in. (250 mm, 300 mm, 1050 mm and 1200 mm), Types 2, up to and including 48 in. (1200 mm), Type 3, up to and including 18 in. (450 mm), Type 4 up to and including 10 in. (250 mm), for all pipe classes.

(c) Concrete sewer, storm drain, and culvert pipe Class 3 will only be permitted for pipe culverts Type 1, up to and including 10 in (250 mm), Type 2, up to and including 30 in. (750 mm), Type 3, up to and including 15 in. (375 mm); Type 4, up to and including 10 in. (250 mm), for all pipe classes.”

Replace the pipe tables in Article 542.03 of the Standard Specifications with the following:

"Table 1A: Classes of Reinforced Concrete Pipe for the Respective Diameters of Pipe and Fill Heights over the Top of the Pipe"							
Nominal Diameter in.	Type 1	Type 2	Type 3	Type 4	Type 5	Type 6	Type 7
	Fill Height: 3' and less 1' min cover	Fill Height: Greater than 3' not exceeding 10'	Fill Height: Greater than 10' not exceeding 15'	Fill Height: Greater than 15' not exceeding 20'	Fill Height: Greater than 20' not exceeding 25'	Fill Height: Greater than 25' not exceeding 30'	Fill Height: Greater than 30' not exceeding 35'
12	IV	II	III	IV	IV	V	V
15	IV	II	III	IV	IV	V	V
18	IV	II	III	IV	IV	V	V
21	III	II	III	IV	IV	V	V
24	III	II	III	IV	IV	V	V
30	IV	II	III	IV	IV	V	V
36	III	II	III	IV	IV	V	V
42	II	II	III	IV	IV	V	V
48	II	II	III	IV	IV	V	V
54	II	II	III	IV	IV	V	V
60	II	II	III	IV	IV	V	V
66	II	II	III	IV	IV	V	V
72	II	II	III	IV	V	V	V
78	II	II	III	IV	2020	2370	2730
84	II	II	III	IV	2020	2380	2740
90	II	II	III	1680	2030	2390	2750
96	II	III	III	1690	2040	2400	2750
102	II	III	III	1700	2050	2410	2760
108	II	III	1360	1710	2060	2410	2770

Notes:

A number indicates the D-Load for the diameter and depth of fill and that a special design is required.

Design assumptions: Water filled pipe, Type 2 bedding and Class C Walls

Table IA: Classes of Reinforced Concrete Pipe for the Respective Diameters of Pipe and Fill Heights over the Top of the Pipe (Metric)							
Nominal Diameter mm	Type 1	Type 2	Type 3	Type 4	Type 5	Type 6	Type 7
	Fill Height: 1 m and less 0.3 m min cover	Fill Height: Greater than 1 m not exceeding 3 m	Fill Height: Greater than 3 m not exceeding 4.5 m	Fill Height: Greater than 4.5 m not exceeding 6 m	Fill Height: Greater than 6 m not exceeding 7.5 m	Fill Height: Greater than 7.5 m not exceeding 9 m	Fill Height: Greater than 9 m not exceeding 10.5 m
300	IV	II	III	IV	IV	V	V
375	IV	II	III	IV	IV	V	V
450	IV	II	III	IV	IV	V	V
525	III	II	III	IV	IV	V	V
600	III	II	III	IV	IV	V	V
750	IV	II	III	IV	IV	V	V
900	III	II	III	IV	IV	V	V
1050	II	II	III	IV	IV	V	V
1200	II	II	III	IV	IV	V	V
1350	II	II	III	IV	IV	V	V
1500	II	II	III	IV	IV	V	V
1650	II	II	III	IV	IV	V	V
1800	II	II	III	IV	V	V	V
1950	II	II	III	IV	100	110	130
2100	II	II	III	IV	100	110	130
2250	II	II	III	80	100	110	130
2400	II	III	III	80	100	110	130
2550	II	III	III	80	100	120	130
2700	II	III	70	80	100	120	130

Notes:

A number indicates the D-Load for the diameter and depth of fill and that a special design is required.

Design assumptions: Water filled pipe, Type 2 bedding and Class C Walls

TABLE IB: THICKNESS OF CORRUGATED STEEL PIPE FOR THE RESPECTIVE DIAMETER OF PIPE AND FILL HEIGHTS OVER THE TOP OF THE PIPE FOR 2 2/3"x1/2", 3"x1" AND 5"x1" CORRUGATIONS																					
Nominal Diameter in.	Type 1			Type 2			Type 3			Type 4			Type 5			Type 6			Type 7		
	Fill Height:			Fill Height:			Fill Height:			Fill Height:			Fill Height:			Fill Height:			Fill Height:		
	3' and less 1' min. cover			Greater than 3' not exceeding 10'			Greater than 10' not exceeding 15'			Greater than 15' not exceeding 20'			Greater than 20' not exceeding 25'			Greater than 25' not exceeding 30'			Greater than 30' not exceeding 35'		
2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	2 2/3" x 1/2"	3"x1"	5"x1"	
12	0.064			0.064			0.064			0.064			0.064			0.064		0.064			
15	0.064			0.064			0.064			0.064			0.064			0.064		(0.079)			
18	(0.079)			0.064			0.064			0.064			0.064			0.064		(0.079)			
21	(0.079)			0.064			0.064			0.064			(0.079)			(0.079)		(0.079)			
24	(0.079)			0.064			0.064			0.064			(0.079)			(0.079)		(0.109)			
30	(0.109E)			0.064			0.064			(0.079)			(0.079)			(0.109)		0.109			
36	(0.109E)			0.064			(0.079)			(0.079)			(0.109)			0.109		(0.138E)			
42	0.079			0.064			(0.079)			(0.079)			(0.109)			(0.109E)		(0.109E)			
48	0.109	(0.109)	0.109	(0.109)	0.079	0.079	(0.109)	0.079	(0.109)	0.109	(0.109)	0.109	(0.138E)	0.109	0.109	(0.138E)	0.109	(0.138E)	0.109	(0.138)	
54	0.109	(0.109)	0.109	(0.109)	0.079	0.079	0.109	(0.109)	0.109	0.109	(0.109)	0.109	(0.138E)	0.109	(0.138)	(0.138E)	0.109	(0.138E)	0.138	0.138	
60	0.109	0.109	0.109	0.109	0.079	(0.109)	0.109	(0.109)	0.109	0.109	(0.109)	0.109	(0.138E)	0.138	(0.138)	(0.138E)	0.138	(0.138E)	(0.138E)	(0.138E)	
66	(0.138)	0.109	0.109	0.109	0.079	(0.109)	0.109	(0.109)	0.109	0.109	(0.109)	0.109	(0.138E)	0.138	(0.138)	(0.138E)	0.138	(0.138E)	(0.138E)	0.138E	
72	0.138	0.109	(0.138)	0.138	(0.109)	(0.109)	0.138	(0.109)	0.109	0.138	0.109	0.109	(0.168E)	0.138	(0.138)	(0.168E)	0.138E	(0.168E)	(0.138E)	0.138E	
78	0.168	0.109	(0.138)	0.168	(0.109)	0.109	0.168	0.109	0.109	0.168	0.109	(0.138)	0.168	(0.138)	H0.168E	0.138E	0.138E	H0.168E	0.138E	(0.168E)	
84	0.168	(0.138)	(0.138)	0.168	(0.109)	0.109	0.168	0.109	0.109	0.168	0.109	(0.138)	0.168	(0.138)	H0.168E	0.138E	0.138E	H0.168E	(0.168E)	(0.168E)	
90		(0.138)	(0.138)		(0.109)	0.109		0.109	0.109		(0.138)	(0.138)		(0.138)	0.138		(0.168E)	(0.168E)	(0.168E)	(0.168E)	
96		(0.138)	(0.138)		(0.109)	0.109		0.109	0.109		(0.138)	(0.138)		(0.138)	0.138		(0.168E)	(0.168E)	(0.168E)	(0.168E)	
102		0.109Z	0.109Z		(0.109)	0.109		0.109	(0.138)		(0.138)	(0.138)		(0.138)	0.138		(0.168E)	(0.168E)	H0.138E	H0.168E	
108		0.109Z	(0.138Z)		0.109	0.109		0.109	(0.138)		(0.138)	0.138		0.138	(0.168)		(0.168E)	(0.168E)	H0.138E	H0.168E	
114		0.109Z	(0.138Z)		0.109	0.109		0.109	(0.138)		(0.138)	0.138		(0.168)	0.168		(0.168E)	0.168E	H0.138E	H0.168E	
120		0.109Z	(0.138Z)		0.109	0.109		(0.138)	(0.138)		(0.138)	0.138		(0.168)	0.168		H0.138E	H0.168E	H0.168E	H0.168E	
126		0.138Z	0.138Z		0.138	0.138		0.138	0.138		0.138	(0.168)		0.138	(0.168)		H0.138E	H0.168E	H0.168E	H0.168E	
132		0.138Z	0.138Z		0.138	0.138		0.138	0.138		(0.168)	(0.168)		0.168	0.168		H0.138E	H0.168E	H0.168E	H0.168E	
138		0.138Z	0.138Z		0.138	0.138		0.138	0.138		(0.168)	(0.168)		(0.168E)	H0.168E		H0.168E	H0.168E	H0.168E	H0.168E	
144		0.168Z	0.168Z		0.168	0.168		0.168	0.168		0.168	0.168		H0.168E	H0.168E		H0.168E	H0.168E	H0.168E	H0.168E	

Notes:

- * Aluminized Type 2 Steel or Precoated Galvanized Steel shall be required for diameters up to 42" according to Article 1006.01, 1 1/2" x 1/4" corrugations shall be used for diameters less than 12".
- Thicknesses are based on longitudinal riveted seam fabrication, values in "()" can be reduced by one gage thickness if helical seam fabrication is utilized.
- A thickness preceded by "H" indicates only helical seam fabrication is allowed.
- E Elongation according to Article 542.04(e)
- Z 1'-6" Minimum fill

TABLE IB: THICKNESS OF CORRUGATED STEEL PIPE FOR THE RESPECTIVE DIAMETER OF PIPE AND FILL HEIGHTS OVER THE TOP OF THE PIPE FOR 68 mm x 13 mm, 75 mm x 25 mm AND 125 mm x 25 mm CORRUGATIONS (Metric)										
Nominal Diameter mm *	Type 1 Fill Height:		Type 2 Fill Height:		Type 3 Fill Height:		Type 4 Fill Height:		Type 5 Fill Height:	
	1 m and less 0.3 m min. cover	Greater than 1 m not exceeding 3 m	Greater than 3 m not exceeding 4.5 m	Greater than 4.5 m not exceeding 6 m	Greater than 6 m not exceeding 7.5 m	Greater than 7.5 m not exceeding 9 m	Greater than 9 m not exceeding 10.5 m	Fill Height:	Fill Height:	Fill Height:
68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm
300	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63
375	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63	1.63
450	(2.01)	1.63	1.63	1.63	1.63	(2.01)	(2.01)	(2.01)	(2.01)	(2.01)
525	(2.01)	1.63	1.63	1.63	1.63	(2.01)	(2.01)	(2.01)	(2.01)	(2.01)
600	(2.01)	1.63	1.63	1.63	1.63	(2.01)	(2.01)	(2.01)	(2.01)	(2.01)
750	(2.77E)	1.63	1.63	1.63	1.63	(2.77)	(2.77)	(2.77)	(2.77)	(2.77)
900	(2.77E)	1.63	(2.01)	(2.01)	(2.01)	(2.77)	(2.77)	(2.77)	(2.77)	(2.77)
1050	2.01	1.63	(2.01)	(2.01)	(2.01)	(2.77)	(2.77)	(2.77)	(2.77)	(2.77)
1200	2.77	(2.77)	2.01	2.01	2.01	2.77	2.77	2.77	2.77	2.77
1350	2.77	(2.77)	2.01	2.01	2.01	2.77	2.77	2.77	2.77	2.77
1500	2.77	2.77	2.01	2.01	2.01	2.77	2.77	2.77	2.77	2.77
1650	(3.51)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
1800	3.51	2.77	3.51	3.51	3.51	2.77	2.77	2.77	2.77	2.77
1950	4.27	2.77	4.27	4.27	4.27	2.77	2.77	2.77	2.77	2.77
2100	4.27	(3.51)	4.27	4.27	4.27	2.77	2.77	2.77	2.77	2.77
2250	(3.51)	(3.51)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
2400	(3.51)	(3.51)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
2550	2.77Z	2.77Z	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
2700	2.77Z	(3.51Z)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
2850	(3.51Z)	(3.51Z)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
3000	2.77Z	(3.51Z)	2.77	2.77	2.77	2.77	2.77	2.77	2.77	2.77
3150	3.51Z	3.51Z	3.51	3.51	3.51	3.51	3.51	3.51	3.51	3.51
3300	3.51Z	3.51Z	3.51	3.51	3.51	3.51	3.51	3.51	3.51	3.51
3450	3.51Z	3.51Z	3.51	3.51	3.51	3.51	3.51	3.51	3.51	3.51
3600	4.27Z	4.27Z	4.27	4.27	4.27	4.27	4.27	4.27	4.27	4.27

Notes:

* Aluminized Type 2 Steel or Precoated Galvanized Steel shall be required for diameters up to 1050 mm according to Article 1006.01, 38 mm x 6.5 mm corrugations shall be used for diameters less than 300 mm.

Thicknesses are based on longitudinal riveted seam fabrication, values in "()" can be reduced by one gage thickness if helical seam fabrication is utilized.

E A thickness preceded by an "H" indicates only helical seam fabrication is allowed.

Z Elongation according to Article 542.04(e)

450 mm Minimum Fill

TABLE IC: THICKNESS OF CORRUGATED ALUMINUM ALLOY PIPE FOR THE RESPECTIVE DIAMETER OF PIPE AND FILL HEIGHTS OVER THE TOP OF THE PIPE FOR 2 2/3"x1/2" AND 3"x1" CORRUGATIONS														
Nominal Diameter in.	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7	
	Fill Height:		Fill Height:		Fill Height:		Fill Height:		Fill Height:		Fill Height:		Fill Height:	
	3' and less 1' min. cover		Greater than 3' not exceeding 10'		Greater than 10' not exceeding 15'		Greater than 15' not exceeding 20'		Greater than 20' not exceeding 25'		Greater than 25' not exceeding 30'		Greater than 30' not exceeding 35'	
	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"	2 2/3"x1/2"	3"x1"
12	(0.075)		0.060		0.060		0.060		0.060		0.060		0.060	
15	(0.075)		0.060		0.060		0.060		0.060		0.060		(0.075)	
18	(0.075)		0.060		0.060		0.060		0.060		0.060		H 0.060	
21	H 0.060E		0.060		0.060		0.060		0.060		H 0.060		H 0.060E	
24	(0.105E)		0.060		0.060		(0.075)		(0.105)		(0.105)		(0.105E)	
30	H 0.075E	H 0.060	0.075	H 0.060	0.075	H 0.060	(0.105)	H 0.060	(0.105)	H 0.060	H 0.075E	H 0.060	H 0.075E	H 0.060
36	(0.135E)	H 0.060E	0.075	H 0.060	(0.105)	H 0.060	(0.105)	H 0.060	(0.105)	H 0.060	H 0.075E	H 0.060	H 0.075E	H 0.060E
42	0.105E	(0.075)	0.105	0.060	0.105	0.060	0.105	0.060	0.105	(0.075)	0.105E	0.105	0.105E	(0.105E)
48	0.105E	(0.075)	0.105	0.060	0.105	0.060	0.105	0.060	0.105	(0.105)	0.105E	(0.105E)	0.105E	(0.135E)
54	0.105E	(0.105)	0.105	0.060	0.105	0.060	0.105	0.060	0.105	(0.105)	0.105E	(0.105E)	(0.135E)	(0.135E)
60	0.135E	(0.105)	0.135	0.060	0.135	(0.075)	0.135	(0.105)	0.135	(0.105)	0.135E	(0.135E)	(0.164E)	(0.135E)
66	0.164E	(0.105)	0.164	0.060	0.164	(0.075)	0.164	(0.105)	0.164	(0.135)	0.164E	(0.135E)	H 0.164E	(0.135E)
72	0.164E	(0.105)	0.164	0.060	0.164	(0.075)	0.164	(0.105)	0.164	(0.135)	H 0.164E	(0.135E)	H 0.164E	(0.164E)
78	(0.135)	0.075	0.075	0.075	(0.105)	(0.105)	(0.105)	(0.105)	(0.135)	(0.135)	(0.135E)	(0.135E)	(0.164E)	(0.164E)
84	(0.135)	0.105	0.105	0.105	0.105	0.105	(0.135)	(0.135)	(0.135)	(0.135)	(0.164E)	(0.164E)	(0.164E)	(0.164E)
90		(0.135)	0.105	0.105	0.105	0.105	(0.135)	(0.135)	(0.135)	(0.135)		(0.164E)	(0.164E)	(0.164E)
96		(0.135)	0.105	0.105	0.105	0.105	(0.135)	(0.135)	(0.135)	(0.135)		(0.164E)	(0.164E)	H 0.135E
102		0.135Z	0.135	0.135	0.135	0.135	0.135	0.135	0.135	(0.164)		(0.164E)	(0.164E)	H 0.135E
108		0.135Z	0.135	0.135	0.135	0.135	0.135	0.135	0.135	(0.164)		(0.164E)	(0.164E)	H 0.164E
114		0.164Z		0.164		0.164		0.164		0.164		H 0.164E		H 0.164E
120		0.164Z		0.164		0.164		0.164		0.164		H 0.164E		H 0.164E

Notes:

Thicknesses are based on longitudinal riveted seam fabrication, values in "()" can be reduced by one gage thickness if helical seam fabrication is utilized.
 A thickness preceded by an "H" indicates only helical seam fabrication is allowed.

E Elongation according to Article 542.04(e), the elongation requirement for Type 1 fill heights may be eliminated for fills above 1'-6"
 Z 1'-6" Minimum fill

TABLE IC: THICKNESS OF CORRUGATED ALUMINUM ALLOY PIPE FOR THE RESPECTIVE DIAMETER OF PIPE AND FILL HEIGHTS OVER THE TOP OF THE PIPE FOR 68 mm x 13 mm AND 75 mm x 25 mm CORRUGATIONS (Metric)														
Nominal Diameter mm	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7	
	Fill Height: 1 m and less 0.3 m min. cover		Fill Height: Greater than 1 m not exceeding 3 m		Fill Height: Greater than 3 m not exceeding 4.5 m		Fill Height: Greater than 4.5 m not exceeding 6 m		Fill Height: Greater than 6 m not exceeding 7.5 m		Fill Height: Greater than 7.5 m not exceeding 9 m		Fill Height: Greater than 9 m not exceeding 10.5 m	
	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm	68 x 13 mm	75 x 25 mm
300	(1.91)		1.52		1.52		1.52		1.52		1.52		1.52	
375	(1.91)		1.52		1.52		1.52		1.52		1.52		(1.91)	
450	(1.91)		1.52		1.52		1.52		1.52		(1.91)		H 1.52	
525	H 1.52E		1.52		1.52		1.52		(1.91)		H 1.52		H 1.52E	
600	(2.67E)		1.52		1.52		(1.91)		(2.67)		(2.67)		(2.67E)	
750	H 1.91E	H 1.52	1.91	H 1.52	1.91	H 1.52	(2.67)	H 1.52	(2.67)	H 1.52	H 1.91E	H 1.52	H 1.91E	H 1.52
900	(3.43E)	H 1.52E	1.91	H 1.52	(2.67)	H 1.52	(2.67)	H 1.52	(3.43)	H 1.52	H 1.91E	H 1.52	H 1.91E	H 1.52E
1050	2.67E	(1.91)	2.67	1.52	2.67	1.52	2.67	1.52	2.67	(1.91)	2.67	2.67	2.67E	(2.67E)
1200	2.67E	(1.91)	2.67	1.52	2.67	1.52	2.67	(1.91)	2.67	(2.67)	2.67E	(2.67E)	2.67E	(3.43E)
1350	2.67E	(2.67)	2.67	1.52	2.67	1.52	2.67	(1.91)	2.67	(2.67)	2.67E	(2.67E)	(3.43E)	(3.43E)
1500	3.43E	(2.67)	3.43	1.52	3.43	(1.91)	3.43	(2.67)	3.43	(2.67)	3.43E	(3.43E)	(4.17E)	(3.43E)
1650	4.17E	(2.67)	4.17	1.52	4.17	(1.91)	4.17	(2.67)	4.17	(3.43)	4.17E	(3.43E)	H 4.17E	(3.43E)
1800	4.17E	(2.67)	4.17	1.52	4.17	(1.91)	4.17	(2.67)	4.17	(3.43)	H 4.17E	(3.43E)	H 4.17E	(4.17E)
1950		(3.43)		1.91		(2.67)		(2.67)		(3.43)		(3.43E)		(4.17E)
2100		(3.43)		2.67		2.67		(3.43)		(3.43)		(4.17E)		(4.17E)
2250		(3.43)		2.67		2.67		(3.43)		(3.43)		(4.17E)		(4.17E)
2400		(3.43)		2.67		2.67		(3.43)		(3.43)		(4.17E)		H 3.43E
2550		3.43Z		3.43		3.43		3.43		(4.17)		(4.17E)		H 3.43E
2700		3.43Z		3.43		3.43		3.43		(4.17)		(4.17E)		H 4.17E
2850		4.17Z		4.17		4.17		4.17		4.17		H 4.17E		H 4.17E
3000		4.17Z		4.17		4.17		4.17		4.17		H 4.17E		H 4.17E

Notes:

Thicknesses are based on longitudinal riveted seam fabrication, values in "()" can be reduced by one gage thickness if helical seam fabrication is utilized.

A thickness preceded by an "H" indicates only helical seam fabrication is allowed.

E Elongation according to Article 542.04(e), the elongation requirement for Type 1 fill heights may be eliminated for fills above 450 mm.

Z 450 mm Minimum fill

Table IIA: THICKNESS FOR CORRUGATED STEEL PIPE ARCHES AND CORRUGATED ALUMINUM ALLOY PIPE ARCHES FOR THE RESPECTIVE EQUIVALENT ROUND SIZE OF PIPE AND FILL HEIGHTS OVER THE TOP OF PIPE																
Equivalent Round Pipe Size in	Corrugated Steel & Aluminum Pipe Arch 2 2/3" x 1/2"	Corrugated Steel & Aluminum Pipe Arch 3" x 1"	Corrugated Steel Arch Pipe Arch 5" x 1"	Min. Cover	Type 1						Type 2					
					Fill Height: 3' and less						Fill Height: Greater than 3' not exceeding 10'					
					Steel			Aluminum			Steel			Aluminum		
	Span (in.)*	Rise (in.)	Span (in.)	Rise (in.)	Steel & Aluminum	2 2/3" x 1/2"	3"x1"	5" x 1"	2 2/3" x 1/2"	3"x1"	5" x 1"	2 2/3" x 1/2"	3"x1"	5" x 1"	2 2/3" x 1/2"	3"x1"
15	17	13			1'-6"	0.064			0.060			0.064			0.064	
18	21	15			1'-6"	0.064			0.060			0.064			0.064	
21	24	18			1'-6"	0.064			(0.075)			0.064			0.064	
24	28	20			1'-6"	(0.079)			(0.105)			0.064			0.064	
30	35	24			1'-6"	(0.079)			(0.105)			0.064			(0.079)	
36	42	29			1'-6"	(0.079)			0.105			0.064			0.064	
42	49	33			1'-6"	0.109			0.105			(0.109)			(0.109)	
48	57	38			1'-6"	0.109			0.135			0.109			0.109	
54	64	43			1'-6"	0.109			0.164			0.109			0.109	
60	71	47	66	51	66	51	0.138	(0.109)	0.109	0.164	(0.075)	0.138	(0.109)	0.109	0.164	(0.075)
66	77	52	73	55	73	55	0.168	(0.109)	0.109	0.075	(0.075)	0.168	(0.109)	0.109	0.168	0.075
72	83	57	81	59	81	59	0.168	(0.109)	0.109	0.105	(0.105)	0.168	(0.109)	0.109	0.168	0.105
78			87	63	87	63		0.109	0.109	0.105	(0.105)		(0.109)	0.109	0.105	0.105
84			95	67	95	67		0.109	0.109	0.105	(0.105)		(0.109)	0.109	0.105	0.105
90			103	71	103	71		0.109	0.109	0.135	(0.135)		(0.109)	0.109	0.135	0.135
96			112	75	112	75		0.109	(0.138)	0.164	(0.164)		0.109	(0.138)	0.164	0.164
102			117	79	117	79		0.109	(0.138)	0.164	(0.164)		0.109	(0.138)	0.164	0.164
108			128	83	128	83		0.138	0.138				0.138	0.138	0.138	
114			137	87	137	87		0.138	0.138				0.138	0.138	0.138	
120			142	91	142	91		0.168	0.168				0.168	0.168	0.168	

Notes:

* Aluminumized Type 2 Steel or Precoated Galvanized Steel shall be required for steel spans up to 42' according to Article 1006.01.

Thicknesses are based on longitudinal riveted seam fabrication, values in (") can be reduced by one gage thickness if helical seam fabrication is utilized.

The Type 1 corrugated steel or aluminum pipe arches shall be placed on soil having a minimum bearing capacity of 3 tons per square foot.

The Type 2 and 3 corrugated steel or aluminum pipe arches shall be placed on soil having a minimum bearing capacity of 2 tons per square foot.

This minimum bearing capacity will be determined by the Engineer in the field.

Equivalent Round Size (mm)	Corrugated Steel & Aluminum Pipe Arch 68 x 13 mm	Corrugated Steel & Aluminum Pipe Arch 75 x 25 mm	Corrugated Steel Pipe Arch 125 x 25 mm	Min. Cover	Type 1						Type 2						Type 3						
					Fill Height:						Fill Height:						Fill Height:						
					Steel			Aluminum			Steel			Aluminum			Steel			Aluminum			
	Span (mm)	Rise (mm)	Span (mm)	Rise (mm)	Steel & Aluminum	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm	68 x 13 mm	75 x 25 mm	125 x 25 mm
375	430	330			0.5 m	1.63			1.52			1.63			1.63			1.63			1.63		
450	530	380			0.5 m	1.63			1.52			1.63			1.63			1.63			1.63		
525	610	460			0.5 m	1.63			(1.91)			1.63			1.63			1.63			1.63		
600	710	510			0.5 m	(2.01)			(2.67)			1.63			1.63			1.63			1.63		
750	870	630			0.5 m	(2.01)			(2.67)			1.63			1.63			(2.01)			(2.67)		
900	1060	740			0.5 m	(2.01)			2.67			1.63			1.63			1.63			1.63		
1050	1240	840			0.5 m	2.77			2.67			(2.77)			(2.77)			(2.77)			(2.77)		
1200	1440	970	1340	1050	0.5 m	2.77	(2.77)		3.43	1.52		2.77	2.01		2.77	2.01		1.52	1.52		2.77	2.01	
1350	1620	1100	1520	1170	0.5 m	2.77	(2.77)	2.77	4.17	(1.91)		2.77	2.01	2.01	2.77	2.01	2.01	1.52	1.52		2.77	2.77	2.77
1500	1800	1200	1670	1300	0.5 m	3.51	(2.77)	2.77	4.17	(1.91)		3.51	2.01	(2.77)	3.51	2.01	(2.77)	1.52	1.52		3.51	2.77	2.77
1650	1950	1320	1850	1400	0.5 m	4.27	(2.77)	2.77		1.91		4.27	2.01	(2.77)	4.27	2.01	(2.77)	1.91	1.91		4.27	2.77	2.77
1800	2100	1450	2050	1500	0.5 m	4.27	(2.77)	2.77		2.67		4.27	2.01	(2.77)	2.67	2.01	(2.77)	2.67	2.67		2.67	2.77	2.77
1950			2200	1620	0.5 m		2.77	2.77		2.67			(2.77)	2.77	2.67		2.77	2.67	2.67		2.77	2.77	2.77
2100			2400	1720	0.5 m		2.77	2.77		2.67			(2.77)	2.77	2.67		2.77	2.67	2.67		2.77	2.77	2.77
2250			2600	1820	0.5 m		2.77	2.77		3.43			2.77	2.77	3.43		2.77	3.43	3.43		2.77	2.77	2.77
2400			2840	1920	0.5 m		2.77	(3.51)		4.17			2.77	2.77	4.17		2.77	4.17	4.17		2.77	(3.51)	
2550			2970	2020	0.5 m		2.77	(3.51)		4.17			2.77	2.77	4.17		2.77	4.17	4.17		2.77	(3.51)	
2700			3240	2120	0.5 m		3.51	3.51					3.51	3.51			3.51	3.51			3.51	3.51	
2850			3470	2220	0.5 m		3.51	3.51					3.51	3.51			3.51	3.51			3.51	3.51	
3000			3600	2320	0.5 m		4.27	4.27					4.27	4.27			4.27	4.27			4.27	4.27	

Notes:

* Aluminized Type 2 Steel or Precoated Galvanized Steel shall be required for steel spans up to 1060 mm according to Article 1006.01. Thicknesses are based on longitudinal riveted seam fabrication, values in "()" can be reduced by one gage thickness if helical seam fabrication is utilized. The Type 1 corrugated steel or aluminum pipe arches shall be placed on soil having a minimum bearing capacity of 290 kN per square meter. The Type 2 and 3 corrugated steel or aluminum pipe arches shall be placed on soil having a minimum bearing capacity of 192 kN per square meter. This minimum bearing capacity will be determined by the Engineer in the field.

Table IIB: CLASSES OF REINFORCED CONCRETE ELLIPTICAL AND REINFORCED CONCRETE ARCH PIPE FOR THE RESPECTIVE EQUIVALENT ROUND SIZE OF PIPE AND FILL HEIGHTS OVER THE TOP OF PIPE												
Equivalent Round Size (in.)	Reinforced Concrete Elliptical pipe (in.)		Reinforced Concrete Arch pipe (in.)		Minimum Cover	Type 1		Type 2		Type 3		
						Fill Height: 3' and less		Fill Height: Greater than 3' not exceeding 10'		Fill Height: Greater than 10' not exceeding 15'		
		Span	Rise	Span	Rise	RCCP HE & A	HE	Arch	HE	Arch	HE	Arch
15	23	14	18	11	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
18	23	14	22	13 1/2	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
21	30	19	26	15 1/2	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
24	30	19	28 1/2	18	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
27	34	22	36 1/4	22 1/2	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
30	38	24	36 1/4	22 1/2	1'-0"	HE-III	A-III	HE-III	A-III	HE-IV	A-IV	
36	45	29	43 3/4	26 5/8	1'-0"	HE-II	A-II	HE-III	A-III	HE-IV	A-IV	
42	53	34	51 1/8	31 5/16	1'-0"	HE-I	A-II	HE-III	A-III	HE-IV	A-IV	
48	60	38	58 1/2	36	1'-0"	HE-I	A-II	HE-III	A-III	1460	1450	
54	68	43	65	40	1'-0"	HE-I	A-II	HE-III	A-III	1460	1460	
60	76	48	73	45	1'-0"	HE-I	A-II	HE-III	A-III	1460	1470	
66	83	53	88	54	1'-0"	HE-I	A-II	HE-III	A-III	1470	1480	
72	91	58	88	54	1'-0"	HE-I	A-II	HE-III	A-III	1470	1480	

Notes:

A number indicates the D-Load for the diameter and depth of fill and that a special design is required.

Design assumptions: Water filled pipe, AASHTO Type 2 installation per AASHTO LRFD Table 12.10.2.1-1

Table IIB: CLASSES OF REINFORCED CONCRETE ELLIPTICAL AND REINFORCED CONCRETE ARCH PIPE FOR THE RESPECTIVE EQUIVALENT ROUND SIZE OF PIPE AND FILL HEIGHTS OVER THE TOP OF PIPE (Metric)											
Equivalent Round Size (mm)	Reinforced Concrete Elliptical pipe (mm)		Reinforced Concrete Arch pipe (mm)		Minimum Cover	Type 1		Type 2		Type 3	
	Span	Rise	Span	Rise		Fill Height: 1 m and less		Fill Height: Greater than 1 m not exceeding 3 m		Fill Height: Greater than 3 m not exceeding 4.5 m	
						HE	Arch	HE	Arch	HE	Arch
375	584	356	457	279	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
450	584	356	559	343	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
525	762	483	660	394	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
600	762	483	724	457	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
686	864	559	921	572	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
750	965	610	921	572	0.3 m	HE-III	A-III	HE-III	A-III	HE-IV	A-IV
900	1143	737	1111	676	0.3 m	HE-II	A-II	HE-III	A-III	HE-IV	A-IV
1050	1346	864	1299	795	0.3 m	HE-I	A-II	HE-III	A-III	HE-IV	A-IV
1200	1524	965	1486	914	0.3 m	HE-I	A-II	HE-III	A-III	70	70
1350	1727	1092	1651	1016	0.3 m	HE-I	A-II	HE-III	A-III	70	70
1500	1930	1219	1854	1143	0.3 m	HE-I	A-II	HE-III	A-III	70	70
1676	2108	1346	2235	1372	0.3 m	HE-I	A-II	HE-III	A-III	70	70
1800	2311	1473	2235	1372	0.3 m	HE-I	A-II	HE-III	A-III	70	70

Notes:

A number indicates the D-Load for the diameter and depth of fill and that a special design is required.

Design assumptions: Water filled pipe, AASHTO Type 2 installation per AASHTO LRFD Table 12.10.2.1-1

**TABLE IIIA: PLASTIC PIPE PERMITTED
FOR A GIVEN PIPE DIAMETER AND FILL HEIGHT OVER THE TOP OF THE PIPE**

Nominal Diameter (in.)	Type 1					Type 2					Type 3					Type 4			
	Fill Height: 3' and less, with 1' min					Fill Height: Greater than 3', not exceeding 10'					Fill Height: Greater than 10', not exceeding 15'					Fill Height: Greater than 15', not exceeding 20'			
	PVC	CPVC	PE	CPE	CPP	PVC	CPVC	PE	CPE	CPP	PVC	CPVC	PE	CPE	CPP	PVC	CPVC	PE	CPP
10	X	X	X	X	NA	X	X	X	X	NA	X	X	X	X	NA	X	X	X	NA
12	X	X	X	X	X	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
15	X	X	NA	X	X	X	X	NA	X	X	X	X	X	NA	X	X	X	NA	X
18	X	X	X	X	X	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
21	X	X	NA	NA	NA	X	X	NA	NA	NA	X	X	NA	NA	NA	X	X	NA	NA
24	X	X	X	X	X	X	X	X	X	X	X	X	NA	NA	NA	X	X	X	NA
30	X	X	X	X	X	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
36	X	X	X	X	X	X	X	X	X	X	X	X	X	NA	NA	X	X	X	NA
42	X	NA	X	X	NA	X	NA	X	NA	NA	X	NA	X	NA	NA	X	NA	X	NA
48	X	NA	X	X	X	X	NA	X	NA	NA	X	NA	X	NA	NA	X	NA	X	NA

Notes:

- PVC Polyvinyl Chloride (PVC) pipe with a smooth interior
- CPVC Corrugated Polyvinyl Chloride (CPVC) pipe with a smooth interior
- PE Polyethylene (PE) pipe with a smooth interior
- CPE Corrugated Polyethylene (PE) pipe with a smooth interior
- CPP Corrugated Polypropylene (CPP) pipe with a smooth interior
- X This material may be used for the given pipe diameter and fill height
- NA Not Available

TABLE IIIA: PLASTIC PIPE PERMITTED FOR A GIVEN PIPE DIAMETER AND FILL HEIGHT OVER THE TOP OF THE PIPE (Metric)														
Nominal Diameter (mm)	Type 1 Fill Height: 1 m and less, with 0.3 m min. cover				Type 2 Fill Height: Greater than 1 m, not exceeding 3 m				Type 3 Fill Height: Greater than 3 m, not exceeding 4.5 m				Type 4 Fill Height: Greater than 4.5 m, not exceeding 6 m	
	PVC	CPVC	PE	CPE	CPP	PVC	CPVC	PE	CPE	CPP	PVC	CPVC	PE	CPP
	250	X	X	X	NA	X	X	X	X	NA	X	X	X	NA
300	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
375	X	X	NA	X	X	X	X	NA	X	X	X	X	NA	X
450	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
525	X	X	NA	NA	NA	X	X	NA	NA	NA	X	X	NA	NA
600	X	X	X	X	X	X	X	X	NA	NA	X	X	X	NA
750	X	X	X	X	X	X	X	X	NA	X	X	X	X	NA
900	X	X	X	X	X	X	X	X	NA	NA	X	X	X	NA
1000	X	NA	X	X	NA	X	NA	X	NA	NA	X	NA	X	NA
1200	X	NA	X	X	X	X	NA	X	NA	NA	X	NA	X	NA

Notes:

- PVC Polyvinyl Chloride (PVC) pipe with a smooth interior
- CPVC Corrugated Polyvinyl Chloride (CPVC) pipe with a smooth interior
- PE Polyethylene (PE) pipe with a smooth interior
- CPE Corrugated Polyethylene (PE) pipe with a smooth interior
- CPP Corrugated Polypropylene (CPP) pipe with a smooth interior
- X This material may be used for the given pipe diameter and fill height
- NA Not Available

TABLE IIIB: PLASTIC PIPE PERMITTED FOR A GIVEN PIPE DIAMETER AND FILL HEIGHT OVER THE TOP OF THE PIPE							
Nominal Diameter (in.)	Type 5			Type 6			Type 7
	Fill Height: Greater than 20', not exceeding 25'			Fill Height: Greater than 25', not exceeding 30'			Fill Height: Greater than 30', not exceeding 35'
	PVC	CPVC		PVC	CPVC		
10	X	X		X	X		X
12	X	X		X	X		X
15	X	X		X	X		X
18	X	X		X	X		X
21	X	X		X	X		X
24	X	X		X	X		X
30	X	X		X	X		X
36	X	X		X	X		X
42	X	NA		X	NA		NA
48	X	NA		X	NA		NA

Notes:

PVC Polyvinyl Chloride (PVC) pipe with a smooth interior

CPVC Corrugated Polyvinyl Chloride (CPVC) pipe with a smooth interior

X This material may be used for the given pipe diameter and fill height

NA Not Available

TABLE IIIB: PLASTIC PIPE PERMITTED FOR A GIVEN PIPE DIAMETER AND FILL HEIGHT OVER THE TOP OF THE PIPE (metric)						
Nominal Diameter (mm)	Type 5		Type 6		Type 7	
	Fill Height: Greater than 6 m, not exceeding 7.5 m		Fill Height: Greater than 7.5 m, not exceeding 9 m		Fill Height: Greater than 9 m, not exceeding 10.5 m	
	PVC	CPVC	PVC	CPVC	CPVC	
250	X	X	X	X	X	
300	X	X	X	X	X	
375	X	X	X	X	X	
450	X	X	X	X	X	
525	X	X	X	X	X	
600	X	X	X	X	X	
750	X	X	X	X	X	
900	X	X	X	X	X	
1000	X	NA	X	NA	NA	
1200	X	NA	X	NA	NA	

Notes:

- PVC Polyvinyl Chloride (PVC) pipe with a smooth interior
- CPVC Corrugated Polyvinyl Chloride (CPVC) pipe with a smooth interior
- PE Polyethylene (PE) pipe with a smooth interior
- X This material may be used for the given pipe diameter and fill height
- NA Not Available

Revise the first sentence of the first paragraph of Article 542.04(c) of the Standard Specifications to read:

“Compacted aggregate, at least 4 in. (100 mm) in depth below the pipe culvert, shall be placed the entire width of the trench and for the length of the pipe culvert, except compacted impervious material shall be used for the outer 3 ft (1 m) at each end of the pipe culvert.”

Revise the seventh paragraph of Article 542.04(d) of the Standard Specifications to read:

“PVC, PE and CPP pipes shall be joined according to the manufacturer’s specifications.”

Replace the third sentence of the first paragraph of Article 542.04(h) of the Standard Specifications with the following:

“The total cover required for various construction loadings shall be the responsibility of the Contractor.”

Delete “Table IV : Wheel Loads and Total Cover” in Article 542.04(h) of the Standard Specifications.

Revise the first and second paragraphs of Article 542.04(i) of the Standard Specifications to read:

“(i) Deflection Testing for Pipe Culverts. All PE, PVC and CPP pipe culverts shall be tested for deflection not less than 30 days after the pipe is installed and the backfill compacted. The testing shall be performed in the presence of the Engineer.

For PVC, PE, and CPP pipe culverts with diameters 24 in. (600 mm) or smaller, a mandrel drag shall be used for deflection testing. For PVC, PE, and CPP pipe culverts with diameters over 24 in. (600 mm), deflection measurements other than by a mandrel shall be used.”

Revise Articles 542.04(i)(1) and (2) of the Standard Specifications to read:

“(1) For all PVC pipe: as defined using ASTM D 3034 methodology.

(2) For all PE and CPP pipe: the average inside diameter based on the minimum and maximum tolerances specified in the corresponding ASTM or AASHTO material specifications.”

Revise the second sentence of the second paragraph of Article 542.07 of the Standard Specifications to read:

“When a prefabricated end section is used, it shall be of the same material as the pipe culvert, except for polyethylene (PE), polyvinylchloride (PVC), and polypropylene (PP) pipes which shall have metal end sections.”

Revise the first paragraph of Article 1040.03 of the Standard Specifications to read:

“1040.03 Polyvinyl Chloride (PVC) Pipe. Acceptance testing of PVC pipe and fittings shall be accomplished during the same construction season in which they are installed. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written certification that the material meets those properties. The pipe shall meet the following additional requirements.”

Delete Articles 1040.03(e) and (f) of the Standard Specifications.

Revise Articles 1040.04(c) and (d) of the Standard Specifications to read:

“(c) PE Profile Wall Pipe for Insertion Lining. The pipe shall be according to ASTM F 894. When used for insertion lining of pipe culverts, the pipe liner shall have a minimum pipe stiffness of 46 psi (317 kPa) at five percent deflection for nominal inside diameters of 42 in. (1050 mm) or less. For nominal inside diameters of greater than 42 in. (1050 mm), the pipe liner shall have a minimum pipe stiffness of 32.5 psi (225 kPa) at five percent deflection. All sizes shall have wall construction that presents essentially smooth internal and external surfaces.

(d) PE Pipe with a Smooth Interior. The pipe shall be according to ASTM F 714 (DR 32.5) with a minimum cell classification of PE 335434 as defined in ASTM D 3350. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written certification that the material meets those properties and the resin used to manufacture the pipe meets or exceeds the minimum cell classification requirements.”

Add the following to Section 1040 of the Standard Specifications:

“1040.08 Polypropylene (PP) Pipe. Storage and handling shall be according to the manufacturer's recommendations, except in no case shall the pipe be exposed to direct sunlight for more than six months. Acceptance testing of the pipe shall be accomplished during the same construction season in which it is installed. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written certification that the material meets those properties. The pipe shall meet the following additional requirements.

(a) Corrugated PP Pipe with a Smooth Interior. The pipe shall be according to AASHTO M 330 (nominal size – 12 to 60 in. (300 to 1500 mm)). The pipe shall be Type S or D.

(b) Perforated Corrugated PP Pipe with A Smooth Interior. The pipe shall be according to AASHTO M 330 (nominal size – 12 to 60 in. (300 to 1500 mm)). The pipe shall be

Type SP. In addition, the top centerline of the pipe shall be marked so that it is readily visible from the top of the trench before backfilling, and the upper ends of the slot perforations shall be a minimum of ten degrees below the horizontal.”

80324

LRFD STORM SEWER BURIAL TABLES (BDE)

Effective: November 1, 2013

Revised: April 1, 2015

Revise Article 550.02 of the Standard Specifications to read as follows:

"Item	Article Section
(a) Clay Sewer Pipe	1040.02
(b) Extra Strength Clay Pipe	1040.02
(c) Concrete Sewer, Storm Drain, and Culvert Pipe	1042
(d) Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe	1042
(e) Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe (Note 1)	1042
(f) Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe (Note 1)	1042
(g) Polyvinyl Chloride (PVC) Pipe	1040.03
(h) Corrugated Polyvinyl Chloride (PVC) Pipe with a Smooth Interior	1040.03
(i) Corrugated Polypropylene (CPP) Pipe with Smooth Interior	1040.08
(j) Rubber Gaskets and Preformed Flexible Joint Sealants for Concrete Pipe	1056
(k) Mastic Joint Sealer for Pipe	1055
(l) External Sealing Band	1057
(m) Fine Aggregate (Note 2)	1003.04
(n) Coarse Aggregate (Note 3)	1004.05
(o) Reinforcement Bars and Welded Wire Fabric	1006.10
(p) Handling Hole Plugs	1042.16
(q) Polyethylene (PE) Pipe with a Smooth Interior	1040.04
(r) Corrugated Polyethylene (PE) Pipe with a Smooth Interior	1040.04

Note 1. The class of elliptical and arch pipe used for various storm sewer sizes and heights of fill shall conform to the requirements for circular pipe.

Note 2. The fine aggregate shall be moist.

Note 3. The coarse aggregate shall be wet."

Revise the table for permitted materials in Article 550.03 of the Standard Specifications as follows:

"Class	Materials
A	Rigid Pipes: Clay Sewer Pipe Extra Strength Clay Pipe Concrete Sewer, Storm Drain, and Culvert Pipe Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe
B	Rigid Pipes: Clay Sewer Pipe Extra Strength Clay Pipe Concrete Sewer, Storm Drain, and Culvert Pipe Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe Flexible Pipes: Polyvinyl Chloride (PVC) Pipe Corrugated Polyvinyl Chloride Pipe (PVC) with a Smooth Interior Polyethylene (PE) Pipe with a Smooth Interior Corrugated Polyethylene (PE) Pipe with a Smooth Interior Corrugated Polypropylene (CPP) Pipe with a Smooth Interior"

Replace the storm sewers tables in Article 550.03 of the Standard Specifications with the following:

STORM SEWERS																
KIND OF MATERIAL PERMITTED AND STRENGTH REQUIRED FOR A GIVEN PIPE DIAMETERS AND FILL HEIGHTS OVER THE TOP OF THE PIPE																
Nominal Diameter in.	Type 3								Type 4							
	Fill Height: Greater than 10' not exceeding 15'								Fill Height: Greater than 15' not exceeding 20'							
	RCCP	CSP	ESCP	PVC	CPVC	PE	CPE	CPP	RCCP	CSP	ESCP	PVC	CPVC	PE	CPP	
10	NA	2	X	X	X	X	X	NA	NA	3	X	X	X	X	NA	
12	III	2	X	X	X	X	NA	X	IV	NA	NA	X	X	X	NA	
15	III	3	X	X	X	NA	NA	X	IV	NA	NA	X	X	NA	X	
18	III	NA	X	X	X	X	NA	X	IV	NA	NA	X	X	X	NA	
21	III	NA	NA	X	X	NA	NA	NA	IV	NA	NA	X	X	NA	NA	
24	III	NA	NA	X	X	X	NA	NA	IV	NA	NA	X	X	X	NA	
27	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
30	III	NA	NA	NA	X	X	NA	X	IV	NA	NA	X	X	X	NA	
33	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
36	III	NA	NA	NA	X	X	NA	NA	IV	NA	NA	X	X	X	NA	
42	III	NA	NA	NA	NA	X	NA	NA	IV	NA	NA	X	NA	X	NA	
48	III	NA	NA	NA	X	X	NA	NA	IV	NA	NA	X	NA	X	NA	
54	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
60	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
66	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
72	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
78	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
84	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA	
90	III	NA	NA	NA	NA	NA	NA	NA	1680	NA	NA	NA	NA	NA	NA	
96	III	NA	NA	NA	NA	NA	NA	NA	1690	NA	NA	NA	NA	NA	NA	
102	III	NA	NA	NA	NA	NA	NA	NA	1700	NA	NA	NA	NA	NA	NA	
108	1360	NA	NA	NA	NA	NA	NA	NA	1710	NA	NA	NA	NA	NA	NA	

RCCP Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe

CSP Concrete Sewer, Storm drain, and Culvert Pipe

PVC Polyvinyl Chloride Pipe

CPVC Corrugated Polyvinyl Chloride Pipe

ESCP Extra Strength Clay Pipe

PE Polyethylene Pipe with a Smooth Interior

CPE Corrugated Polyethylene Pipe with a Smooth Interior

CPP Corrugated Polypropylene pipe with a Smooth Interior

X This material may be used for the given pipe diameter and fill height.

NA This material is Not Acceptable for the given pipe diameter and fill height.

* May also use Standard Strength Clay Pipe

Note RCCP with a number instead of a Roman numeral shall be furnished according to AASHTO M170 Section 6. This number represents the D-load to produce a 0.01 in crack.

STORM SEWERS (metric)															
KIND OF MATERIAL PERMITTED AND STRENGTH REQUIRED FOR A GIVEN PIPE DIAMETERS AND FILL HEIGHTS OVER THE TOP OF THE PIPE															
Nominal Diameter in.	Type 3							Type 4							
	Fill Height: Greater than 3 m not exceeding 4.5 m							Fill Height: Greater than 4.5 m not exceeding 6 m							
	RCCP	CSP	ESCP	PVC	CPVC	PE	CPE	CPP	RCCP	CSP	ESCP	PVC	CPVC	PE	CPP
250	NA	2	X	X	X	X	X	NA	NA	3	X	X	X	X	NA
300	III	2	X	X	X	X	NA	X	IV	NA	NA	X	X	X	NA
375	III	3	X	X	X	NA	NA	X	IV	NA	NA	X	X	NA	X
450	III	NA	X	X	X	X	NA	X	IV	NA	NA	X	X	X	NA
525	III	NA	X	X	X	NA	NA	X	IV	NA	NA	X	X	NA	NA
600	III	NA	NA	NA	X	X	NA	NA	IV	NA	NA	X	X	X	NA
675	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
750	III	NA	NA	NA	X	X	NA	X	IV	NA	NA	X	X	X	NA
825	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
900	III	NA	NA	NA	X	X	NA	NA	IV	NA	NA	X	X	X	NA
1050	III	NA	NA	NA	X	X	NA	NA	IV	NA	NA	X	NA	X	NA
1200	III	NA	NA	NA	X	NA	X	NA	IV	NA	NA	X	NA	X	NA
1350	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
1500	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
1650	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
1800	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
1950	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
2100	III	NA	NA	NA	NA	NA	NA	NA	IV	NA	NA	NA	NA	NA	NA
2250	III	NA	NA	NA	NA	NA	NA	NA	80	NA	NA	NA	NA	NA	NA
2400	III	NA	NA	NA	NA	NA	NA	NA	80	NA	NA	NA	NA	NA	NA
2550	III	NA	NA	NA	NA	NA	NA	NA	80	NA	NA	NA	NA	NA	NA
2700	70	NA	NA	NA	NA	NA	NA	NA	80	NA	NA	NA	NA	NA	NA
RCCP	Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe														
CSP	Concrete Sewer, Storm drain, and Culvert Pipe														
PVC	Polyvinyl Chloride Pipe														
CPVC	Corrugated Polyvinyl Chloride Pipe														
ESCP	Extra Strength Clay Pipe														
PE	Polyethylene Pipe with a Smooth Interior														
CPE	Corrugated Polyethylene Pipe with a Smooth Interior														
CPP	Corrugated Polypropylene pipe with a Smooth Interior														
X	This material may be used for the given pipe diameter and fill height.														
NA	This material is Not Acceptable for the given pipe diameter and fill height.														
*	May also use Standard Strength Clay Pipe														
Note	RCCP with a number instead of a Roman numeral shall be furnished according to AASHTO M170 Section 6. This number represents the metric D-load to a 25.4 micro-meter crack.														

STORM SEWERS KIND OF MATERIAL PERMITTED AND STRENGTH REQUIRED FOR A GIVEN PIPE DIAMETERS AND FILL HEIGHTS OVER THE TOP OF THE PIPE									
Nominal Diameter in.	Type 5			Type 6			Type 7		
	Fill Height: Greater than 20' not exceeding 25'			Fill Height: Greater than 25' not exceeding 30'			Fill Height: Greater than 30' not exceeding 35'		
	RCCP	PVC	CPVC	RCCP	PVC	CPVC	RCCP	RCCP	CPVC
10	NA	X	X	NA	X	X	NA	NA	X
12	IV	X	X	V	X	X	V	V	X
15	IV	X	X	V	X	X	V	V	X
18	IV	X	X	V	X	X	V	V	X
21	IV	X	X	V	X	X	V	V	X
24	IV	X	X	V	X	X	V	V	X
27	IV	NA	NA	V	NA	NA	V	V	NA
30	IV	X	X	V	X	X	V	V	X
33	IV	NA	NA	V	NA	NA	V	V	NA
36	IV	X	X	V	X	X	V	V	X
42	IV	X	NA	V	X	NA	V	V	NA
48	IV	X	NA	V	X	NA	V	V	NA
54	IV	NA	NA	V	NA	NA	V	V	NA
60	IV	NA	NA	V	NA	NA	V	V	NA
66	IV	NA	NA	V	NA	NA	V	V	NA
72	V	NA	NA	V	NA	NA	V	V	NA
78	2020	NA	NA	2370	NA	NA	2730	2730	NA
84	2020	NA	NA	2380	NA	NA	2740	2740	NA
90	2030	NA	NA	2390	NA	NA	2750	2750	NA
96	2040	NA	NA	2400	NA	NA	2750	2750	NA
102	2050	NA	NA	2410	NA	NA	2760	2760	NA
108	2060	NA	NA	2410	NA	NA	2770	2770	NA

RCCP Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe

PVC Polyvinyl Chloride Pipe

CPVC Corrugated Polyvinyl Chloride Pipe

ESCP Extra Strength Clay Pipe

X This material may be used for the given pipe diameter and fill height.

NA This material is Not Acceptable for the given pipe diameter and fill height.

Note RCCP with a number instead of a Roman numeral shall be furnished according to AASHTO M170 Section 6. This number represents the D-load to produce a 0.01 in crack.

Revise the sixth paragraph of Article 550.06 of the Standard Specifications to read:

“PVC, PE and CPP pipes shall be joined according to the manufacturer’s specifications.”

Revise the first and second paragraphs of Article 550.08 of the Standard Specifications to read:

“550.08 Deflection Testing for Storm Sewers. All PVC, PE, and CPP storm sewers shall be tested for deflection not less than 30 days after the pipe is installed and the backfill compacted. The testing shall be performed in the presence of the Engineer.

For PVC, PE, and CPP storm sewers with diameters 24 in. (600 mm) or smaller, a mandrel drag shall be used for deflection testing. For PVC, PE, and CPP storm sewers with diameters over 24 in. (600 mm), deflection measurements other than by a mandrel shall be used.”

Revise the fifth paragraph of Article 550.08 to read as follows.

“The outside diameter of the mandrel shall be 95 percent of the base inside diameter. For all PVC pipe the base inside diameter shall be defined using ASTM D 3034 methodology. For all PE and CPP pipe, the base inside diameter shall be defined as the average inside diameter based on the minimum and maximum tolerances specified in the corresponding ASTM or AASHTO material specifications.”

Revise the first paragraph of Article 1040.03 of the Standard Specifications to read:

“1040.03 Polyvinyl Chloride (PVC) Pipe. Acceptance testing of PVC pipe and fittings shall be accomplished during the same construction season in which they are installed. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written certification that the material meets those properties. The pipe shall meet the following additional requirements.”

Delete Articles 1040.03(e) and (f) of the Standard Specifications.

Revise Articles 1040.04(c) and (d) of the Standard Specifications to read:

“(c) PE Profile Wall Pipe for Insertion Lining. The pipe shall be according to ASTM F 894. When used for insertion lining of pipe culverts, the pipe liner shall have a minimum pipe stiffness of 46 psi (317 kPa) at five percent deflection for nominal inside diameters of 42 in. (1050 mm) or less. For nominal inside diameters of greater than 42 in. (1050 mm), the pipe liner shall have a minimum pipe stiffness of 32.5 psi (225 kPa) at five percent deflection. All sizes shall have wall construction that presents essentially smooth internal and external surfaces.

(d) PE Pipe with a Smooth Interior. The pipe shall be according to ASTM F 714 (DR 32.5) with a minimum cell classification of PE 335434 as defined in ASTM D 3350. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written

certification that the material meets those properties and the resin used to manufacture the pipe meets or exceeds the minimum cell classification requirements.”

Add the following to Section 1040 of the Standard Specifications:

“1040.08 Polypropylene (PP) Pipe. Storage and handling shall be according to the manufacturer's recommendations, except in no case shall the pipe be exposed to direct sunlight for more than six months. Acceptance testing of the pipe shall be accomplished during the same construction season in which it is installed. The section properties shall be according to the manufacturer pre-submitted geometric properties on file with the Department. The manufacturer shall submit written certification that the material meets those properties. The pipe shall meet the following additional requirements.

- (a) Corrugated PP Pipe with a Smooth Interior. The pipe shall be according to AAHSTO M 330 (nominal size – 12 to 60 in. (300 to 1500 mm)). The pipe shall be Type S or D.
- (b) Perforated Corrugated PP Pipe with A Smooth Interior. The pipe shall be according to AASHTO M 330 (nominal size – 12 to 60 in. (300 to 1500 mm)). The pipe shall be Type SP. In addition, the top centerline of the pipe shall be marked so that it is readily visible from the top of the trench before backfilling, and the upper ends of the slot perforations shall be a minimum of ten degrees below the horizontal.”

MECHANICAL SIDE TIE BAR INSERTER (BDE)

Effective: August 1, 2014

Revised: January 1, 2015

Add the following to Article 420.03 of the Standard Specifications:

“(k) Mechanical Side Tie Bar Inserters 1103.18”

Revise Article 420.05(b) of the Standard Specifications to read:

“(b) Longitudinal Construction Joint. The tie bars shall be installed using one of the following methods.

- (1) Preformed or Drilled Holes. The tie bars shall be installed with an approved nonshrink grout or chemical adhesive providing a minimum pull-out strength as follows.

Bar Size	Minimum Pull-Out Strength
No. 6 (No. 19)	11,000 lb (49 kN)
No. 8 (No. 25)	19,750 lb (88 kN)

Holes shall be blown clean and dry prior to placing the grout or adhesive. If compressed air is used, the pneumatic tool lubricator shall be bypassed and a filter installed on the discharge valve to keep water and oil out of the lines. The installation shall be with methods and tools conforming to the grout or adhesive manufacturer's recommendations.

The Contractor shall load test five percent of the first 500 tie bars installed. No further installation will be allowed until the initial five percent testing has been completed and approval to continue installation has been given by the Engineer. Testing will be required for 0.5 percent of the bars installed after the initial 500. For each bar that fails to pass the minimum requirements, two more bars selected by the Engineer shall be tested. Each bar that fails to meet the minimum load requirement shall be reinstalled and retested. The equipment and method used for testing shall meet the requirements of ASTM E 488. All tests shall be performed within 72 hours of installation. The tie bars shall be installed and approved before concrete is placed in the adjacent lane.”

- (2) Inserted. The tie bars shall be installed with the use of a mechanical side tie bar inserter. The inserter shall insert the tie bars with vibration while still within the extrusion process, after the concrete has been struck off and consolidated without deformation of the slab. The inserter shall remain stationary relative to the pavement when inserting tie bars, while the formless paver continues to move in the direction of paving.

A void greater than 1/8 in. (3 mm) at any location around the tie bar shall require immediate adjustment of the paving operation. A void greater than 1/2 in. (13 mm) shall be repaired with a nonshrink grout or chemical adhesive after the concrete has hardened. If at the end of the day of paving more than 20 percent of the tie bars show a void larger than 1/8 in. (3 mm) at any point around the bar, the use of the side tie bar inserter shall be discontinued.

(3) Formed in Place. The tie bar shall be formed in place as shown on the plans.

The sealant reservoir shall be formed either by sawing after the concrete has set according to Article 420.05(a) or by hand tools when the concrete is in a plastic state."

Add the following to Section 1103 of the Standard Specifications:

"1103.18 Mechanical Side Bar Inserters. The mechanical side tie bar inserter shall be self-contained and supported on the formless paver with the ability to move independently from the formless paver. The insertion apparatus shall vibrate within a frequency of 2000 to 6000 vpm. A vibrating reed tachometer, hand type, shall be provided according to Article 1103.12."

80342

PAVEMENT PATCHING (BDE)

Effective: January 1, 2010

Revise the first sentence of the second paragraph of Article 701.17(e)(1) of the Standard Specifications to read:

“In addition to the traffic control and protection shown elsewhere in the contract for pavement, two devices shall be placed immediately in front of each open patch, open hole, and broken pavement where temporary concrete barriers are not used to separate traffic from the work area.”

80254

PAVEMENT STRIPING - SYMBOLS (BDE)

Effective: January 1, 2015

Revise the Symbol Table of Article 780.14 of the Supplemental Specifications to read:

“SYMBOLS

Symbol	Large Size sq ft (sq m)	Small Size sq ft (sq m)
Through Arrow	11.5 (1.07)	6.5 (0.60)
Left or Right Arrow	15.6 (1.47)	8.8 (0.82)
2 Arrow Combination Left (or Right) and Through	26.0 (2.42)	14.7 (1.37)
3 Arrow Combination Left, Right, and Through	38.4 (3.56)	20.9 (1.94)
Lane Drop Arrow	41.5 (3.86)	--
Wrong Way Arrow	24.3 (2.26)	--
Railroad "R" 6 ft (1.8 m)	3.6 (0.33)	--
Railroad "X" 20 ft (6.1 m)	54.0 (5.02)	--
International Symbol of Accessibility	3.1 (0.29)	--
Bike Symbol	4.7 (0.44)	--
Shared Lane Symbol	8.0 (0.74)	--“

PRECAST CONCRETE HANDHOLE (BDE)

Effective: August 1, 2014

Revise the third paragraph of Article 814.03 of the Standard Specifications to read:

“Handholes shall be constructed as shown on the plans and shall be cast-in-place, composite concrete, or precast units. Heavy duty handholes shall be either cast-in-place or precast units.”

Add the following to Article 814.03 of the Standard Specifications:

“(c) Precast Concrete. Precast concrete handholes shall be fabricated according to Article 1042.17. Where a handhole is contiguous to a sidewalk, preformed joint filler of 1/2 inch (13 mm) thickness shall be placed between the handhole and the sidewalk.”

Add the following to Section 1042 of the Standard Specifications:

“1042.17 Precast Concrete Handholes. Precast concrete handholes shall be according to Articles 1042.03(a)(c)(d)(e).”

80343

PROGRESS PAYMENTS (BDE)

Effective: November 2, 2013

Revise Article 109.07(a) of the Standard Specifications to read:

“(a) Progress Payments. At least once each month, the Engineer will make a written estimate of the quantity of work performed in accordance with the contract, and the value thereof at the contract unit prices. The amount of the estimate approved as due for payment will be vouchered by the Department and presented to the State Comptroller for payment. No amount less than \$1000.00 will be approved for payment other than the final payment.

Progress payments may be reduced by liens filed pursuant to Section 23(c) of the Mechanics' Lien Act, 770 ILCS 60/23(c).

If a Contractor or subcontractor has defaulted on a loan issued under the Department's Disadvantaged Business Revolving Loan Program (20 ILCS 2705/2705-610), progress payments may be reduced pursuant to the terms of that loan agreement. In such cases, the amount of the estimate related to the work performed by the Contractor or subcontractor, in default of the loan agreement, will be offset, in whole or in part, and vouchered by the Department to the Working Capital Revolving Fund or designated escrow account. Payment for the work shall be considered as issued and received by the Contractor or subcontractor on the date of the offset voucher. Further, the amount of the offset voucher shall be a credit against the Department's obligation to pay the Contractor, the Contractor's obligation to pay the subcontractor, and the Contractor's or subcontractor's total loan indebtedness to the Department. The offset shall continue until such time as the entire loan indebtedness is satisfied. The Department will notify the Contractor and Fund Control Agent in a timely manner of such offset. The Contractor or subcontractor shall not be entitled to additional payment in consideration of the offset.

The failure to perform any requirement, obligation, or term of the contract by the Contractor shall be reason for withholding any progress payments until the Department determines that compliance has been achieved.”

RETROREFLECTIVE SHEETING FOR HIGHWAY SIGNS (BDE)

Effective: November 1, 2014

Revise the first sentence of the first paragraph of Article 1091.03(a)(3) of the Standard Specifications to read:

“When tested according to ASTM E 810, with averaging, the sheeting shall have a minimum coefficient of retroreflection as show in the following tables.”

Replace the Tables for Type AA sheeting, Type AP sheeting, Type AZ sheeting and Type ZZ sheeting in Article 1091.03(a)(3) with the following.

Type AA Sheeting
Minimum Coefficient of Retroreflection
Candelas/foot candle/sq ft (candelas/lux/sq m) of material

Type AA (Average of 0 and 90 degree rotation)

Observation Angle (deg.)	Entrance Angle (deg.)	White	Yellow	Red	Green	Blue	FO
0.2	-4	800	600	120	80	40	200
0.2	+30	400	300	60	35	20	100
0.5	-4	200	150	30	20	10	75
0.5	+30	100	75	15	10	5	35

Type AA (45 degree rotation)

Observation Angle (deg.)	Entrance Angle (deg.)	Yellow	FO
0.2	-4	500	165
0.2	+30	115	40
0.5	-4	140	65
0.5	+30	60	30

Type AP Sheeting
Minimum Coefficient of Retroreflection
Candelas/foot candle/sq ft (candelas/lux/sq m) of material

Type AP (Average of 0 and 90 degree rotation)

Observation Angle (deg.)	Entrance Angle (deg.)	White	Yellow	Red	Green	Blue	Brown	FO
0.2	-4	500	380	75	55	35	25	150
0.2	+30	180	135	30	20	15	10	55
0.5	-4	300	225	50	30	20	15	90
0.5	+30	90	70	15	10	7.5	5	30

Type AZ Sheeting
Minimum Coefficient of Retroreflection
Candelas/foot candle/sq ft (candelas/lux/sq m) of material

Type AZ (Average of 0 and 90 degree rotation)

Observation Angle (deg.)	Entrance Angle (deg.)	White	Yellow	Red	Green	Blue	FYG	FY
0.2	-4	375	280	75	45	25	300	230
0.2	+30	235	170	40	25	15	190	150
0.5	-4	245	180	50	30	20	200	155
0.5	+30	135	100	25	15	10	100	75
1.0	-4	50	37.5	8.5	5	2	45	25
1.0	+30	22.5	20	5	3	1	25	12.5

Type ZZ Sheeting
Minimum Coefficient of Retroreflection
Candelas/foot candle/sq ft (candelas/lux/sq m) of material

Type ZZ (Average of 0 and 90 degree rotation)

Observation Angle (deg.)	Entrance Angle (deg.)	White	Yellow	Red	Green	Blue	FYG	FY	FO
0.2	-4	570	425	90	60	30	460	340	170
0.2	+30	190	140	35	20	10	150	110	65
0.5	-4	400	300	60	40	20	320	240	120
0.5	+30	130	95	20	15	7	100	80	45
1.0	-4	115	90	17	12	5	95	70	35
1.0	+30	45	35	7	5	2	35	25	15

REINFORCEMENT BARS (BDE)

Effective: November 1, 2013

Revise the first and second paragraphs of Article 508.05 of the Standard Specifications to read:

“508.05 Placing and Securing. All reinforcement bars shall be placed and tied securely at the locations and in the configuration shown on the plans prior to the placement of concrete. Manual welding of reinforcement may only be permitted on precast concrete products as indicated in the current Bureau of Materials and Physical Research Policy Memorandum “Quality Control / Quality Assurance Program for Precast Concrete Products”, and for precast prestressed concrete products as indicated in the Department’s current “Manual for Fabrication of Precast Prestressed Concrete Products”. Reinforcement bars shall not be placed by sticking or floating into place or immediately after placement of the concrete.

Bars shall be tied at all intersections, except where the center to center dimension is less than 1 ft (300 mm) in each direction, in which case alternate intersections shall be tied. Molded plastic clips may be used in lieu of wire to secure bar intersections, but shall not be permitted in horizontal bar mats subject to construction foot traffic or to secure longitudinal bar laps. Plastic clips shall adequately secure the reinforcement bars, and shall permit the concrete to flow through and fully encase the reinforcement. Plastic clips may be recycled plastic, and shall meet the approval of the Engineer. The number of ties as specified shall be doubled for lap splices at the stage construction line of concrete bridge decks when traffic is allowed on the first completed stage during the pouring of the second stage.”

Revise the fifth paragraph of Article 508.05 of the Standard Specifications to read:

“Supports for reinforcement in bridge decks shall be metal. For all other concrete construction the supports shall be metal or plastic. Metal bar supports shall be made of cold-drawn wire, or other approved material and shall be either epoxy coated, galvanized or plastic tipped. When the reinforcement bars are epoxy coated, the metal supports shall be epoxy coated. Plastic supports may be recycled plastic. Supports shall be provided in sufficient number and spaced to provide the required clearances. Supports shall adequately support the reinforcement bars, and shall permit the concrete to flow through and fully encase the reinforcement. The legs of supports shall be spaced to allow an opening that is a minimum 1.33 times the nominal maximum aggregate size used in the concrete. Nominal maximum aggregate size is defined as the largest sieve which retains any of the aggregate sample particles. All supports shall meet the approval of the Engineer.”

Revise the first sentence of the eighth paragraph of Article 508.05 of the Standard Specifications to read:

“Epoxy coated reinforcement bars shall be tied with plastic coated wire, epoxy coated wire, or molded plastic clips where allowed.”

Add the following sentence to the end of the first paragraph of Article 508.06(c) of the Standard Specifications:

“In addition, the total slip of the bars within the splice sleeve of the connector after loading in tension to 30 ksi (207 MPa) and relaxing to 3 ksi (20.7 MPa) shall not exceed 0.01 in. (254 microns).”

Revise Article 1042.03(d) of the Standard Specifications to read:

“(d) Reinforcement and Accessories: The concrete cover over all reinforcement shall be within $\pm 1/4$ in. (± 6 mm) of the specified cover.

Welded wire fabric shall be accurately bent and tied in place.

Miscellaneous accessories to be cast into the concrete or for forming holes and recesses shall be carefully located and rigidly held in place by bolts, clamps, or other effective means. If paper tubes are used for vertical dowel holes, or other vertical holes which require grouting, they shall be removed before transportation to the construction site.”

SIDEWALK, CORNER, OR CROSSWALK CLOSURE (BDE)

Effective: January 1, 2015

| Revised: April 1, 2015

Revise the first sentence of Article 1106.02(m) of the Supplemental Specifications to read:

“The top and bottom panels shall have alternating white and orange stripes sloping 45 degrees on both sides.”

80354

STEEL COST ADJUSTMENT (BDE) (RETURN FORM WITH BID)

Effective: April 2, 2004

Revised: July 1, 2015

Description. Steel cost adjustments will be made to provide additional compensation to the Contractor, or a credit to the Department, for fluctuations in steel prices when optioned by the Contractor. The bidder shall indicate on the attached form whether or not this special provision will be part of the contract and submit the completed form with his/her bid. Failure to submit the form or failure to indicate contract number, company name, and sign and date the form shall make this contract exempt of steel cost adjustments for all items of steel. Failure to indicate "Yes" for any item of work will make that item of steel exempt from steel cost adjustment.

Types of Steel Products. An adjustment will be made for fluctuations in the cost of steel used in the manufacture of the following items:

Metal Piling (excluding temporary sheet piling)
Structural Steel
Reinforcing Steel

Other steel materials such as dowel bars, tie bars, mesh reinforcement, guardrail, steel traffic signal and light poles, towers and mast arms, metal railings (excluding wire fence), and frames and grates will be subject to a steel cost adjustment when the pay items they are used in have a contract value of \$10,000 or greater.

The adjustments shall apply to the above items when they are part of the original proposed construction, or added as extra work and paid for by agreed unit prices. The adjustments shall not apply when the item is added as extra work and paid for at a lump sum price or by force account.

Documentation. Sufficient documentation shall be furnished to the Engineer to verify the following:

- (a) The dates and quantity of steel, in lb (kg), shipped from the mill to the fabricator.
- (b) The quantity of steel, in lb (kg), incorporated into the various items of work covered by this special provision. The Department reserves the right to verify submitted quantities.

Method of Adjustment. Steel cost adjustments will be computed as follows:

$$SCA = Q \times D$$

Where: SCA = steel cost adjustment, in dollars

Q = quantity of steel incorporated into the work, in lb (kg)

D = price factor, in dollars per lb (kg)

$$D = MPI_M - MPI_L$$

Where: MPI_M = The Materials Cost Index for steel as published by the Engineering News-Record for the month the steel is shipped from the mill. The indices will be converted from dollars per 100 lb to dollars per lb (kg).

MPI_L = The Materials Cost Index for steel as published by the Engineering News-Record for the month prior to the letting for work paid for at the contract price; or for the month the agreed unit price letter is submitted by the Contractor for extra work paid for by agreed unit price,. The indices will be converted from dollars per 100 lb to dollars per lb (kg).

The unit weights (masses) of steel that will be used to calculate the steel cost adjustment for the various items are shown in the attached table.

No steel cost adjustment will be made for any products manufactured from steel having a mill shipping date prior to the letting date.

If the Contractor fails to provide the required documentation, the method of adjustment will be calculated as described above; however, the MPI_M will be based on the date the steel arrives at the job site. In this case, an adjustment will only be made when there is a decrease in steel costs.

Basis of Payment. Steel cost adjustments may be positive or negative but will only be made when there is a difference between the MPI_L and MPI_M in excess of five percent, as calculated by:

$$\text{Percent Difference} = \{(MPI_L - MPI_M) \div MPI_L\} \times 100$$

Steel cost adjustments will be calculated by the Engineer and will be paid or deducted when all other contract requirements for the items of work are satisfied. Adjustments will only be made for fluctuations in the cost of the steel as described herein. No adjustment will be made for changes in the cost of manufacturing, fabrication, shipping, storage, etc.

The adjustments shall not apply during contract time subject to liquidated damages for completion of the entire contract.

Attachment

Item	Unit Mass (Weight)
Metal Piling (excluding temporary sheet piling)	
Furnishing Metal Pile Shells 12 in. (305 mm), 0.179 in. (3.80 mm) wall thickness)	23 lb/ft (34 kg/m)
Furnishing Metal Pile Shells 12 in. (305 mm), 0.250 in. (6.35 mm) wall thickness)	32 lb/ft (48 kg/m)
Furnishing Metal Pile Shells 14 in. (356 mm), 0.250 in. (6.35 mm) wall thickness)	37 lb/ft (55 kg/m)
Other piling	See plans
Structural Steel	See plans for weights (masses)
Reinforcing Steel	See plans for weights (masses)
Dowel Bars and Tie Bars	6 lb (3 kg) each
Mesh Reinforcement	63 lb/100 sq ft (310 kg/sq m)
Guardrail	
Steel Plate Beam Guardrail, Type A w/steel posts	20 lb/ft (30 kg/m)
Steel Plate Beam Guardrail, Type B w/steel posts	30 lb/ft (45 kg/m)
Steel Plate Beam Guardrail, Types A and B w/wood posts	8 lb/ft (12 kg/m)
Steel Plate Beam Guardrail, Type 2	305 lb (140 kg) each
Steel Plate Beam Guardrail, Type 6	1260 lb (570 kg) each
Traffic Barrier Terminal, Type 1 Special (Tangent)	730 lb (330 kg) each
Traffic Barrier Terminal, Type 1 Special (Flared)	410 lb (185 kg) each
Steel Traffic Signal and Light Poles, Towers and Mast Arms	
Traffic Signal Post	11 lb/ft (16 kg/m)
Light Pole, Tenon Mount and Twin Mount, 30 - 40 ft (9 – 12 m)	14 lb/ft (21 kg/m)
Light Pole, Tenon Mount and Twin Mount, 45 - 55 ft (13.5 – 16.5 m)	21 lb/ft (31 kg/m)
Light Pole w/Mast Arm, 30 - 50 ft (9 – 15.2 m)	13 lb/ft (19 kg/m)
Light Pole w/Mast Arm, 55 - 60 ft (16.5 – 18 m)	19 lb/ft (28 kg/m)
Light Tower w/Luminaire Mount, 80 - 110 ft (24 – 33.5 m)	31 lb/ft (46 kg/m)
Light Tower w/Luminaire Mount, 120 - 140 ft (36.5 – 42.5 m)	65 lb/ft (97 kg/m)
Light Tower w/Luminaire Mount, 150 - 160 ft (45.5 – 48.5 m)	80 lb/ft (119 kg/m)
Metal Railings (excluding wire fence)	
Steel Railing, Type SM	64 lb/ft (95 kg/m)
Steel Railing, Type S-1	39 lb/ft (58 kg/m)
Steel Railing, Type T-1	53 lb/ft (79 kg/m)
Steel Bridge Rail	52 lb/ft (77 kg/m)
Frames and Grates	
Frame	250 lb (115 kg)
Lids and Grates	150 lb (70 kg)

Return With Bid

**ILLINOIS DEPARTMENT
OF TRANSPORTATION**

**OPTION FOR
STEEL COST ADJUSTMENT**

The bidder shall submit this completed form with his/her bid. Failure to submit the form or properly complete contract number, company name, and sign and date the form shall make this contract exempt of steel cost adjustments for all items of steel. Failure to indicate "Yes" for any item of work will make that item of steel exempt from steel cost adjustment. After award, this form, when submitted shall become part of the contract.

Contract No.: _____

Company Name: _____

Contractor's Option:

Is your company opting to include this special provision as part of the contract plans for the following items of work?

Metal Piling	Yes	<input type="checkbox"/>
Structural Steel	Yes	<input type="checkbox"/>
Reinforcing Steel	Yes	<input type="checkbox"/>
Dowel Bars, Tie Bars and Mesh Reinforcement	Yes	<input type="checkbox"/>
Guardrail	Yes	<input type="checkbox"/>
Steel Traffic Signal and Light Poles, Towers and Mast Arms	Yes	<input type="checkbox"/>
Metal Railings (excluding wire fence)	Yes	<input type="checkbox"/>
Frames and Grates	Yes	<input type="checkbox"/>

Signature: _____ **Date:** _____

TEMPORARY CONCRETE BARRIER (BDE)

Effective: January 1, 2015

Revised: July 1, 2015

Revise Article 704.02 of the Standard Specifications to read:

“704.02 Materials. Materials shall be according to the following.

Item	Article/Section
(a) Precast Temporary Concrete Barrier	1042
(b) Reinforcement Bars	1006.10(a)
(c) Connecting Pins and Anchor Pins (Note 1)	
(d) Connecting Loop Bars (Note 2)	
(e) Packaged Rapid Hardening Mortar or Concrete	1018

Note 1. Connecting Pins and Anchor Pins shall be according to the requirements of ASTM F 1554 Grade 36 (Grade 250).

Note 2. Connecting loop bars shall be smooth bars according to the requirements of ASTM A 36 (A 36M).”

Revise Article 704.04 of the Standard Specifications to read:

“704.04 Installation. The barriers shall be seated on bare, clean pavement or paved shoulder and connected together in a smooth, continuous line at the locations provided by the Engineer.

Except on bridge decks, or where alternate anchoring details are shown on the plans, the barrier unit at each end of an installation shall be anchored to the pavement or paved shoulder using six anchor pins and protected with an impact attenuator as shown on the plans. When pinning of additional barrier units within the installation is specified, three anchor pins shall be installed in the traffic side holes of the required barriers.

Where both pinned and unpinned barrier units are used in a continuous installation, a transition shall be provided between them. The transition from pinned to unpinned barrier shall consist of two anchor pins installed in the end holes on the traffic side of the first barrier beyond the pinned section and one anchor pin installed in the middle hole on the traffic side of the second barrier beyond the pinned section. The third barrier beyond the pinned section shall then be unpinned.

Barriers located on bridge decks shall be restrained as shown on the plans. Anchor pins shall not be installed through bridge decks, unless otherwise noted.

Barriers or attachments damaged during transportation or handling, or by traffic during the life of the installation, shall be repaired or replaced. The Engineer will be the sole judge in determining which units or attachments require repair or replacement.

The barriers shall be removed when no longer required by the contract. After removal, all anchor holes in the pavement or paved shoulder shall be filled with a rapid hardening mortar or concrete. Only enough water to permit placement and consolidation by rodding shall be used and the material shall be struck-off flush.”

Add the following after the first paragraph of Article 704.05 of the Standard Specifications:

“Anchor pins, except for the six anchor pins for the barrier unit at each end of an installation, will be measured for payment as each, per anchor pin installed.”

Add the following after the second paragraph of Article 704.06 of the Standard Specifications:

“Anchor pins, except for the six anchor pins for the barrier unit at each end of an installation, will be paid for at the contract unit price per each for PINNING TEMPORARY CONCRETE BARRIER.”

80355

TRACKING THE USE OF PESTICIDES (BDE)

Effective: August 1, 2012

Add the following paragraph after the first paragraph of Article 107.23 of the Standard Specifications:

“Within 48 hours of the application of pesticides, including but not limited to herbicides, insecticides, algaecides, and fungicides, the Contractor shall complete and return to the Engineer, Operations form “OPER 2720”.”

80301

TRAINING SPECIAL PROVISIONS (BDE) This Training Special Provision supersedes Section 7b of the Special Provision entitled "Specific Equal Employment Opportunity Responsibilities," and is in implementation of 23 U.S.C. 140(a).

As part of the contractor's equal employment opportunity affirmative action program, training shall be provided as follows:

The contractor shall provide on-the-job training aimed at developing full journeyman in the type of trade or job classification involved. The number of trainees to be trained under this contract will be . In the event the contractor subcontracts a portion of the contract work, he shall determine how many, if any, of the trainees are to be trained by the subcontractor, provided however, that the contractor shall retain the primary responsibility for meeting the training requirements imposed by this special provision. The contractor shall also insure that this Training Special Provision is made applicable to such subcontract. Where feasible, 25 percent of apprentices or trainees in each occupation shall be in their first year of apprenticeship or training.

The number of trainees shall be distributed among the work classifications on the basis of the contractor's needs and the availability of journeymen in the various classifications within the reasonable area of recruitment. Prior to commencing construction, the contractor shall submit to the Illinois Department of Transportation for approval the number of trainees to be trained in each selected classification and training program to be used. Furthermore, the contractor shall specify the starting time for training in each of the classifications. The contractor will be credited for each trainee employed by him on the contract work who is currently enrolled or becomes enrolled in an approved program and will be reimbursed for such trainees as provided hereinafter.

Training and upgrading of minorities and women toward journeyman status is a primary objective of this Training Special Provision. Accordingly, the contractor shall make every effort to enroll minority trainees and women (e.g. by conducting systematic and direct recruitment through public and private sources likely to yield minority and women trainees) to the extent such persons are available within a reasonable area of recruitment. The contractor will be responsible for demonstrating the steps that he has taken in pursuance thereof, prior to a determination as to whether the contractor is in compliance with this Training Special Provision. This training commitment is not intended, and shall not be used, to discriminate against any applicant for training, whether a member of a minority group or not.

No employee shall be employed as a trainee in any classification in which he has successfully completed a training course leading to journeyman status or in which he has been employed as a journeyman. The contractor should satisfy this requirement by including appropriate questions in the employee application or by other suitable means. Regardless of the method used the contractor's records should document the findings in each case.

The minimum length and type of training for each classification will be as established in the training program selected by the contractor and approved by the Illinois Department of Transportation and the Federal Highway Administration. The Illinois Department of Transportation and the Federal Highway Administration shall approve a program, if it is reasonably calculated to meet the equal employment opportunity obligations of the contractor and to qualify the average trainee for journeyman status in the classification concerned by the end of the training period. Furthermore, apprenticeship programs registered with the U.S. Department of Labor, Bureau of Apprenticeship and Training, or with a State apprenticeship agency recognized by the Bureau and training programs approved by not necessarily sponsored by the U.S. Department of Labor, Manpower Administration, Bureau of Apprenticeship and Training shall also be considered acceptable provided it is being administered in a manner consistent with the equal employment obligations of Federal-aid highway construction contracts. Approval or acceptance of a training program shall be obtained from the State prior to commencing work on the classification covered by the program. It is the intention of these provisions that training is to be provided in the construction crafts rather than clerk-typists or secretarial-type positions. Training is permissible in lower level management positions such as office engineers, estimators, timekeepers, etc., where the training is oriented toward construction applications. Training in the laborer classification may be permitted provided that significant and meaningful training is provided and approved by the Illinois Department of Transportation and the Federal Highway Administration. Some offsite training is permissible as long as the training is an integral part of an approved training program and does not comprise a significant part of the overall training.

Except as otherwise noted below, the contractor will be reimbursed 80 cents per hour of training given an employee on this contract in accordance with an approved training program. As approved by the Engineer, reimbursement will be made for training of persons in excess of the number specified herein. This reimbursement will be made even though the contractor receives additional training program funds from other sources, provided such other source does not specifically prohibit the contractor from receiving other reimbursement. Reimbursement for offsite training indicated above may only be made to the contractor where he does one or more of the following and the trainees are concurrently employed on a Federal-aid project; contributes to the cost of the training, provides the instruction to the trainee or pays the trainee's wages during the offsite training period.

No payment shall be made to the contractor if either the failure to provide the required training, or the failure to hire the trainee as a journeyman, is caused by the contractor and evidences a lack of good faith on the part of the contractor in meeting the requirement of this Training Special Provision. It is normally expected that a trainee will begin his training on the project as soon as feasible after start of work utilizing the skill involved and remain on the project as long as training opportunities exist in his work classification or until he has completed his training program.

It is not required that all trainees be on board for the entire length of the contract. A contractor will have fulfilled his responsibilities under this Training Special Provision if he has provided acceptable training to the number of trainees specified. The number trained shall be determined on the basis of the total number enrolled on the contract for a significant period.

Trainees will be paid at least 60 percent of the appropriate minimum journeyman's rate specified in the contract for the first half of the training period, 75 percent for the third quarter of the training period, and 90 percent for the last quarter of the training period, unless apprentices or trainees in an approved existing program are enrolled as trainees on this project. In that case, the appropriate rates approved by the Departments of Labor or Transportation in connection with the existing program shall apply to all trainees being trained for the same classification who are covered by this Training Special Provision.

The contractor shall furnish the trainee a copy of the program he will follow in providing the training. The contractor shall provide each trainee with a certification showing the type and length of training satisfactorily complete.

The contractor will provide for the maintenance of records and furnish periodic reports documenting his performance under this Training Special Provision.

METHOD OF MEASUREMENT The unit of measurement is in hours.

BASIS OF PAYMENT This work will be paid for at the contract unit price of 80 cents per hour for TRAINEES. The estimated total number of hours, unit price and total price have been included in the schedule of prices.

20338

WARM MIX ASPHALT (BDE)

Effective: January 1, 2012

Revised: November 1, 2014

Description. This work shall consist of designing, producing and constructing Warm Mix Asphalt (WMA) in lieu of Hot Mix Asphalt (HMA) at the Contractor's option. Work shall be according to Sections 406, 407, 408, 1030, and 1102 of the Standard Specifications, except as modified herein. In addition, any references to HMA in the Standard Specifications, or the special provisions shall be construed to include WMA.

WMA is an asphalt mixture which can be produced at temperatures lower than allowed for HMA utilizing approved WMA technologies. WMA technologies are defined as the use of additives or processes which allow a reduction in the temperatures at which HMA mixes are produced and placed. WMA is produced by the use of additives, a water foaming process, or combination of both. Additives include minerals, chemicals or organics incorporated into the asphalt binder stream in a dedicated delivery system. The process of foaming injects water into the asphalt binder stream, just prior to incorporation of the asphalt binder with the aggregate.

Approved WMA technologies may also be used in HMA provided all the requirements specified herein, with the exception of temperature, are met. However, asphalt mixtures produced at temperatures in excess of 275 °F (135 °C) will not be considered WMA when determining the grade reduction of the virgin asphalt binder grade.

Equipment.

Revise the first paragraph of Article 1102.01 of the Standard Specifications to read:

"1102.01 Hot-Mix Asphalt Plant. The hot-mix asphalt (HMA) plant shall be the batch-type, continuous-type, or dryer drum plant. The plants shall be evaluated for prequalification rating and approval to produce HMA according to the current Bureau of Materials and Physical Research Policy Memorandum, "Approval of Hot-Mix Asphalt Plants and Equipment". Once approved, the Contractor shall notify the Bureau of Materials and Physical Research to obtain approval of all plant modifications. The plants shall not be used to produce mixtures concurrently for more than one project or for private work unless permission is granted in writing by the Engineer. The plant units shall be so designed, coordinated and operated that they will function properly and produce HMA having uniform temperatures and compositions within the tolerances specified. The plant units shall meet the following requirements."

Add the following to Article 1102.01(a) of the Standard Specifications.

"(13) Equipment for Warm Mix Technologies.

- a. Foaming. Metering equipment for foamed asphalt shall have an accuracy of ± 2 percent of the actual water metered. The foaming control system shall be electronically interfaced with the asphalt binder meter.

- b. Additives. Additives shall be introduced into the plant according to the supplier's recommendations and shall be approved by the Engineer. The system for introducing the WMA additive shall be interlocked with the aggregate feed or weigh system to maintain correct proportions for all rates of production and batch sizes."

Mix Design Verification.

Add the following to Article 1030.04 of the Standard Specifications.

"(e) Warm Mix Technologies.

- (1) Foaming. WMA mix design verification will not be required when foaming technology is used alone (without WMA additives). However, the foaming technology shall only be used on HMA designs previously approved by the Department.
- (2) Additives. WMA mix designs utilizing additives shall be submitted to the Engineer for mix design verification."

Construction Requirements.

Revise the second paragraph of Article 406.06(b)(1) of the Standard Specifications to read:

"The HMA shall be delivered at a temperature of 250 to 350 °F (120 to 175 °C).
WMA shall be delivered at a minimum temperature of 215 °F (102 °C)."

Basis of Payment.

This work will be paid at the contract unit price bid for the HMA pay items involved. Anti-strip will not be paid for separately, but shall be considered as included in the cost of the work.

WEEKLY DBE TRUCKING REPORTS (BDE)

Effective: June 2, 2012

| Revised: April 2, 2015

| The Contractor shall submit a weekly report of Disadvantaged Business Enterprise (DBE) trucks hired by the Contractor or subcontractors (i.e. not owned by the Contractor or subcontractors) that are used for DBE goal credit.

| The report shall be submitted to the Engineer on Department form "SBE 723" within ten business days following the reporting period. The reporting period shall be Monday through Sunday for each week reportable trucking activities occur.

Any costs associated with providing weekly DBE trucking reports shall be considered as included in the contract unit prices bid for the various items of work involved and no additional compensation will be allowed.

80302

DRILLED SOLDIER PILE RETAINING WALL

Effective: September 20, 2001

Revised: **January 3, 2014**

Description. This work shall consist of providing all labor, materials, and equipment necessary to fabricate and furnish the soldier piles, create and maintain the shaft excavations, set and brace the soldier piles into position and encase the soldier piles in concrete to the specified elevation. Also included in this work is the backfilling of the remainder of the shaft excavation with Controlled Low-Strength Material (CLSM), and the furnishing and installation of lagging. All work shall be according to the details shown on the plans and as directed by the Engineer.

The remainder of the retaining wall components as shown on the plans, such as concrete facing, shear studs, reinforcement bars, tie backs, hand rails, and various drainage items etc., are not included in this Special Provision but are paid for as specified elsewhere in this Contract.

Materials. The materials used for the soldier piles and lagging shall satisfy the following requirements:

- (a) The structural steel components for the soldier piles shall conform to the requirements of AASHTO M270, Grade 36 (M270M Grade 250), unless otherwise designated on the plans.
- (b) The soldier pile encasement concrete shall be Class DS according to Article 516.02.
- (c) The Controlled Low-Strength Material (CLSM), used for backfilling shaft excavations above the soldier pile encasement concrete and for backfilling secant lagging excavations, to the existing ground surface, shall be according to Section 1019.
- (d) Temporary casing shall be produced by electric seam, butt, or spiral welding to produce a smooth wall surface, fabricated from steel satisfying ASTM A252 Grade 2. The minimum wall thickness shall be as required to resist the anticipated installation and dewatering stresses, as determined by the Contractor, but in no case less than 1/4 in. (6 mm).
- (e) Drilling slurry shall consist of a polymer or mineral base material. Mineral slurry shall have both a mineral grain size that will remain in suspension with sufficient viscosity and gel characteristics to transport excavated material to a suitable screening system. The percentage and specific gravity of the material used to make the suspension shall be sufficient to maintain the stability of the excavation and to allow proper concrete placement. For polymer slurry, the calcium hardness of the mixing water shall not exceed 100 mg/L.
- (f) Timber Lagging. The minimum tabulated unit stress in bending (F_b), used for the design of the timber lagging, shall be 1000 psi (6.9 MPa) unless otherwise specified on the plans. When treated timber lagging is specified on the plans, the method of treatment shall be according to Article 1007.12. All timber shall meet the inspection requirements of Article 1007.01.

- (g) **Precast Concrete Lagging.** Precast concrete lagging shall be according to Section 504 of the Standard Specifications, except as modified herein. Unless specified otherwise, precast concrete lagging surfaces exposed to view in the completed wall shall be finished according to Article 503.15. When specified on the plans, the exposed surface shall be finished with a concrete form liner approved by the Engineer. The back face of the panel shall be roughly screeded to eliminate open pockets of aggregate and surface distortions in excess of 1/4 in. Reinforcement for precast concrete lagging shall be epoxy coated. Lifting inserts shall have a total minimum design capacity based on yield strength of 4 times the dead load calculated for the width of lagging used. Fabric bearing pads, when specified on the plans, shall meet the requirements of Section 1082. Threaded inserts, or other accessories, cast into the precast concrete lagging shall be galvanized according to AASHTO M111 or M232 as applicable.

Equipment. The drilling equipment shall have adequate capacity, including power, torque and down thrust, to create a shaft excavation of the maximum diameter specified to a depth of 20 percent beyond the depths shown on the plans. Concrete equipment shall be according to Article 1020.03.

Construction Requirements. The shaft excavation for each soldier pile shall extend to the tip elevation indicated on the plans for soldier piles terminating in soil or to the required embedment in rock when rock is indicated on the contract plans. The Contractor shall satisfy the following requirements:

- (a) **Drilling Methods.** The soldier pile installation shall be according to Articles 516.06(a),(b), or(c).

No shaft excavation shall be made adjacent to a soldier pile with encasement concrete that has a compressive strength less than 1500 psi (10.35 MPa), nor adjacent to secant lagging until the CLSM has reach sufficient strength to maintain its position and shape unless otherwise approved by the Engineer. Materials removed or generated from the shaft excavations shall be disposed of by the Contractor according to Article 202.03. Excavation by blasting will not be permitted.

- (b) **Drilling Slurry.** During construction, the level of the slurry shall be maintained at a height sufficient to prevent caving of the hole. In the event of a sudden or significant loss of slurry to the hole, the construction of that shaft shall be stopped and the shaft excavation backfilled or supported by temporary casing until a method to stop slurry loss, or an alternate construction procedure, has been developed and approved by the Engineer.

- (c) **Obstructions.** Obstructions shall be defined as any object (such as but not limited to, boulders, logs, old foundations, etc.) that cannot be removed with normal earth drilling procedures, but requires special augers, tooling, core barrels or rock augers to remove the obstruction. When obstructions are encountered, the Contractor shall notify the Engineer and upon concurrence of the Engineer, the Contractor shall begin working to core, break up, push aside, or remove the obstruction. Lost tools or equipment in the excavation, as a

result of the Contractor's operation, shall not be defined as obstructions and shall be removed at the Contractor's expense.

- (d) Top of Rock. The top of rock will be considered as the point where rock, defined as bedded deposits and conglomerate deposits exhibiting the physical characteristics and difficulty of rock removal as determined by the Engineer, is encountered which cannot be drilled with earth augers and/or underreaming tools configured to be effective in the soils indicated in the contract documents, and requires the use of special rock augers, core barrels, air tools, blasting, or other methods of hand excavation.
- (e) Design Modifications. If the top of rock elevation encountered is below that estimated on the plans, such that the soldier pile length above rock is increased by more than 10 percent, the Engineer shall be contacted to determine if any soldier pile design changes are required. In addition, if the type of soil or rock encountered is not similar to that shown in the subsurface exploration data, the Engineer shall be contacted to determine if revisions are necessary.
- (f) Soldier Pile Fabrication and Placement. The soldier pile is defined as the structural steel section(s) shown on the plans as well as any connecting plates used to join multiple sections. The types of soldier piles shall be defined as HP, W Sections, or Built-Up Sections. Cleaning and painting of all steel components, when specified, shall be as shown on the plans and accomplished according to Section 506. This work will not be paid for separately, but shall be considered included in the cost of Furnishing Soldier Piles of the type specified.

The soldier pile shall be shop fabricated such that no field welding is required. The Contractor shall attach suitable bracing or support to maintain the position of the soldier pile within the shaft excavation such that the final location will satisfy the Construction Tolerances portion of this Special Provision. The bracing or supports shall remain in place until the concrete for encasement has reached a minimum compressive strength of 1500 psi (10.35 MPa).

When embedment in rock is indicated on the plans, modification to the length of a soldier pile may be required to satisfy the required embedment. The modification shall be made to the top of the soldier pile unless otherwise approved by the Engineer. When the top of rock encountered is above the estimated elevation indicated on the plans, the soldier piles shall be cut to the required length. If the top of rock encountered is below that estimated on the plans, the Contractor shall either furnish longer soldier piles or splice on additional length of soldier pile per Article 512.05(a) to satisfy the required embedment in rock. In order to avoid delays, the Contractor may have additional soldier pile sections fabricated as necessary to make the required adjustments. Additional soldier pile quantities, above those shown on the plans, shall not be furnished without prior written approval by the Engineer.

- (g) Concrete Placement. Concrete work shall be performed according to Article 516.12 and as specified herein.

The soldier pile encasement concrete pour shall be made in a continuous manner from the bottom of the shaft excavation to the elevation indicated on the plans. Concrete shall be placed as soon as possible after the excavation is completed and the soldier pile is secured in the proper position. Uneven levels of concrete placed in front, behind, and on the sides of the soldier pile shall be minimized to avoid soldier pile movement, and to ensure complete encasement.

Following the soldier pile encasement concrete pour, the remaining portion of the shaft excavation shall be backfilled with CLSM according to Section 593. CLSM Secant lagging placement shall be placed as soon as practical after the shaft excavation is cleared.

(h) Construction Tolerances. The soldier piles shall be installed within the excavation to satisfy the following tolerances:

(1) The center of the soldier pile shall be within 2 in. (50 mm) of plan location in any direction at the top of the pile.

(2) The out of vertical plumbness of the soldier pile shall not exceed 1/8 in./ft. (10 mm/m)

(3) The top of the soldier pile shall be within ± 2 in. (± 50 mm) of the plan elevation.

(i) Timber Lagging. Timber lagging, when required by the plans, installed below the original ground surface, shall be placed from the top down as the excavation proceeds. Lagging shown above grade shall be installed and backfilled against prior to installing any permanent facing to minimize post construction deflections. Over-excavation required to place the timber lagging behind the flanges of the soldier piles shall be the minimum necessary to install the lagging. Any voids produced behind the lagging shall be filled with porous granular embankment at the Contractors expense. When the plans require the Contractor to design the timber lagging, the design shall be based on established practices published in FHWA or AASHTO documents considering lateral earth pressure, construction loading, traffic surcharges and the lagging span length(s). The nominal thickness of the lagging selected shall not be less than 3 in. (75 mm) and shall satisfy the minimum tabulated unit stress in bending (F_b) stated elsewhere in this Special Provision. The Contractor shall be responsible for the successful performance of the lagging system until the concrete facing is installed. When the nominal timber lagging thickness(s) and allowable stress are specified on the plans, the timber shall be according to Article 1007.03.

(j) Precast Concrete Lagging. Precast concrete lagging, when required by the plans, installed below the original ground surface, shall be placed from the top down as the excavation proceeds. Lagging shown above grade shall be installed and backfilled against prior to installing any permanent facing to minimize post construction deflections. Over-excavation required to place the precast lagging behind the flanges of the soldier piles shall be the minimum necessary to install the lagging. Any voids produced behind the lagging shall be filled with porous granular embankment at the Contractor's expense. When the plans require the Contractor to design the precast concrete lagging, the design shall be based on established practices published in FHWA or AASHTO documents considering lateral earth pressure, construction loading, traffic surcharges and the lagging span length(s). The

Contractor shall be responsible for the successful performance of the lagging system until the permanent concrete facing, when specified on the plans, is installed.

The precast concrete lagging shall be reinforced with a minimum of 0.31 square inches/foot (655 Sq. mm/meter) of horizontal and vertical reinforcement per unit width of lagging with a minimum thickness of 3 in. (75 mm).

When precast concrete lagging is exposed to view in the completed wall, shop drawings for the lagging shall be submitted according to Article 1042.03(b) and Article 105.04 of the Standard Specifications. The supplier selected by the Contractor shall submit complete design calculations and shop drawings, prepared and sealed by an Illinois Licensed Structural Engineer, for approval by the Engineer.

- (k) Structure Excavation. When structure excavation is necessary to place a concrete facing, it shall be made and paid for according to Section 502 except that the horizontal limits for structure excavation shall be from the face of the soldier pile to a vertical plane 2 ft. (600 mm) from the finished face of the wall. The depth shall be from the top of the original ground surface to the bottom of the concrete facing. The additional excavation necessary to place the lagging whether through soil or CLSM shall be included in this work.
- (l) Geocomposite Wall Drain. When required by the plans, the geocomposite wall drain shall be installed and paid for according to Section 591 except that, in the case where a concrete facing is specified on the plans, the wall drain shall be installed on the concrete facing side of the lagging with the pervious (fabric) side of the drain installed to face the lagging. When a concrete facing is not specified on the plans, the pervious (fabric) side of the drain shall be installed to face the soil. In this case, the drain shall be installed in stages as the lagging is installed. The wall drain shall be placed in sections and spliced, or kept on a continuous roll, so that as each piece of lagging is placed, the drain can be properly located as the excavation proceeds.

Method of Measurement. The furnishing of soldier piles will be measured for payment in feet (meters) along the centerline of the soldier pile for each of the types specified. The length shall be determined as the difference between the plan top of soldier pile and the final as built shaft excavation bottom.

The drilling and setting of soldier piles in soil and rock, will be measured for payment and the volumes computed in cubic feet (cubic meters) for the shaft excavation required to set the soldier piles according to the plans and specifications, and accepted by the Engineer. These volumes shall be the theoretical volumes computed using the diameter(s) of the shaft(s) shown in the plans and the depth of the excavation in soil and/or rock as appropriate. The depth in soil will be defined as the difference in elevation between the ground surface at the time of concrete placement and the bottom of the shaft excavation or the top of rock (when present), whichever is encountered first. The depth in rock will be defined as the difference in elevation between the measured top of rock and the bottom of the shaft excavation.

Drilling and placing CLSM secant lagging shall be measured for payment in cubic feet (cubic meters) of the shaft excavation required to install the secant lagging as shown in the plans.

This volume shall be the theoretical volume computed using the diameter(s) shown on the plans and the difference in elevation between the as built shaft excavation bottom and the ground surface at the time of the CLSM placement.

Timber and precast concrete lagging shall be measured for payment in square feet (square meters) of lagging installed to the limits as shown on the plans. The quantity shall be calculated using the minimum lagging length required on the plans multiplied by the as-installed height of lagging, for each bay of lagging spanning between the soldier piles.

Basis of Payment. The furnishing of soldier piles will be paid for at the contract unit price per foot (meter) for FURNISHING SOLDIER PILES, of the type specified, for the total number of feet (meters) furnished to the job site. The cost of any field splices required due to changes in top of rock elevation shall be paid for according to Article 109.04.

The drilling and setting of soldier piles will be paid for at the contract unit price per cubic foot (cubic meter) for DRILLING AND SETTING SOLDIER PILES (IN SOIL) and DRILLING AND SETTING SOLDIER PILES (IN ROCK). The required shaft excavation, soldier pile encasement concrete and any CLSM backfill required around each soldier pile will not be paid for separately but shall be included in this item.

Timber lagging will be paid for at the contract unit price per square foot (square meter) for UNTREATED TIMBER LAGGING, or TREATED TIMBER LAGGING as detailed on the plans. Precast concrete lagging will be paid for at the contract unit price per square foot (square meter) for PRECAST CONCRETE LAGGING as detailed on the plans.

The secant lagging will be paid for at the contract unit price per cubic foot (cubic meter) for SECANT LAGGING. The required shaft excavation and CLSM backfill required to fill that excavation shall be included in this item.

Obstruction mitigation shall be paid for according to Article 109.04.

No additional compensation, other than noted above, will be allowed for removing and disposing of excavated materials, for furnishing and placing concrete, CLSM, bracing, lining, temporary casings placed and removed or left in place, or for any excavation made or concrete placed outside of the plan diameter(s) of the shaft(s) specified.

PIPE UNDERDRAINS FOR STRUCTURES

Effective: May 17, 2000

Revised: January 22, 2010

Description. This work shall consist of furnishing and installing a pipe underdrain system as shown on the plans, as specified herein, and as directed by the Engineer.

Materials. Materials shall meet the requirements as set forth below:

The perforated pipe underdrain shall be according to Article 601.02 of the Standard Specifications. Outlet pipes or pipes connecting to a separate storm sewer system shall not be perforated.

The drainage aggregate shall be a combination of one or more of the following gradations, FA1, FA2, CA5, CA7, CA8, CA11, or CA13 thru 16, according to Sections 1003 and 1004 of the Standard Specifications.

The fabric surrounding the drainage aggregate shall be Geotechnical Fabric for French Drains according to Article 1080.05 of the Standard Specifications.

Construction Requirements. All work shall be according to the applicable requirements of Section 601 of the Standard Specifications except as modified below.

The pipe underdrains shall consist of a perforated pipe drain situated at the bottom of an area of drainage aggregate wrapped completely in geotechnical fabric and shall be installed to the lines and gradients as shown on the plans.

Method of Measurement. Pipe Underdrains for Structures shall be measured for payment in feet (meters), in place. Measurement shall be along the centerline of the pipe underdrains. All connectors, outlet pipes, elbows, and all other miscellaneous items shall be included in the measurement. Concrete headwalls shall be included in the cost of Pipe Underdrains for Structures, but shall not be included in the measurement for payment.

Basis of Payment. This work will be paid for at the contract unit price per foot (meter) for PIPE UNDERDRAINS FOR STRUCTURES of the diameter specified. Furnishing and installation of the drainage aggregate, geotechnical fabric, forming holes in structural elements and any excavation required, will not be paid for separately, but shall be included in the cost of the pipe underdrains for structures.



Route FAP 361 (Longmeadow Parkway)
Section 13-00215-10-PV
County Kane County

Marked Rte. FAP 361 (Longmeadow Parkway)
Project No. 2013-1894
Contract No. _____

This plan has been prepared to comply with the provisions of the National Pollutant Discharge Elimination System (NPDES) Permit No. ILR10 (Permit ILR10), issued by the Illinois Environmental Protection Agency (IEPA) for storm water discharges from construction site activities.

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Carl Schoedel
Print Name
Director of Transportation/County Engineer
Title
Kane County Division of Transportation
Agency

Signature

Date

I. Site Description:

- A. Provide a description of the project location (include latitude and longitude):

Longmeadow Parkway Section B is an extension of the existing roadway from approximately 1,400 feet east of Randall Road to approximately 2,300 feet east of IL Rte 31 in Kane County; Sections 8, 9, & 10, T43N, R8E; begin: 42.139655N, -88.331165W / end: 42.139695N, -88.288737W. Improvements include approximately 3,600 feet of IL Rte 31 and approximately 1,400 feet of Sleepy Hollow Road.

- B. Provide a description of the construction activity which is the subject of this plan:

Construction includes the widening and reconstruction of Longmenadow Parkway from a 2-lane section to a 4-lane lane divided by a median. The improvements include a bridge and MSE walls over IL Rte 31, an on-ramp to IL Rte 31, and widening of IL Rte 31 from a 2-lane section to a 5-lane section. The improvements will use a combination of enclosed drainage with curb and gutter and open swale drainage systems. Stormwater detention is provided in compliance with Kane County & IDOT requirements. Water quality runoff volume retention is also provided in compliance with Kane County requirements. Temporary and permanent soil erosion and sediment control are provided for all phases of construction. Wetland boundaries and wetland disturbance disturbance are displayed on the plans sheets. There are no areas of floodplain within the limits of Section B.

- C. Provide the estimated duration of this project:

Estimated duration of this project is two (2) years

- D. The total area of the construction site is estimated to be 108 acres.

The total area of the site estimated to be disturbed by excavation, grading or other activities is 33 acres.

- E. The following is a weighted average of the runoff coefficient for this project after construction activities are completed:

Weighted C= 0.60

- F. List all soils found within project boundaries. Include map unit name, slope information, and erosivity:

La Rose loam, 5 to 10 percent slopes, eroded
La Rose loam, 10 to 18 percent slopes, eroded

Herbert silt loam, 0 to 2 percent slopes
 Harpster silty clay loam, 0 to 2 percent slopes
 Houghton muck, 0 to 2 percent slopes
 Brenton silt loam, 0 to 2 percent slopes
 Drummer silty clay loam, 0 to 2 percent slopes
 Thorp silt loam, 0 to 2 percent slopes
 Varna silt loam, 2 to 4 percent slopes
 Varna silt loam, 4 to 6 percent slopes, eroded
 Peotone silty clay loam, 0 to 2 percent slopes
 Elpaso silty clay loam, 0 to 2 percent slopes
 Mundelein silt loam, 0 to 2 percent slopes
 Kidami silt loam, 2 to 4 percent slopes
 Kidami loam, 4 to 6 percent slopes, eroded
 Kidami loam, 6 to 12 percent slopes, eroded
 Senachwine silt loam, 12 to 20 percent slopes
 Octagon silt loam, 2 to 4 percent slopes
 Octagon silt loam, 4 to 6 percent slopes, eroded
 Octagon silt loam, 6 to 12 percent slopes, eroded
 Barony silt loam, 0 to 2 percent slopes
 Barony silt loam, 2 to 5 percent slopes
 Kaneville silt loam, 0 to 2 percent slopes

Please Refer to Attached Map and Summary

- G. Provide an aerial extent of wetland acreage at the site:

Please Refer to Attached Wetland Delineation Mapping

- H. Provide a description of potentially erosive areas associated with this project:

Potentially erosive soils are present just west of IL Rte 31 due to the high slopes and the high levels of excavation required to achieve the proposed finished profile.

- I. The following is a description of soil disturbing activities by stages, their locations, and their erosive factors (e.g. steepness of slopes, length of slopes, etc):
- 1) Installation of construction fencing, sediment control, silt fence and vegetation
 - 2) Clearing of the project site as shown in the staging plan
 - 3) Grading of detention ponds; this work is to be completed concurrently with the construction of sump pits, sediment basins, and temporary aggregate berms
 - 4) Topsoil spreading with temporary or permanent soil stabilization measures and the construction of permanent soil erosion and sediment control measures
 - 5) Removal of temporary soil erosion and sediment control measures
- J. See the erosion control plans and/or drainage plans for this contract for information regarding drainage patterns, approximate slopes anticipated before and after major grading activities, locations where vehicles enter or exit the site and controls to prevent offsite sediment tracking (to be added after contractor identifies locations), areas of soil disturbance, the location of major structural and non-structural controls identified in the plan, the location of areas where stabilization practices are expected to occur, surface waters (including wetlands) and locations where storm water is discharged to surface water including wetlands.
- K. Identify who owns the drainage system (municipality or agency) this project will drain into:
- Longmeadow Parkway - Kane County Division of Transportation
 IL Rte 31 - Illinois Department of Transportation
- L. The following is a list of General NPDES ILR40 permittees within whose reporting jurisdiction this project is located.

Kane County Division of Transportation
 Illinois Department of Transportation

Village of Algonquin

- M. The following is a list of receiving water(s) and the ultimate receiving water(s) for this site. The location of the receiving waters can be found on the erosion and sediment control plans:

Un-named Tributaries to the Fox River
Fox River

- N. Describe areas of the site that are to be protected or remain undisturbed. These areas may include steep slopes, highly erodible soils, streams, stream buffers, specimen trees, natural vegetation, nature preserves, etc.

All areas outside of the grading limits of the proposed roadway and all areas outside of the proposed ROW shall be protected and remain undisturbed.

- O. The following sensitive environmental resources are associated with this project, and may have the potential to be impacted by the proposed development:

- ☐ Floodplain
- ☒ Wetland Riparian
- ☐ Threatened and Endangered Species
- ☒ Historic Preservation
- ☐ 303(d) Listed receiving waters for suspended solids, turbidity, or siltation
- ☐ Receiving waters with Total Maximum Daily Load (TMDL) for sediment, total suspended solids, turbidity or siltation
- ☐ Applicable Federal, Tribal, State or Local Programs
- ☐ Other

1. 303(d) Listed receiving waters (fill out this section if checked above):

N/A

- a. The name(s) of the listed water body, and identification of all pollutants causing impairment:

N/A

- b. Provide a description of how erosion and sediment control practices will prevent a discharge of sediment resulting from a storm event equal to or greater than a twenty-five (25) year, twenty-four (24) hour rainfall event:

N/A

- c. Provide a description of the location(s) of direct discharge from the project site to the 303(d) water body:

N/A

- d. Provide a description of the location(s) of any dewatering discharges to the MS4 and/or water body:

N/A

2. TMDL (fill out this section if checked above)

- a. The name(s) of the listed water body:

N/A

- b. Provide a description of the erosion and sediment control strategy that will be incorporated into the site design that is consistent with the assumptions and requirements of the TMDL:

N/A

- c. If a specific numeric waste load allocation has been established that would apply to the project's discharges, provide a description of the necessary steps to meet that allocation:

N/A

- P. The following pollutants of concern will be associated with this construction project:

- ☒ Soil Sediment
- ☒ Petroleum (gas, diesel, oil, kerosene, hydraulic oil / fluids)

- | | |
|---|--|
| <input checked="" type="checkbox"/> Concrete | <input checked="" type="checkbox"/> Antifreeze / Coolants |
| <input checked="" type="checkbox"/> Concrete Truck Waste | <input checked="" type="checkbox"/> Waste water from cleaning construction equipment |
| <input checked="" type="checkbox"/> Concrete Curing Compounds | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Solid Waste Debris | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Paints | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Solvents | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Fertilizers / Pesticides | <input type="checkbox"/> Other (specify) |

II. Controls:

This section of the plan addresses the controls that will be implemented for each of the major construction activities described in I.C. above and for all use areas, borrow sites, and waste sites. For each measure discussed, the Contractor will be responsible for its implementation as indicated. The Contractor shall provide to the Resident Engineer a plan for the implementation of the measures indicated. The Contractor, and subcontractors, will notify the Resident Engineer of any proposed changes, maintenance, or modifications to keep construction activities compliant with the Permit ILR10. Each such Contractor has signed the required certification on forms which are attached to, and are a part of, this plan:

A. **Erosion and Sediment Controls:** At a minimum, controls must be coordinated, installed and maintained to:

1. Minimize the amount of soil exposed during construction activity;
2. Minimize the disturbance of steep slopes;
3. Maintain natural buffers around surface waters, direct storm water to vegetated areas to increase sediment removal and maximize storm water infiltration, unless infeasible;
4. Minimize soil compaction and, unless infeasible, preserve topsoil.

B. **Stabilization Practices:** Provided below is a description of interim and permanent stabilization practices, including site- specific scheduling of the implementation of the practices. Site plans will ensure that existing vegetation is preserved where attainable and disturbed portions of the site will be stabilized. Stabilization practices may include but are not limited to: temporary seeding, permanent seeding, mulching, geotextiles, sodding, vegetative buffer strips, protection of trees, preservation of mature vegetation, and other appropriate measures. Except as provided below in II(B)(1) and II(B)(2), stabilization measures shall be initiated **immediately** where construction activities have temporarily or permanently ceased, but in no case more than **one (1) day** after the construction activity in that portion of the site has temporarily or permanently ceases on all disturbed portions of the site where construction will not occur for a period of fourteen (14) or more calendar days.

1. Where the initiation of stabilization measures is precluded by snow cover, stabilization measures shall be initiated as soon as practicable.
2. On areas where construction activity has temporarily ceased and will resume after fourteen (14) days, a temporary stabilization method can be used.

The following stabilization practices will be used for this project:

- | | |
|---|--|
| <input checked="" type="checkbox"/> Preservation of Mature Vegetation | <input checked="" type="checkbox"/> Erosion Control Blanket / Mulching |
| <input type="checkbox"/> Vegetated Buffer Strips | <input type="checkbox"/> Sodding |
| <input checked="" type="checkbox"/> Protection of Trees | <input type="checkbox"/> Geotextiles |
| <input checked="" type="checkbox"/> Temporary Erosion Control Seeding | <input type="checkbox"/> Other (specify) dust control watering |
| <input type="checkbox"/> Temporary Turf (Seeding, Class 7) | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Temporary Mulching | <input type="checkbox"/> Other (specify) |
| <input checked="" type="checkbox"/> Permanent Seeding | <input type="checkbox"/> Other (specify) |

Describe how the stabilization practices listed above will be utilized during construction:

1) Protection of Trees/Temporary Fence: All trees designated to be saved, or outside the limits of construction, shall be protected prior to beginning any clearing or removal work and shall remain protected during subsequent construction work. Protection of trees shall be as shown on the plans or directed by the Engineer and in accordance with Article 201.05 of the Illinois Department of Transportation's Standard Specifications for Road and Bridge, latest edition.

2) Temporary Erosion Control Seeding: This item will be applied to all bare areas every seven days to minimize the amount of exposed surface areas. Temporary Erosion Control Seeding shall consist of areas as shown on the plans, areas disturbed during the removal of Soil and Erosion measures, or directed by the Engineer and in

accordance with the Illinois Department of Transportation's Standard Specifications for Road and Bridge, latest edition.

3) Permanent Seeding: This item will be utilized in small areas where sodding has failed as an interim remedy until sod can be replaced or as designated in rural areas where sod is not a prudent alternative. All disturbed areas, identified to receive seeding, will be stabilized via seeding immediately following final grading.

4) Erosion Control Blanket: This item will be used within 24 hours after seeding operations have been completed, in ditches/swales and sloped areas that require protection from erosion. Erosion control blankets shall be installed over fill slopes, high velocity areas and slopes steeper than 3:1 that have been brought to final grade. Erosion Control Blanket will be installed in accordance to IDOT Specification 251.04.

5) Dust Control Watering - This item will be provided for areas exposed during the mass grading/excavation to control the discharge of sediment through wind erosion during dry periods of construction, areas that are exposed during excavation shall receive dust control watering to minimize dust.

Describe how the stabilization practices listed above will be utilized after construction activities have been completed:

The erosion control practices listed above shall be removed upon final stabilization or incorporated into the final stabilization of the site.

- C. **Structural Practices:** Provided below is a description of structural practices that will be implemented, to the degree attainable, to divert flows from exposed soils, store flows or otherwise limit runoff and the discharge of pollutants from exposed areas of the site. Such practices may include but are not limited to: perimeter erosion barrier, earth dikes, drainage swales, sediment traps, ditch checks, subsurface drains, pipe slope drains, level spreaders, storm drain inlet protection, rock outlet protection, reinforced soil retaining systems, gabions, and temporary or permanent sediment basins. The installation of these devices may be subject to Section 404 of the Clean Water Act.

The following structural practices will be used for this project:

- | | |
|---|--|
| <input checked="" type="checkbox"/> Perimeter Erosion Barrier | <input type="checkbox"/> Rock Outlet Protection |
| <input checked="" type="checkbox"/> Temporary Ditch Check | <input checked="" type="checkbox"/> Riprap |
| <input checked="" type="checkbox"/> Storm Drain Inlet Protection | <input type="checkbox"/> Gabions |
| <input type="checkbox"/> Sediment Trap | <input type="checkbox"/> Slope Mattress |
| <input type="checkbox"/> Temporary Pipe Slope Drain | <input checked="" type="checkbox"/> Retaining Walls |
| <input type="checkbox"/> Temporary Sediment Basin | <input type="checkbox"/> Slope Walls |
| <input type="checkbox"/> Temporary Stream Crossing | <input type="checkbox"/> Concrete Revetment Mats |
| <input checked="" type="checkbox"/> Stabilized Construction Exits | <input type="checkbox"/> Level Spreaders |
| <input type="checkbox"/> Turf Reinforcement Mats | <input checked="" type="checkbox"/> Other (specify) Culvert Inlet Protection |
| <input checked="" type="checkbox"/> Permanent Check Dams | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Permanent Sediment Basin | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Aggregate Ditch | <input type="checkbox"/> Other (specify) |
| <input type="checkbox"/> Paved Ditch | <input type="checkbox"/> Other (specify) |

Describe how the structural practices listed above will be utilized during construction:

1) Perimeter Erosion Barrier: This item will be used to demarcate the perimeter of the project location and for the prevention of silt/sediment from leaving the site. Perimeter erosion barrier will be modified as necessary to accommodate the construction and repaired/replaced as necessary. This item will remain in place until all remaining items of the project have been completed.

2) Temporary Ditch Checks - These items will be used throughout the project limit as shown on the plans to reduce the runoff velocity and to trap silt before drains outside the project limit.

3) Storm Drain Inlet Protection: This item will be utilized at all manholes, catch basins and inlets with open grates. Inlet filters will be installed directly on the drainage structure or under the grate of the drainage structure resting on the lip of the frame. Inlet filters will be checked on a regular basis and any sediment/debris will be removed to maintain inlet protection. Storm Drain Inlet Protection will be done in accordance with Article 280.04 of the IDOT Specifications. Pipe protection will be implemented at outfalls.

4) Stabilized Construction Exits - Stabilized Construction Exits shall be used at the locations indicated on the plans for all construction traffic entering or exiting the construction site, Stabilized Construction Exits shall be continuously maintained during construction operations.

5) Rock Check Dams - This item will be provided for the proposed swales in areas with steep slopes, rock check dams will be placed at regular intervals as shown on the plans during construction and will be cleaned on a regular basis.

6) Stone Riprap: Stone Riprap of size and class as indicated on the plans will be placed to prevent erosion and scouring at the flared end sections at the upstream and downstream of storm sewer and culvert systems as shown on the plans and maintained as directed by the engineer.

8) Retaining Walls - Retaining Walls shall be used as indicated on the plans to limit the areas of excavation. Please refer to the structural drawings for detail and the plans for general layout of retaining walls

9) Culvert Inlet Protection - This item will be provided at locations where surface water is intercepted by a storm sewer culvert as indicated on the plans, this item will consist of stone placed in front of the culvert to prevent the discharge of transported sediment.

Describe how the structural practices listed above will be utilized after construction activities have been completed:

Temporary structural features including perimeter erosion barrier, temporary ditch checks, storm drain inlet protection, culvert inlet protection, and stabilized construction exits shall be removed upon completion of construction and final grade stabilization. Permanent structural features including rock check dams, stone riprap, and retaining walls shall be maintained throughout construction and shall become permanent features of the proposed improvements.

D. Treatment Chemicals

Will polymer flocculants or treatment chemicals be utilized on this project: ☒ Yes ☐ No

If yes above, identify where and how polymer flocculants or treatment chemicals will be utilized on this project.

Polymer flocculants may be used in conjunction with dewatering operations. At the discretion of the contractor and the direction of the engineer, polymer flocculants may be used to remove suspended solids from water pumped from excavations as required by construction operations. All pumping/dewatering shall follow the dewatering plan. All treated material resulting from the use of polymer flocculants shall be removed by the contractor.

E. **Permanent Storm Water Management Controls:** Provided below is a description of measures that will be installed during the construction process to control volume and pollutants in storm water discharges that will occur after construction operations have been completed. The installation of these devices may be subject to Section 404 of the Clean Water Act.

1. Such practices may include but are not limited to: storm water detention structures (including wet ponds), storm water retention structures, flow attenuation by use of open vegetated swales and natural depressions, infiltration of runoff on site, and sequential systems (which combine several practices).

The practices selected for implementation were determined on the basis of the technical guidance in Chapter 41 (Construction Site Storm Water Pollution Control) of the IDOT Bureau of Design and Environment Manual. If practices other than those discussed in Chapter 41 are selected for implementation or if practices are applied to situations different from those covered in Chapter 41, the technical basis for such decisions will be explained below.

2. Velocity dissipation devices will be placed at discharge locations and along the length of any outfall channel as necessary to provide a non-erosive velocity flow from the structure to a water course so that the natural physical and biological characteristics and functions are maintained and protected (e.g. maintenance of hydrologic conditions such as the hydroperiod and hydrodynamics present prior to the initiation of construction activities).

Description of permanent storm water management controls:

Stormwater management facilities are provided throughout the proposed improvements as required by the Kane County Ordinance. Four (4) separate conventional stormwater facilities are proposed in Section B. Stormwater facilities are proposed at STA 2081+00 LT, STA 2143+00 LT, STA 2163+00 RT, and STA

2180+00 RT. The detention basins have been designed in compliance with the Kane County Stormwater Ordinance and provide both stormwater detention storage and water quality runoff volume retention. Outlet protection in the form of riprap is proposed at storm sewers outletting to ponds, at discharge points to ponds and at overflow points where flow may be concentrated. The improvements proposed on IL Rte 31 have been designed in compliance with IDOT requirements and detention for the proposed improvements is provided in oversized storm sewer pipes. Please refer to the plans set for detailed pond plan information

- F. **Approved State or Local Laws:** The management practices, controls and provisions contained in this plan will be in accordance with IDOT specifications, which are at least as protective as the requirements contained in the Illinois Environmental Protection Agency's Illinois Urban Manual. Procedures and requirements specified in applicable sediment and erosion site plans or storm water management plans approved by local officials shall be described or incorporated by reference in the space provided below. Requirements specified in sediment and erosion site plans, site permits, storm water management site plans or site permits approved by local officials that are applicable to protecting surface water resources are, upon submittal of an NOI, to be authorized to discharge under the Permit ILR10 incorporated by reference and are enforceable under this permit even if they are not specifically included in the plan.

Description of procedures and requirements specified in applicable sediment and erosion site plans or storm water management plans approved by local officials:

The management practices, controls, and other provisions contained in this plan are at least as protective as the requirements contained in the Illinois Environmental Protection Agency's Illinois Urban Manual Standards and Specifications which was used as a guide in designing the erosion and sediment control features. Procedures and requirements specified in applicable soil erosion and sediment control plans or storm water management plans approved by local officials shall be described or incorporated by reference below. Requirements specified in soil erosion and sediment control plans, site permits, storm water management site plans, or site permits approved by county, state, or local officials that are applicable to protecting surface water resources are, upon submittal of a Notice of Intent (NOI), incorporated and enforceable under this permit even if they are not specifically included in the plan.

The soil erosion and sediment control for this site must meet the requirements of the following agencies:

Kane-DuPage Soil and Water Conservation District
Kane County Division of Transportation
Kane County Forest Preserve District
Illinois Department of Transportation
Illinois Environmental Protection Agency
U.S. Army Corps of Engineers

- G. **Contractor Required Submittals:** Prior to conducting any professional services at the site covered by this plan, the Contractor and each subcontractor responsible for compliance with the permit shall submit to the Resident Engineer a Contractor Certification Statement, BDE 2342a.

1. The Contractor shall provide a construction schedule containing an adequate level of detail to show major activities with implementation of pollution prevention BMPs, including the following items:
 - Approximate duration of the project, including each stage of the project
 - Rainy season, dry season, and winter shutdown dates
 - Temporary stabilization measures to be employed by contract phases
 - Mobilization timeframe
 - Mass clearing and grubbing/roadside clearing dates
 - Deployment of Erosion Control Practices
 - Deployment of Sediment Control Practices (including stabilized construction entrances/exits)
 - Deployment of Construction Site Management Practices (including concrete washout facilities, chemical storage, refueling locations, etc.)
 - Paving, saw-cutting, and any other pavement related operations
 - Major planned stockpiling operations
 - Timeframe for other significant long-term operations or activities that may plan non-storm water discharges such as dewatering, grinding, etc.
 - Permanent stabilization activities for each area of the project

2. The Contractor and each subcontractor shall provide, as an attachment to their signed Contractor Certification Statement, a discussion of how they will comply with the requirements of the permit in regard to the following items and provide a graphical representation showing location and type of BMPs to be used when applicable:
- Vehicle Entrances and Exits – Identify type and location of stabilized construction entrances and exits to be used and how they will be maintained.
 - Material Delivery, Storage and Use – Discuss where and how materials including chemicals, concrete curing compounds, petroleum products, etc. will be stored for this project.
 - Stockpile Management – Identify the location of both on-site and off-site stockpiles. Discuss what BMPs will be used to prevent pollution of storm water from stockpiles.
 - Waste Disposal – Discuss methods of waste disposal that will be used for this project.
 - Spill Prevention and Control – Discuss steps that will be taken in the event of a material spill (chemicals, concrete curing compounds, petroleum, etc.)
 - Concrete Residuals and Washout Wastes – Discuss the location and type of concrete washout facilities to be used on this project and how they will be signed and maintained.
 - Litter Management – Discuss how litter will be maintained for this project (education of employees, number of dumpsters, frequency of dumpster pick-up, etc.).
 - Vehicle and Equipment Fueling – Identify equipment fueling locations for this project and what BMPs will be used to ensure containment and spill prevention.
 - Vehicle and Equipment Cleaning and Maintenance – Identify where equipment cleaning and maintenance locations for this project and what BMPs will be used to ensure containment and spill prevention.
 - Dewatering Activities – Identify the controls which will be used during dewatering operations to ensure sediments will not leave the construction site.
 - Polymer Flocculants and Treatment Chemicals – Identify the use and dosage of treatment chemicals and provide the Resident Engineer with Material Safety Data Sheets. Describe procedures on how the chemicals will be used and identify who will be responsible for the use and application of these chemicals. The selected individual must be trained on the established procedures.
 - Additional measures indicated in the plan.

III. Maintenance:

When requested by the Contractor, the Resident Engineer will provide general maintenance guides to the Contractor for the practices associated with this project. The following additional procedures will be used to maintain, in good and effective operating conditions, the vegetation, erosion and sediment control measures and other protective measures identified in this plan. It will be the Contractor's responsibility to attain maintenance guidelines for any manufactured BMPs which are to be installed and maintained per manufacture's specifications.

The following is a description of procedures that will be used to maintain, in good and effective operating conditions, vegetation, soil erosion and sediment control measures, and other protective measures identified in this plan and standard specifications:

The contractor will identify an Erosion Control Representative for the project. His duties will be to supervise the maintenance of the soil erosion and sediment control measures and implementation of this plan.

The following shall be the minimum maintenance required:

- A. Vegetative soil erosion measures - the vegetative growth of temporary and permanent seeding, vegetative filters, etc, shall be maintained periodically and supplied adequate watering and fertilizer. The vegetative cover shall be removed and reseeded as necessary.
- B. Aggregate ditch checks / Rock Check Dams shall be cleaned of sediment when the sediment has reached a depth of 50% of the height of the aggregate berm.
- C. Sediment control, silt fence will be examined regularly and repaired as necessary. Sediment shall be removed when it reaches a height equal to 50% of the height of the barrier.
- D. Temporary seeding for erosion control will be repaired when bare stops and washout occur.
- E. Stabilized construction entrances shall have sediment build up removed as necessary.
- F. Inlet filters shall be cleaned on a regular basis
- G. Temporary and permanent erosion control measures shall be inspected weekly or after any rainfall event in excess of 0.50".

IV. Inspections:

Qualified personnel shall inspect disturbed areas of the construction site which have not yet been finally stabilized, structural control measures, and locations where vehicles and equipment enter and exit the site using IDOT Storm Water Pollution Prevention Plan Erosion Control Inspection Report (BC 2259). Such inspections shall be conducted at least once every seven (7) calendar days and within twenty-four (24) hours of the end of a storm or by the end of the following business or work day that is 0.5 inch or greater or equivalent snowfall.

Inspections may be reduced to once per month when construction activities have ceased due to frozen conditions. Weekly inspections will recommence when construction activities are conducted, or if there is 0.5" or greater rain event, or a discharge due to snowmelt occurs.

If any violation of the provisions of this plan is identified during the conduct of the construction work covered by this plan, the Resident Engineer shall notify the appropriate IEPA Field Operations Section office by email at: epa.swnoncomp@illinois.gov, telephone or fax within twenty-four (24) hours of the incident. The Resident Engineer shall then complete and submit an "Incidence of Non-Compliance" (ION) report for the identified violation within five (5) days of the incident. The Resident Engineer shall use forms provided by IEPA and shall include specific information on the cause of noncompliance, actions which were taken to prevent any further causes of noncompliance, and a statement detailing any environmental impact which may have resulted from the noncompliance. All reports of non-compliance shall be signed by a responsible authority in accordance with Part VI. G of the Permit ILR10.

The Incidence of Non-Compliance shall be mailed to the following address:

Illinois Environmental Protection Agency
Division of Water Pollution Control
Attn: Compliance Assurance Section
1021 North Grand East
Post Office Box 19276
Springfield, Illinois 62794-9276

Additional Inspections Required:

The following is a description of procedures that will be used to maintain, in good and effective operating conditions, vegetation, soil erosion and sediment control measures, and other protective measures identified in this plan and standard specifications: The contractor will designate an Erosion Control Representative for project. His duties will be to supervise the maintenance of the soil erosion and sediment control measures and implementation of this plan.

The following shall be the minimum maintenance required:

- A. Vegetative soil erosion measures - the vegetative growth of temporary and permanent seeding, vegetative filters, etc, shall be maintained periodically and supplied adequate watering and fertilizer. The vegetative cover shall be removed and reseeded as necessary.
- B. Pumping basins shall be cleaned of sediment when the sediment has reached a depth of 50% of the height of the aggregate berm.
- C. Sediment control, silt fence will be examined regularly and repaired as necessary. Sediment shall be removed when it reaches a height equal to 50% of the height of the barrier.
- D. Temporary seeding for erosion control will be repaired when bare stops and washout occur.
- E. Stabilized construction entrances shall have sediment build up removed as necessary.
- F. Inlet filters shall be cleaned on a regular basis
- G. Temporary and permanent erosion control measures shall be inspected weekly or after any rainfall event in excess of 0.50".

The engineer will be responsible for conducting soil erosion and sediment control inspections. The contractor's SESCM shall be notified when the inspections are to take place and is expected to be present during the inspections. A maintenance inspection report will be completed after each inspection. A copy of the report is to be completed by the inspector and stored on-site with a copy given to the contractor.

The inspection shall include all disturbed areas of the construction site which have not been finally stabilized, the structural control measures, locations where vehicles enter or exit the site and all major outfalls. Such inspection shall be conducted at least once every seven calendar days and within 24 hours of the end of a rain storm (or equivalent snowfall) that is 0.5 inches or greater. Depth of rain fall will be determined by an on-site rain gauge. The engineer shall read the rain gauge daily and after each rain storm.

- A. Disturbed areas and areas used for storage of materials that are exposed to precipitation shall be inspected for evidence of, or the potential for, pollutants entering the drainage system and waterways. Soil erosion and sediment control measures identified in the plan shall be observed to ensure that they are operating correctly. If repair is necessary, it will be initiated within 24 hours of the completion of the inspection report. Where discharge locations or points are accessible, they shall be inspected to ascertain whether the measures are effective in preventing significant impacts to receiving waters. Locations where vehicles enter or exit the site shall be inspected for evidence of off-site tracking.

B. Based on the results of the inspection, the description of potential pollutant sources and pollution prevention measures shall be evaluated. The storm water pollution prevention plan shall be revised as appropriate as soon as practicable after such inspection. Any changes to this plan resulting from the required inspection shall be implemented within seven calendar days following the inspection.

C. A report summarizing the scope of the inspection, name(s) and qualifications of personnel making the inspection, the date(s) of the inspection, major observations relating to the implementation of this Stormwater Pollution Prevention Plan, and action taken and retained as part of the plan for at least three years after the date of inspection. The report shall be signed in accordance with the general permit.

D. If any violations of the provisions of this plan are identified during the conduct of the construction work covered by this plan, the engineer shall complete and file an "incidence of noncompliance" (ion) report for the identified violation. The engineer shall use forms provided by the Illinois Environmental Protection Agency and shall include specific information about the cause of noncompliance, actions which were taken to prevent any further causes of noncompliance, and a statement detailing any environmental impact which may have resulted from the non-compliance. All reports of noncompliance shall be signed by a responsible authority in accordance with the general permit. The report of noncompliance shall be mailed to the Incidence of Non-Compliance Address listed above:

V. Failure to Comply:

Failure to comply with any provisions of this Storm Water Pollution Prevention Plan will result in the implementation of a National Pollutant Discharge Elimination System/Erosion and Sediment Control Deficiency Deduction against the Contractor and/or penalties under the Permit ILR10 which could be passed on to the Contractor.



Prior to conducting any professional services at the site covered by this contract, the Contractor and every subcontractor must complete and return to the Resident Engineer the following certification. A separate certification must be submitted by each firm. Attach to this certification all items required by Section II.G of the Storm Water Pollution Prevention Plan (SWPPP) which will be handled by the Contractor/subcontractor completing this form.

Route	<u>FAP 361 (Longmeadow Parkway)</u>	Marked Rte.	<u>FAP 361 (Longmeadow Parkway)</u>
Section	<u>13-00215-10-PV</u>	Project No.	<u>2013-1894</u>
County	<u>Kane County</u>	Contract No.	<u></u>

This certification statement is a part of SWPPP for the project described above, in accordance with the General NPDES Permit No. ILR10 issued by the Illinois Environmental Protection Agency.

I certify under penalty of law that I understand the terms of the Permit No. ILR 10 that authorizes the storm water discharges associated with industrial activity from the construction site identified as part of this certification.

In addition, I have read and understand all of the information and requirements stated in SWPPP for the above mentioned project; I have received copies of all appropriate maintenance procedures; and, I have provided all documentation required to be in compliance with the Permit ILR10 and SWPPP and will provide timely updates to these documents as necessary.

- ☐ Contractor
- ☐ Sub-Contractor

<u></u> Print Name	<u></u> Signature
<u></u> Title	<u></u> Date
<u></u> Name of Firm	<u></u> Telephone
<u></u> Street Address	<u></u> City/State/ZIP

Items which this Contractor/subcontractor will be responsible for as required in Section II.G. of SWPPP:



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BORING LOG BLA-B01

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 911.85 ft
North: 1993539.72 ft
East: 985715.72 ft
Station: 2079+29.20
Offset: 73.98 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	910.8	13-inch thick, dark brown, organic SILTY LOAM															
		--TOPSOIL--															
		Loose, brown ORGANIC SILT			1	2 2 3	NP	94									
	908.9	Very soft to soft, gray SILTY CLAY, little gravel	5		2	3 2 4	0.25 P	18									
					3	2 2 3	< 0.25 P	14									
	903.9	Stiff, gray SILTY CLAY LOAM, trace gravel	10		4	2 3 5	1.15 B	14									
					5	3 4 6	1.39 B	13									
	896.9		15		6	3 6 9	1.56 B	12									
		Boring terminated at 15.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **09-16-2014** Complete Drilling **09-16-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **3.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG BLA-B02

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 911.41 ft
North: 1993526.73 ft
East: 986036.27 ft
Station: 2082+50.02
Offset: 74.41 LT

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Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	910.2	14-inch thick, organic SILTY LOAM --TOPSOIL-- Very soft to stiff, gray SILTY CLAY, trace gravel			1	1 2 2	< 0.25 P	28									
			5		2	3 4 5	0.50 P	13									
					3	2 4 7	1.00 P	11									
	903.4	Medium dense, gray SANDY GRAVEL --Saturated--	10		4	5 8 16	NP										
					5	8 11 16	NP	14									
	898.4	Dense, pinkish gray LOAM, little gravel			6	8 14 16	NP	9									
	896.4	Boring terminated at 15.00 ft	15														
			20														
			25														

GENERAL NOTES

Begin Drilling **09-15-2014** Complete Drilling **09-15-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **8.00 ft**
At Completion of Drilling \blacktriangledown **8.00 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG BLA-B03

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 885.32 ft
North: 1993530.72 ft
East: 991776.79 ft
Station: 2140+00.83
Offset: 205.59 LT

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Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	884.0	16-inch thick, brown SILTY LOAM															
		--TOPSOIL--															
		Stiff to very stiff, brown CLAY LOAM, trace gravel			1	3 3 5	2.00 P	14									
			5		2	3 4 4	1.23 S	13									
					3	3 5 7	2.38 S	13									
			10		4	3 7 8	3.53 B	13									
					5	4 6 12	2.13 B	13									
	871.7	Dense, brown SANDY LOAM, trace gravel			6	5 13 23	NP	12									
	870.3	--Moist--	15														
		Boring terminated at 15.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **10-27-2014** Complete Drilling **10-27-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-50 ATV**
Driller **K&K** Logger **A. Happel** Checked by **B. Wilson**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG BLA-B04

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 885.29 ft
North: 1993538.26 ft
East: 992174.57 ft
Station: 2144+00.44
Offset: 200.65 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		15-inch thick, brown SILTY LOAM															
	883.8	--TOPSOIL--															
		Stiff to hard, brown CLAY LOAM to LOAM, trace gravel			1	3 3 4	1.64 S	14									
			5		2	3 5 6	2.30 S	13									
					3	4 5 7	4.18 B	14									
	875.8		10		4	3 4 6	1.39 S	15									
	874.8	--Hard drilling from 10'-11'; possible cobble--															
		Medium dense, brown SANDY LOAM, trace gravel			5	7 4 6	NP	13									
		--Moist--															
	870.3		15		6	4 6 10	NP	15									
		Boring terminated at 15.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **10-27-2014** Complete Drilling **10-27-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-50 ATV**
Driller **K&K** Logger **A. Happel** Checked by **B. Wilson**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG BLA-R01

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 919.14 ft
North: 1993395.84 ft
East: 987381.17 ft
Station: 2095+99.21
Offset: 2.87 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	918.84	4-inch thick, ASPHALT															
	918.5	--PAVEMENT--															
		4-inch thick, CRUSHED STONE															
		--BASE COURSE--															
		Very stiff to hard, brown and gray SILTY CLAY LOAM, trace gravel			1	7 5 5 6	> 4.50 P	9									
		--FILL--															
	914.6	Soft to medium stiff, black and brown ORGANIC SILTY CLAY to SILTY CLAY LOAM, some silt and sand lenses	5		2	2 4 4 4	2.50 P	13									
					3	2 3 4 4	1.00 P	45									
					4	2 2 3 2	0.50 P	25									
			10		5	2 2 2 2	0.90 B	42									
					6	1 1 2	0.74 B	59									
			15		7	1 2 1	0.25 P	51									
					8	0 0 1	0.57 B	65									
	901.1	Very soft to soft, gray and brown SILTY CLAY, trace gravel	20		9	2 1 2	0.41 B	17									
					10	2 3 4 3	< 0.25 P	15									
	897.1	Boring terminated at 22.00 ft															
			25														

GENERAL NOTES

Begin Drilling **06-18-2014** Complete Drilling **06-18-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **B-57 TMR**
Driller **R&D** Logger **A. Tomaras** Checked by **B. Wilson**
Drilling Method **2.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **13.00 ft**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG BLA-S01

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 920.47 ft
North: 1993336.94 ft
East: 987384.77 ft
Station: 2096+05.13
Offset: 61.59 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	920.23	23-inch thick, dark brown SILTY LOAM --TOPSOIL-- Stiff to very stiff, brown SILTY CLAY LOAM, trace gravel --FILL--			1	3 3 4	1.07 B	15									
			5		2	3 4 6	2.13 B	12									
	915.0	Stiff, brown and gray SILTY CLAY LOAM, little gravel			3	3 5 7	1.64 B	14									
	912.5	Medium stiff to stiff, brown CLAY LOAM, little gravel	10		4	3 4 6	1.07 B	14									
					5	3 7 8	0.98 B	15									
	907.5	Stiff, gray SILTY CLAY LOAM, little gravel	15		6	3 6 9	1.72 B	13									
	903.7	Very dense, gray SILTY LOAM, trace gravel			7	15 16 28	NR										
					8	25 26 25	NP	13									
	900.5	Boring terminated at 20.00 ft	20														
			25														

GENERAL NOTES

Begin Drilling **09-17-2014** Complete Drilling **09-17-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG BLA-S02

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 873.93 ft
North: 1993417.09 ft
East: 991599.58 ft
Station: 2138+19.47
Offset: 94.78 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	872.9	12-inch thick, black SILTY LOAM --TOPSOIL--								848.4	Loose, gray LOAM, trace gravel						
		Very stiff to hard, dark brown and black SILTY CLAY LOAM, trace gravel			1	7 5 7	4.25 P	19						11	3 4 5	NP	10
			5		2	3 3 3	3.50 P	19				30		12	3 3 4	NP	10
	868.4	Stiff (1.25 P), brown and gray CLAY LOAM			3	2 3 5	NP	11		842.2	Medium dense, gray SILT, trace gravel						
	867.8	Loose to medium dense, brown and gray, LOAM, trace gravel --Moist--			4	6 7 8	NP	9			--Saturated--			13	4 7 10	NR	15
	863.4	Loose to medium dense, brown and gray, fine to medium SAND --Saturated--			5	2 2 3	NP	28									
	859.4	Medium stiff to hard, gray CLAY LOAM, trace gravel	15		6	7 12 17	NP	22		834.9	Stiff to very stiff, gray CLAY LOAM, trace gravel	40		14	5 6 8	1.72 B	13
					7	4 8 11	1.07 B	10									
					8	3 5 7	1.39 B	11						15	9 13 20	2.95 B	13
			20		9	5 7 8	1.15 B	11		828.9	Boring terminated at 45.00 ft	45					
					10	3 4 6	0.82 B	11				50					

GENERAL NOTES

Begin Drilling **10-30-2014** Complete Drilling **10-30-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-50 ATV**
Driller **K&K** Logger **A. Happel** Checked by **B. Wilson**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **10.50 ft**
At Completion of Drilling **9.00 ft**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG BLA-S03

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 870.68 ft
North: 1993227.81 ft
East: 991607.45 ft
Station: 2138+25.79
Offset: 94.56 RT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	869.7	12-inch thick, black SILTY LOAM --TOPSOIL--															
		Stiff to very stiff, brown and gray SILTY CLAY LOAM, trace gravel			1	4 4 6	3.20 S	18						11	21 25 18	NP	8
					2	2 2 5	1.64 S	19				30		12	12 16 23	NP	10
	865.2	Dense, brown and gray SANDY LOAM, little gravel --Saturated--			3	10 16 18	NP	17									
					4	10 15 15	NP	9						13	16 40 50/4"	NP	10
	858.9	Dense to very dense, pinkish gray to gray SILTY LOAM, little gravel			5	12 13 20	NR			836.2 835.9	Brown and gray, fine SAND Hard, pinkish gray CLAY LOAM, trace gravel	35					
					6	13 14 21	NP	9						14	14 19 26	8.61 B	10
					7	7 24 35	NP	9		830.7	Boring terminated at 40.00 ft						
					8	22 27 33	NP	5									
					9	20 28 31	NP	7									
					10	20 30 29	NP	7									

GENERAL NOTES

Begin Drilling **10-30-2014** Complete Drilling **10-30-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-50 ATV**
Driller **K&K** Logger **A. Happel** Checked by **B. Wilson**
Drilling Method **3.25" HSA; boring backfilled upon completion**

WATER LEVEL DATA

While Drilling **5.50 ft**
At Completion of Drilling **34.00 ft**
Time After Drilling **NA**
Depth to Water **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG PDB-01

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 913.82 ft
North: 1993498.26 ft
East: 985669.37 ft
Station: 2078+84.63
Offset: 30.62 LT

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Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	912.7	14-inch thick, dark brown, organic SILTY LOAM --TOPSOIL-- Stiff, brown and gray SILTY CLAY LOAM, trace gravel			1	2 2 4	1.25 P	19									
			5		2	2 5 6	1.39 B	14									
					3	2 4 6	1.72 B	13									
	904.3	Medium dense, pinkish gray LOAM, trace gravel	10		4	3 9 12	1.23 B	14									
					5	10 9 14	NP	9									
	898.8	Boring terminated at 15.00 ft	15		6	4 7 7	NP	9									
			20														
			25														

GENERAL NOTES

Begin Drilling **09-15-2014** Complete Drilling **09-15-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG PDB-02

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 913.11 ft
North: 1993494.11 ft
East: 985786.92 ft
Station: 2080+02.25
Offset: 31.39 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	912.1	12-inch thick, dark brown, organic SILT LOAM															
		--TOPSOIL--															
		Stiff, gray SILTY CLAY to SILTY CLAY LOAM, trace gravel			1	3 3 5	1.75 P	23									
			5		2	3 3 5	1.31 B	15									
					3	4 5 5	1.97 B	12									
			10		4	2 4 5	1.97 B	11									
	902.6	Medium dense, pinkish gray LOAM, little gravel			5	6 7 7	NP	9									
	898.1		15		6	4 5 9	NP	11									
		Boring terminated at 15.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **09-16-2014** Complete Drilling **09-16-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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BORING LOG PDB-03

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 911.35 ft
North: 1993489.70 ft
East: 985887.67 ft
Station: 2081+03.10
Offset: 31.20 LT

Page 1 of 1

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	910.6	9-inch thick, dark brown, organic SILTY LOAM															
	909.7	--TOPSOIL-- 11-inch thick, brown PEAT			1	1 1 2	NP	186									
		Very loose, brown, organic SILT															
	907.1	Soft to stiff, gray SILTY CLAY LOAM, little gravel	5		2	1 0 1	NR										
					3	1 2 2	0.25 P	16									
			10		4	1 3 6	1.25 P	15									
					5	3 5 7	1.23 B	16									
					6	4 7 10	1.72 B	12									
	896.4	Boring terminated at 15.00 ft	15														
			20														
			25														

GENERAL NOTES

Begin Drilling **09-16-2014** Complete Drilling **09-16-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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BORING LOG PDB-04

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 912.75 ft
North: 1993485.97 ft
East: 985985.89 ft
Station: 2082+01.38
Offset: 31.58 LT

Page 1 of 1

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	912.2	7-inch thick, dark brown, organic SILTY LOAM --TOPSOIL-- Loose, brown SANDY LOAM, trace gravel --FILL--			1	4 4 5	NP	12									
			5		2	4 4 5	NP	12									
	907.3	Soft to medium stiff, gray SILTY CLAY, trace gravel			3	2 4 4	0.82 B	14									
			10		4	2 2 4	0.49 B	26									
	902.3	Dense, gray SILTY LOAM, some gravel --Saturated--			5	10 16 19	NP	12									
	899.8	Very stiff, gray SILTY CLAY LOAM, little gravel			6	12 10 10	2.00 P	14									
	897.8	Boring terminated at 15.00 ft	15														
			20														
			25														

GENERAL NOTES

Begin Drilling **09-16-2014** Complete Drilling **09-16-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **11.50 ft**
At Completion of Drilling \blacktriangledown **11.50 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG PDB-05

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 911.04 ft
North: 1993480.74 ft
East: 986086.12 ft
Station: 2083+01.75
Offset: 30.55 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	909.9	14-inch thick, dark brown, organic SILTY LOAM --TOPSOIL-- Very loose, brown PEAT			1	1 1 1	NP	341									
	905.5	Loose, gray SILTY LOAM, little gravel --Saturated--	5		2	1 0 1	NP	535									
	903.0	Soft, gray SILTY CLAY, little gravel	10		3	2 2 2	NP	14									
	900.5	Medium dense, gray SILTY LOAM, trace gravel	15		4	1 2 3	0.49 B	12									
	898.0	Medium dense, gray SILTY LOAM, trace gravel			5	4 7 5	NP	14									
	896.0	Medium dense, gray, coarse SAND, some gravel			6	3 5 6	NP	12									
		Boring terminated at 15.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **09-16-2014** Complete Drilling **09-16-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **6.50 ft**
At Completion of Drilling \blacktriangledown **12.00 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG PDB-06

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 911.24 ft
North: 1993539.24 ft
East: 985836.15 ft
Station: 2080+49.54
Offset: 78.54 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	910.0	15-inch thick, dark brown, organic SILTY LOAM															
		--TOPSOIL--															
	908.2	Very loose, brown, ORGANIC SILT			1	1 1 1	NP	151									
		Loose, gray SILTY LOAM, some gravel			2	1 3 4	NP	14									
		--Saturated--															
	905.7	Very soft to very stiff, gray SILTY CLAY LOAM, trace gravel			3	1 2 3	0.25 P	19									
					4	2 3 4	1.15 B	13									
					5	5 6 10	1.00 P	14									
					6	4 6 9	1.00 P	14									
					7	5 5 11	1.00 P	15									
					8	3 5 10	0.82 B	14									
					9	8 10 10	2.13 B	10									
	888.2	Medium dense, pinkish gray LOAM, trace gravel			10	8 11 14	NP	9									
	886.2	Boring terminated at 25.00 ft	25														

GENERAL NOTES

Begin Drilling **09-15-2014** Complete Drilling **09-15-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **4.00 ft**
At Completion of Drilling ∇ **3.00 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG PDB-07

WEI Job No.: 310-06-01

Client **Crawford, Murphy, & Tilly, Inc.**
Project **Longmeadow Parkway Corridor**
Location **Kane County, Illinois**

Datum: NAVD 88
Elevation: 910.98 ft
North: 1993536.41 ft
East: 985936.19 ft
Station: 2081+49.62
Offset: 79.90 LT

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	909.7	15-inch thick, dark brown, organic SILTY LOAM --TOPSOIL-- Very loose, brown PEAT			1	1 0 1	NP	458									
	906.7	Very soft to very stiff, gray SILTY CLAY LOAM, trace gravel	5		2	1 0 0	NR										
					3	1 0 2	< 0.25 P	18									
			10		4	1 2 4	0.66 B	14									
					5	3 4 6	1.64 B	14									
			15		6	4 7 9	2.30 B	12									
					7	8 13 13	2.62 B	12									
					8	4 6 8	1.64 B	13									
	891.0	Boring terminated at 20.00 ft	20														
			25														

GENERAL NOTES

Begin Drilling **09-15-2014** Complete Drilling **09-15-2014**
Drilling Contractor **Wang Testing Services** Drill Rig **D-25 ATV**
Driller **P&P** Logger **D. Kolpacki** Checked by **B. Wilson**
Drilling Method **2.25" HSA, boring backfilled upon completion**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 922.65 ft
North: 1993496.30 ft
East: 984984.14 ft
Station: 2072+00.08
Offset: 0.02 R

[illegible]

WATER LEVEL DATA

Begin Drilling	06-06-2005	Complete Drilling	06-06-2005
Drilling Contractor	PRECON DRILLING	Drill Rig	CME-75 ATV
Driller	K	Logger	J. Kosloski
		Checked by	B. Fugiel
Drilling Method	3.25-inch HSA		

While Drilling		DRY
At Completion of Drilling		DRY
Time After Drilling	NA	
Depth to Water		NA

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BORING LOG R-026

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 922.63 ft
North: 1993483.71 ft
East: 985283.68 ft
Station: 2074+99.89
Offset: 0.06 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	921.6	12-inch thick, dark brown SILTY LOAM															
		--TOPSOIL--															
	920.1	Stiff, brown SILTY CLAY LOAM			1	5 3 2 2	1.75 P	24									
		Loose, brown SILTY LOAM															
	918.1				2	3 3 6 5	NP	15									
		Loose, brown SANDY LOAM	5														
	916.6				3	3 3 5 5	NP	19									
		Loose, brown SILTY LOAM															
	914.6				4	3 10 8 5	NP	10									
	913.9	Medium dense, brown GRAVELLY SAND															
		Very stiff to hard, brown SILTY CLAY LOAM	10		5	1 1 3 6	2.00 P	15									
					6	4 6 14 33	> 4.50 P	15									
	909.6	Boring terminated at 13.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **06-06-2005** Complete Drilling **06-06-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K** Logger **J. Kosloski** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-027

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 921.00 ft
North: 1993471.14 ft
East: 985583.50 ft
Station: 2077+99.96
Offset: 0.07 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
		36-inch, stiff to very stiff, black SILTY CLAY LOAM --TOPSOIL--			1	2 2 1 2	2.00 P	20									
	918.0	Medium stiff to stiff, brown and gray SILTY CLAY, with interbedded sand lenses			2	4 4 4 5	1.25 P	26									
			5		3	5 4 6 6	0.75 P	15									
	913.5	Medium dense, brown and gray SANDY LOAM			4	2 5 6 9	1.50 P	14									
					5	9 7 11 17											
	910.5	Very stiff, gray SILTY CLAY LOAM			6	4 16 7 8	0.75 P	13									
					7	3 4 7 9	2.25 P	11									
	907.0	Boring terminated at 14.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **9.00 ft**
At Completion of Drilling \blacktriangledown **8.00 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-028

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 919.14 ft
North: 1993453.47 ft
East: 985893.25 ft
Station: 2081+09.97
Offset: 0.47 L

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Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	918.1	12-inch thick, black SILTY CLAY LOAM			1	3 3 3	2.75 P	17									
	917.4	--TOPSOIL--															
	916.1	Very stiff, brown and black SILTY CLAY			2	4 3 2	NP	12									
		--FILL--															
		Loose, brown LOAM															
		--FILL--															
	913.6	Medium stiff to very stiff, brown, black, and gray SILTY CLAY	5		3	3 3 3	0.75 P	20									
		--FILL--															
		Stiff to hard, brown SILTY CLAY			4	3 3 2	1.25 P	16									
	909.6	Medium stiff, gray ORGANIC SILT CLAY LOAM	10		5	3 2 4	4.25 P	13									
	907.6	Very soft, gray SILTY CLAY			6	2 3 3	0.50 P	62									
	905.6	Soft to stiff, gray SILTY CLAY LOAM	15		7	2 2 1	0.25 P	30									
					8	2 2 4	0.25 P	16									
	901.1	Boring terminated at 18.00 ft			9	1 2 3	1.25 P	15									
						4											

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-029

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 917.45 ft
North: 1993435.62 ft
East: 986182.82 ft
Station: 2083+99.81
Offset: 0.01 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	916.9	6-inch thick ASPHALT															
	916.4	--PAVEMENT--															
		6-inch thick CRUSHED STONE AGGREGATE			1	5 7 7 7	3.00 P	11									
		--BASE COURSE--															
		Soft to very stiff, brown SILTY CLAY LOAM			2	10 6 6 9	2.25 P	14									
		--FILL--															
			5		3	4 5 5 6	1.50 P	13									
					4	4 5 7 7	1.75 P	15									
			10		5	3 3 3 4	0.25 P	16									
					6	5 5 4 4	2.50 P	15									
	904.9	Medium stiff to stiff, gray SILTY CLAY LOAM			7	3 3 5 4	0.50 P	15									
			15		8	4 4 4 5	0.50 P	14									
					9	2 4 6 5	1.25 P	13									
	898.4	Boring terminated at 19.00 ft															
			20														
			25														

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-030

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 919.55 ft
North: 1993434.10 ft
East: 986482.73 ft
Station: 2086+99.96
Offset: 0.02 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	919.34	4-inch thick, dark brown SILTY CLAY LOAM			1	6 5 5 5 5	> 4.50 P	10									
	918.6	--TOPSOIL-- Hard, black and brown SILTY CLAY			2	7 7 8 8 12	2.75 P	14									
	915.8	--FILL-- Very stiff, brown SILTY CLAY			3	7 8 8 7	NP	19									
	913.8	Medium dense, brown and gray LOAM	5		4	4 9 9 7	NP	12									
		Medium dense, brown GRAVELLY SAND			5	7 12 14 14	NP	12									
	909.6	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **6.25 ft**
At Completion of Drilling \blacktriangledown **6.50 ft**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-031

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 921.25 ft
North: 1993422.40 ft
East: 986782.46 ft
Station: 2089+99.92
Offset: 0.09 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	921.04	0.4-inch thick, dark brown SILTY CLAY LOAM			1	4 7 5 9	3.25 P	17									
	918.8	Very stiff, brown SILTY CLAY LOAM			2	8 9 9 12	2.50 P	13									
		Medium dense, brown SANDY LOAM	5		3	6 6 7 8	NP	10									
					4	4 7 6 8	NP	10									
					5	5 5 8 10	NP	10									
	911.3	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **9.25 ft**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-032

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 921.31 ft
North: 1993410.59 ft
East: 987082.36 ft
Station: 2093+00.05
Offset: 0.09 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	921.04	4-inch thick, dark brown SILTY CLAY LOAM			1	4 6 6 7	3.50 P	14									
		--TOPSOIL--															
	919.0	Very stiff, black and brown SILTY CLAY			2	6 7 7 9	2.50 P	16									
		--FILL--															
		Stiff to very stiff, brown SILTY CLAY															
	916.8	Loose to medium dense, brown SANDY LOAM	5		3	6 9 13 11	1.75 P	11									
					4	4 4 4 5	NP	11									
					5	2 4 4 4	NP	12									
			10		6	4 5 6 6	NP	9									
					7	3 7 9 10	NP	9									
	907.3	Boring terminated at 14.00 ft	15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-27-2005** Complete Drilling **04-27-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME 55 TMR**
Driller **K&D** Logger **J. Kasnick** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 914.97 ft
North: 1993426.60 ft
East: 987383.31 ft
Station: 2096+00.13
Offset: 27.94 L

[illegible]

WATER LEVEL DATA

Begin Drilling	06-06-2005	Complete Drilling	06-06-2005
Drilling Contractor	PRECON DRILLING	Drill Rig	CME 55 TMR
Driller	K	Logger	J. Kosloski
		Checked by	B. Fugiel
Drilling Method	3.25-inch HSA		

While Drilling		DRY
At Completion of Drilling		DRY
Time After Drilling	NA	
Depth to Water		NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-034

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 920.60 ft
North: 1993410.98 ft
East: 987685.08 ft
Station: 2098+99.69
Offset: 26.61 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	919.6	12-inch thick, dark brown SILTY LOAM			1	4 5 5 5	NP	22									
		--TOPSOIL--															
		Very stiff to hard, brown SILTY CLAY LOAM			2	5 7 7 9	> 4.50 P	21									
			5		3	11 9 10 11	> 4.50 P	12									
					4	6 6 17 22	3.50 P	12									
	912.1	Stiff, brown SILTY CLAY			5	3 6 7 8	1.50 P	17									
			10														
	910.1	Hard, brown and gray SILTY CLAY LOAM			6	4 8 11 14	> 4.50 P	11									
					7	4 9 11 11	> 4.50 P	11									
	906.6	Boring terminated at 14.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **06-06-2005** Complete Drilling **06-06-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K** Logger **J. Kosloski** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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BORING LOG R-035

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 915.86 ft
North: 1993375.04 ft
East: 987981.62 ft
Station: 2102+00.01
Offset: 0.01 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	914.9	12-inch thick, dark brown SILTY LOAM			1	3 3 5 5	NP	14									
	913.9	--TOPSOIL-- Loose, brown LOAM			2	5 7 6 7	4.50 P	17									
		Stiff to hard, brown SILTY CLAY LOAM	5		3	3 6 6 7	2.50 P	13									
					4	3 4 4 5	1.50 P	12									
	907.4	Medium dense, brown SANDY LOAM	10		5	3 5 5 5	NP	10									
	903.9				6	3 6 8 10	NP	11									
		Boring terminated at 12.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **06-06-2005** Complete Drilling **06-06-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K** Logger **J. Kosloski** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-036

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**




Datum: NGVD
Elevation: 907.11 ft
North: 1993363.18 ft
East: 988281.41 ft
Station: 2105+00.04
Offset: 0.06 R

[illegible]

GENERAL NOTES

Begin Drilling	06-06-2005	Complete Drilling	06-06-2005
Drilling Contractor	PRECON DRILLING	Drill Rig	CME-75 ATV
Driller	K	Logger	J. Kosloski
		Checked by	B. Fugiel
Drilling Method	3.25-inch HSA		

WATER LEVEL DATA

While Drilling		DRY
At Completion of Drilling		DRY
Time After Drilling	NA	
Depth to Water		NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-037

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 909.94 ft
North: 1993351.48 ft
East: 988578.16 ft
Station: 2108+00.01
Offset: 0.05 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	909.46	6-inch thick, brown SANDY LOAM			1	3 3 2 4	3.50 P	15									
		--TOPSOIL--															
	907.4	Very stiff, brown SILTY CLAY LOAM			2	4 6 5 6	3.75 P	23									
	906.4	--FILL--															
		Very stiff, black and gray CLAY LOAM			3	2 2 4 5	1.50 P	27									
		Stiff, brown and gray CLAY	5														
	904.4	Very stiff, brown and gray SILTY CLAY			4	3 3 4 6	2.00 P	17									
					5	3 5 8 8	3.00 P	13									
	899.9	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-038

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 911.73 ft
North: 1993339.66 ft
East: 988880.98 ft
Station: 2111+00.04
Offset: 0.05 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	911.26	26-inch thick, black CLAY LOAM --TOPSOIL-- Medium stiff to very stiff, brown SILTY CLAY LOAM			1	1 3 6 7	2.50 P	26									
					2	3 4 4 4	0.50 P	16									
			5		3	3 4 5 8	1.50 P	14									
					4	3 4 7 6	1.50 P	14									
					5	2 4 6 7	2.00 P	13									
	901.7	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-039

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 910.78 ft
North: 1993336.67 ft
East: 989180.92 ft
Station: 2114+00.01
Offset: 0.00 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	910.1	8-inch thick, black CLAY LOAM --TOPSOIL-- Very stiff to hard, brown SILTY CLAY LOAM --FILL--			1	3 5 7 8	2.00 P	12									
					2	4 5 7 8	3.00 P	17									
			5		3	5 10 8 9	3.75 P	12									
	905.3	Stiff, brown SILTY CLAY LOAM			4	4 4 5 6	1.00 P	14									
					5	4 5 8 12	1.00 P	13									
	900.8	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-040

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 911.67 ft
North: 1993334.71 ft
East: 989481.92 ft
Station: 2117+00.0
Offset: 0.04 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	911.26	6-inch thick, black SILTY LOAM --TOPSOIL--			1	2 5 6 6	1.50 P	10									
	910.2	Stiff, brown SILTY CLAY LOAM, with gravel --FILL--			2	3 3 4 5	2.25 P	15									
		Very stiff to hard, brown and gray SILTY CLAY LOAM	5		3	2 2 4 7	2.75 P	14									
					4	5 8 12 16	4.00 P	13									
					5	8 11 11 14	3.50 P	10									
	901.7	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-041

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 909.39 ft
North: 1993332.78 ft
East: 989780.94 ft
Station: 2120+00.04
Offset: 0.04 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	908.9	6-inch thick, dark brown CLAY LOAM			1	2 3 3 6	2.00 P	17									
		--TOPSOIL--															
	906.9	Very stiff, brown SILTY CLAY, with gravel			2	7 11 11 13	NP	10									
		--FILL--															
		Medium dense, brown SANDY LOAM	5		3	9 6 5 4	NP	12									
	903.6	Very stiff, brown SILTY CLAY LOAM			4	5 5 6 9	2.00 P	12									
					5	9 12 11 18	2.00 P	10									
	899.4	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-042

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 903.99 ft
North: 1993331.03 ft
East: 990075.27 ft
Station: 2123+00.12
Offset: 0.14 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	903.4	7-inch thick, dark brown SILTY CLAY LOAM			1	3 3 4 15	1.75 P	13									
		--TOPSOIL--															
		Stiff to very stiff, brown SILTY CLAY LOAM			2	8 10 8 9	1.00 P	10									
		--FILL															
			5		3	4 4 4 6	2.50 P	17									
					4	3 2 3 4	1.50 P	32									
	895.7	Stiff, brown and gray SILTY CLAY LOAM			5	2 2 3 5	1.50 P	24									
	894.0	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-11-2005** Complete Drilling **04-11-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **J&R** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-043

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 905.70 ft
North: 1993328.72 ft
East: 990380.78 ft
Station: 2125+99.90
Offset: 0.25 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	905.26	6-inch thick, brown CLAY LOAM --TOPSOIL-- Loose, brown LOAM			1	2 2 3 4	NP	22									
	903.5	Loose to medium dense, brown SILT, with interbedded sand lenses			2	1 2 4 8	NP	20									
	900.5	Very stiff to hard, brown and gray SILTY CLAY LOAM, some gravel			3	4 9 11 15	NP	16									
					4	6 7 11 17	2.00 P	10									
					5	6 11 12 15	4.00 P	11									
	895.7	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-12-2005** Complete Drilling **04-12-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K&S** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-044

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 904.68 ft
North: 1993327.00 ft
East: 990680.86 ft
Station: 2128+99.98
Offset: 0.04 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	904.2	6-inch thick, brown CLAY LOAM --TOPSOIL-- Stiff, brown CLAY LOAM			1	2 5 4 5	1.75 P	15									
	902.2	Medium dense, brown SILT			2	3 5 7 7	NP	17									
	900.7	Very stiff to hard, brown SILTY CLAY LOAM	5		3	3 4 7 6	2.00 P	15									
					4	5 8 11 13	3.50 P	13									
			10		5	7 12 15 14	4.00 P	10									
					6	6 11 12 13	4.00 P	12									
	891.7	Boring terminated at 13.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-12-2005** Complete Drilling **04-12-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K&S** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**
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Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 888.11 ft
North: 1993325.00 ft
East: 990980.99 ft
Station: 2132+00.11
Offset: 0.12 R

[illegible]

WATER LEVEL DATA

Begin Drilling	04-13-2005	Complete Drilling	04-13-2005
Drilling Contractor	PRECON DRILLING	Drill Rig	CME-75 ATV
Driller	K&S	Logger	Y. Shiu
		Checked by	B. Fugiel
Drilling Method	3.25-inch HSA		

While Drilling		DRY
At Completion of Drilling		DRY
Time After Drilling	NA	
Depth to Water		NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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BORING LOG R-046

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 881.33 ft
North: 1993323.14 ft
East: 991280.86 ft
Station: 2134+99.99
Offset: 0.05 R

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	880.3	12-inch thick, black CLAY LOAM --TOPSOIL--			1	1 4 4 4	2.50 P	26									
		Very stiff, brown SILTY CLAY			2	1 3 4 4	2.00 P	24									
	877.1				3	2 3 3 4	NP	23									
	875.8	Loose, brown SILTY LOAM	5														
		Stiff, brown and black SILTY CLAY			4	5 5 7 8	1.50 P	23									
	872.3				5	3 6 5 5	4.00 P	13									
	871.8	Medium dense, gray SILT															
	871.3	Hard, gray SILTY CLAY LOAM	10														
		Boring terminated at 10.00 ft															
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-13-2005** Complete Drilling **04-13-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K&S** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-047

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 881.10 ft
North: 1993324.67 ft
East: 991717.36 ft
Station: 2139+36.52
Offset: 0.83 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	880.6	6-inch thick, brown CLAY LOAM --TOPSOIL-- Stiff to very stiff, brown SILTY CLAY LOAM			1	2 4 4 5	2.25 P	17									
					2	2 4 4 3	1.25 P	16									
			5		3	2 4 3 7	2.25 P	16									
	874.9	Medium dense, brown LOAM			4	5 6 7 13	NP	10									
	872.1	Dense, brown GRAVELLY SAND			5	4 11 23 25	NP	8									
	871.1	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-13-2005** Complete Drilling **04-13-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K&S** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

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BORING LOG R-048

WEI Job No.: 201-23-01

Client **McDonough Associates Inc.**
Project **Longmeadow Parkway over Fox River**
Location **Kane County, Illinois**

Datum: NGVD
Elevation: 885.42 ft
North: 1993328.06 ft
East: 991880.74 ft
Station: 2140+99.94
Offset: 0.09 L

Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)
	884.9	6-inch thick, black and brown CLAY LOAM			1	1 4 3 3	1.75 P	20									
	883.7	--TOPSOIL-- Stiff, brown CLAY LOAM			2	2 2 3 2	0.50 B	27									
	880.4	Medium stiff, brown SILTY CLAY	5		3	3 3 3 4	NP	16									
		Loose to medium dense, brown SANDY LOAM			4	3 5 7 8	NP	11									
	877.4	Very stiff, brown SILTY CLAY LOAM			5	6 6 7 7	2.50 P	10									
	875.4	Boring terminated at 10.00 ft	10														
			15														
			20														
			25														

GENERAL NOTES

Begin Drilling **04-13-2005** Complete Drilling **04-13-2005**
Drilling Contractor **PRECON DRILLING** Drill Rig **CME-75 ATV**
Driller **K&S** Logger **Y. Shiu** Checked by **B. Fugiel**
Drilling Method **3.25-inch HSA**

WATER LEVEL DATA

While Drilling ∇ **DRY**
At Completion of Drilling \blacktriangledown **DRY**
Time After Drilling **NA**
Depth to Water ∇ **NA**

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

**Illinois Department of Natural Resources
Office of Resource Conservation**

**Conservation Plan for the Incidental Taking of Threatened &
Endangered Species**

For The

**Longmeadow Parkway Project
Algonquin, Barrington Hills, Carpentersville,
& Unincorporated Kane County, Illinois**

Applicant: Kane County Division of Transportation

June 2015

1) Description of Impact

The Longmeadow Parkway Fox River Bridge Corridor (hereon referred to as “Corridor”) is a proposed four-lane Fox River Bridge crossing and four-lane arterial roadway corridor with a median, approximately 5.6 miles in length, to alleviate traffic congestion in northern Kane County. The proposed road passes through portions of the Villages of Algonquin, Carpentersville, and Barrington Hills, as well as unincorporated areas of Kane County. The western terminus is at Huntley Road west of Randall Road, approximately 1,300 feet northwest of the Huntley/Boyer Roads intersection. From Huntley Road to the Fox River, the Corridor primarily traverses mostly undeveloped properties and agricultural fields or new subdivisions. These subdivisions were developed with a dedicated right-of-way (ROW) to accommodate the proposed Corridor. After crossing the Fox River, the Corridor parallels the existing Bolz Road, to the eastern project terminus at Illinois Route 62 (IL 62). A quarry is located north of Bolz Road and east of Sandbloom Road, with undeveloped properties and new subdivisions present along the remainder of the proposed Corridor to IL 62.

The proposed improvements consist of construction of mainline pavements and the construction of a bridge across the Fox River. The proposed widening includes all associated infrastructure, such as lighting, storm sewers, and the construction of permanent Best Management Practices (BMPs) including naturalized detention basins, bioswales and vegetated swales, and minimization of impervious surfaces to facilitate exemplary water quality standards within the Corridor. A grade separation and interchange is proposed at the Illinois Route 31 (IL 31) crossing. Construction of the Fox River Bridge will accommodate two travel lanes in each direction along with curb and gutter, a raised median and a multi-use path on the north side. Figure 1 presents a topographic view of the project area.

The project is divided into four sections, as follows:

- Section A – Huntley Road to Randall Road
- Section B – Randall Road to east of IL 31
- Section C – East of IL 31 to east of IL 25
- Section D – East of IL 25 to IL 62

The project will be constructed in stages per section and will be let individually.

A Blanding’s turtle was identified by the Illinois Natural History Survey (INHS) in 2006 along Sleepy Hollow Road and Highmeadow Parkway south of the Corridor. A map generated from the Illinois Natural Heritage Database is attached depicting the area. In addition, relict shells of spike, slippershell, and purple wartyback mollusks were identified within the Fox River in the vicinity of the proposed Fox River Bridge crossing associated with a shoreline wading survey conducted during the summer in 2014.

Because of this, an Incidental Take Authorization (ITA) for the Blanding’s turtle, and state listed mussels in Kane County (spike, slippershell, purple wartyback, and black sandshell) is requested by the Kane County Division of Transportation (KCDOT), to

pursue the proposed Longmeadow Parkway project in Kane County. A plan to minimize impacts to the Blanding's turtle, spike, slippershell, purple wartyback, and black sandshell populations in the area is proposed under provisions of the ITA.

A) Description of the area to be affected:

Blanding's Turtle

The Blanding's turtle, a state-listed endangered species, (Illinois Endangered Species Protection Board 2015) was last observed at the southeast quadrant of the intersection of Sleepy Hollow Road and Highmeadow Parkway in Algonquin (INHS 2006). This area is an entirely developed residential area with several retention ponds. Habitat for the Blanding's turtle is not present. However, due to the mobility of the species and length of time since it was last observed, the Blanding's turtle could be potentially present throughout the Corridor in identified wetland communities and upland areas for nesting sites located. A total of 29 wetlands and waterways located in undeveloped areas, residential areas, agricultural land, and also along the Fox River were identified within the survey limits. A wetland location map depicting identified wetlands and waterways is attached.

Mussels

The Fox River Bridge crossing is located approximately 627 feet west of the intersection of Bolz Road and Sandbloom Road in Algonquin, Illinois. The center point of the river has coordinates in decimal degrees at 42.139625°N, -88.279922°W. The Township Range and Section for the project are T42N, R8E, S3, 3rd Principal Meridian.

The construction of the Fox River Bridge may require the use of a causeway to construct the pier and superstructure. One pier will be required within the Fox River channel for the construction of the Longmeadow Parkway Bridge over the Fox River. The proposed staging will limit the size of the temporary causeway to less than 1.0 acre during any construction stage. The causeway will be constructed of washed aggregate, with 1:1 slopes along the edges to reach from the river bottom to the top of the causeway. Only one-half of the river or less would be filled at any one time. The temporary causeway and cofferdams will be removed upon the completion of each stage of the bridge project and the stream will be restored.

The letting date is to be determined but is expected to occur between Spring 2016 and Spring 2018. The duration of work at the crossing is anticipated to be 18 months (or two construction seasons) with intermittent stream disturbances. There will be no in-stream work from April 1st to June 15th to avoid impacts to smallmouth bass spawning.

B) Biological data on affected species:

Blanding's Turtle, *Emydoidea blandingii*

The Blanding's turtle has a hinged plastron together with a bright yellow throat and chin (Ernst *et al.* 1994). Wild Blanding's turtles may live more than 77 years (Congdon *et al.* 2001) and females typically mature between 14 and 20 years, laying one clutch of eggs per year (Congdon *et al.* 1983). Blanding's turtles are active from late March through October in northern Illinois with females traveling up to a mile overland from their home ponds to nest (Congdon *et al.* 1983, Kuhns *et al.* 2007). The greatest trapping success occurs from May through mid-July (Benda *et al.* 2007).

Blanding's turtles occupy eutrophic habitats such as ponds marshes or small lakes with clear water and abundant aquatic vegetation, though adjacent uplands are needed for nesting habitat (Ernst *et al.* 1994). Blanding's turtles typically nest throughout the month of June. Nests of up to 19 eggs are laid in sand or sandy loam soils with good drainage and low canopy cover (Ross and Anderson 1990, Kuhns *et al.* 2007). The Blanding's turtle is considered endangered in Illinois (Illinois Endangered Species Protection Board 2015).

The INHS database contains 84 records for Blanding's turtle dating back to 1947 within 20 counties in Illinois.

Recent populations of Blanding's turtles were tallied from the INHS data, with records dated from the year 2000 through 2014 counted as recent populations. Recent reports of the Blanding's turtle are approximately 46 populations known based on Illinois county distributions. The Blanding's turtle appears to be in decline in much of its former Illinois range.

One record was tallied in Kane County from 2006.

In 1995, an amphibian and reptile survey was conducted in the Corridor by the INHS in the early stages as part of the Fox River Bridge Crossings Study (INHS 1995, Federal Highway Administration 2001). An intensive survey was conducted in areas of concern, specifically the west shore of the Fox River which is characterized by upland forest and pasture as well as northwest corner of the intersection of IL 25 and Bolz Road which contains upland forest. However, Blanding's turtles were not identified (Phillips 1995).

Mussels

Spike, *Elliptio dilatata*

The spike mussel is usually found in small to large rivers, but can also be found in the outlet habitats of lakes and reservoirs. The spike generally lives in sandy and muddy substrates, rarely in cobble. The spike shell's features are elongated, moderately thick, and have a brown to black coloration as its characteristics. Its shell can grow up to six inches but is more commonly four inches. This species requires a fish host to distribute their larvae and known hosts include: the gizzard shad (*Dorosoma cepedianum*), flathead catfish (*Pylodictis olivaris*), sauger (*Sander canadensis*), white crappie (*Pomoxis annularis*), black crappie (*Pomoxis nigromaculatus*), and the yellow perch (*Perca flavescens*) (Cummings & Mayer 1992).

The spike mussel is widespread throughout the Midwest and Eastern United States and it ranges as far west as Oklahoma, Nebraska, and South Dakota. Its population is relatively common throughout much of the Midwest but its Conservation Status is listed as imperiled in Illinois and is state threatened (Illinois Endangered Species Protection Board 2015). The spike in Illinois has rapidly declined in small and medium sized streams where it was once abundant.

The INHS database contains 551 records for spike dating back to 1878 within 72 counties.

Recent populations of spike were tallied from the INHS data, with records dated from the year 2000 through 2014 counted as recent populations. Recent reports of the spike are from 61 discrete river/stream systems with approximately 243 populations known based on Illinois county distributions within the 61 river/stream systems. The spike appears to be in decline in much of its former Illinois range.

Approximately 27 records were tallied from the Fox River in Kane County from 1914-2009.

Slippershell, *Alasmidonta viridis*

The slippershell is an Illinois threatened species (Illinois Endangered Species Protection Board 2015). The slippershell is thin-shelled mussel that grows to approximately 1.5 inches in length with a brown to tan periostracum (skin) that is often striped with thin bands of green. The slippershell inhabits small streams in sandy gravel to cobble substrates. The two known host fish for slippershell glochidia (larvae) are the mottled sculpin (*Cottus bairdi*) and the fantail darter (*Etheostoma flabellare*). Both host fish prefer cooler waters associated with groundwater seepage into the stream.

The INHS database contains 323 records for slippershell dating back to 1880 within 35 counties.

Recent populations of slippershell were tallied from the INHS data, with records dated from the year 2000 through 2014 counted as recent populations. Recent reports of the slippershell are from 97 discrete river/stream systems with approximately 193 populations known based on Illinois county distributions within the 97 river/stream systems. The slippershell appears to be in decline in much of its former Illinois range.

One record was tallied from the Fox River in Kane County. A year was not recorded.

Purple Wartyback, *Cyclonaias tuberculata*

The purple wartyback is found in medium to large rivers with large to medium gravel or mixed sand and gravel substrates. Cobble and boulders may be present in the substrate. The purple wartyback's distinguishing features include a rounded shell with a fairly prominent wing, numerous bumps (or warts), and a purple nacre, though white nacre is present in some populations. Known fish hosts for the purple wartyback include: the black bullhead (*Ameiurus melas*), yellow bullhead (*Ameiurus natalis*), flathead catfish (*Pylodictis olivaris*) and the channel catfish (*Ictalurus punctatus*), all of which are common and widespread fish in Illinois (Cummings & Mayer 1992, Badra 2004).

The purple wartyback is commonly found throughout most of the Midwest and Eastern United States and is found as far west as Oklahoma. Within Illinois, Michigan, Wisconsin, Iowa, and Minnesota the specie's conservation status is listed as imperiled. The purple wartyback is state threatened in Illinois (Illinois Endangered Species Board 2015).

The INHS database contains 281 records for purple wartyback dating back to 1881 within 37 counties.

Recent populations of purple wartyback were tallied from the INHS data, with records dated from the year 2000 through 2014 counted as recent populations. Recent reports of the black sandshell are from 18 discrete river/stream systems with approximately 109 populations known based on Illinois county distributions within the 18 river/stream systems. The purple wartyback appears to be in decline in much of its former Illinois range.

Approximately 10 records were tallied from the Fox River in Kane County from 1992-2001.

Black Sandshell, *Ligumia recta*

The black sandshell inhabits larger streams and rivers with hard bottoms such as, firm, compacted sand, sandy gravel, or gravel/cobble in fast flowing water. Despite its name the black sandshell is rarely found in readily shifting sands and is never found in silty conditions (Parmalee & Bogan 1998). The black sandshell

is a thick shelled, elongated mussel that is dark brown or black in maturity, though juvenile and young adults can show a pattern of green rays on a lighter colored shell surface. The black sandshell shows sexual dimorphism and can reach a length of approximately eight inches. (Cummings & Mayer 1992).

Native freshwater mussels require a fish host to distribute their larvae (glochidia). Black sandshells are bradytictic, or long term brooders. Females brood their glochidial larvae from August through the winter to the following July before they are released (Ortmann 1919). Host fish for the glochidia of the black sandshell include the bluegill (*Lepomis macrochirus*), largemouth bass (*Micropterus salmoides*), sauger, and white crappie (*Pomoxis annularis*) (Watters, 1994). Additionally, yellow perch, green sunfish (*Lepomis cyanellus*), rock bass (*Ambloplites rupestris*), and white perch (*Morone americana*) were identified as suitable hosts for *L. recta* by Steg, (1998). Saugers are considered by some to be a primary host fish for black sandshell (Khym and Layzer 2000).

Despite the relatively large number of host fish that carry larval black sandshell, the black sandshell appears to be declining throughout its Midwestern range. While exact causes of black sandshell decline are not reported in the literature, general declines or extirpations in mussel populations are attributed to habitat changes and water quality changes that can be linked to pollution from siltation, and urban runoff. (Downing *et al.* 2010). Recent findings that mussel glochidia are acutely sensitive to small ammonia spikes (USEPA 2009), indicate that ammonia runoff from lawns, turf grass, farms and perhaps wastewater treatment plant overflows during heavy rain events may contribute to a lack of recruitment for larval mussels.

The Illinois Natural History database contains 308 records of black sandshell occurrences within Illinois, dating back to 1878 in 52 counties.

Recent populations of black sandshell were tallied from the INHS data, with records dated from the year 2000 through 2014 counted as recent populations. Recent reports of the black sandshell are from 26 discrete river/stream systems with approximately 122 populations known based on Illinois county distributions within the 26 river/stream systems. The black sandshell appears to be in decline in much of its former Illinois range.

Approximately ten records were tallied from the Fox River in Kane County from 1914-2000.

Other Relevant Survey Data

Fox River Mussel Survey Results for West Dundee, IL, 2002, 2004 and 2007

In 2002, the Kane County Department of Environmental Management undertook a streambank stabilization project along the banks of the Fox River at South End Park in West Dundee, Illinois approximately 4.21 river miles downstream of the

Longmeadow Parkway project. The native mussel fauna was evaluated as that portion of the stream that might be affected by any sedimentation the project potentially could release. Therefore, mussels were moved from the near shore substrates farther off shore. Surveys were repeated in 2004 and 2007 to see if mussels had re-colonized the inshore area. The results are summarized in Figure 2.

One purple wartyback (threatened species) living specimen, was recovered in 2007, approximately three inches in diameter, and greater than 11 years old. One relic purple wartyback was found in 2004. Living purple wartybacks are rarely observed within the Fox River (Klocek 2007).

No living mussels less than four years old were encountered during the survey, suggesting that recent reproduction has not occurred.

Table 1

Native Mussels at South End Park, West Dundee, Illinois During 2002, 2004 and 2007

<i>common name</i>	<i>genus</i>	<i>species</i>	August-2002 <i>number</i>	August-2004 <i>number</i>	July-2007 <i>number</i>	
mucket	Actinonaias	ligamentina	relic	3	2	
elk toe	Alasmodonta	marginata	6	6	4	
threeridge	Amblema	plicata	relic	relic	relic	
purple wartyback	Cyclonaias	tuberculata	0	relic	1	Threatened
spike	Elliptio	dilatata	0	0	old relic	
Wabash pigtoe	Fusconaia	flava	0	2	3	
Plain pocketbook	Lampsilis	cardium	27	20	79	
fatmucket	Lampsilis	siliquoidea	0	0	old relic	
white heelsplitter	Lasmigona	complanata	5	4	1	
black sandshell	Ligumia	recta	2	4	7	Threatened
round pigtoe	Pleurobema	sintoxia	1	relic	26	
giant floater	Pyganodon	grandis	370	166	114	
pimpleback	Quadrula	pustulosa	53	48	152	
lilliput	Toxolasma	parvus	0	11	7	
paper pondshell	Utterbackia	imbecillis	10	9	2	
ellipse	Venustaconcha	ellipsiformis	relic	relic	old relic	
Total Live Specimens			474	273	398	
Total Live Species			8	10	12	
Person Hours			20	20	24	
CPUE			23.7	13.7	16.6	
Approximate Density per square meter			0.496	0.288	0.33	
Szafoni Index			13	14	14	
Szafoni Rating			Highly Valued	Highly Valued	Highly Valued	

A mussel survey was conducted at the Fox River Bridge crossing by Huff & Huff, Inc. on June 11, 2014. The survey was conducted approximately 231 feet upstream of Bolz Road and Sandbloom Road intersection. Seven hundred feet of searchable water depth was surveyed downstream. Shells of both recent and relict native mussel species including state threatened species were identified

immediately adjacent to the project footprint. Recent shells were defined as dead within five years (ligament attached) and relict is defined as dead greater than 5 years, with the periostracum eroded and no ligament present.

Table 2. Summary of Fox River Bridge crossing mussel survey June 11, 2014

	Common Name	Genus species	Number	Condition	
1	spike	<i>Elliptio dilatata</i>	18	1 recent	state threatened
2	round floater	<i>Pleurobema sintoxia</i>	6	1 recent	
3	ellipse	<i>Venustaconcha ellipsiformis</i>	2	relic	
4	slippershell	<i>Alasmodonta viridis</i>	1	relic	state threatened
5	threeridge	<i>Amblema plicata</i>	1	relic	
6	purple wartyback	<i>Cyclonaias tuberculata</i>	1	relic	state threatened
7	Wabash pigtoe	<i>Fusconaia flava</i>	1	relic	
8	plain pocketbook	<i>Lampsilis cardium</i>	1	relic	
9	creek heelsplitter	<i>Lasmigona compressa</i>	1	relic	
10	giant floater	<i>Pyganodon grandis</i>	1	recent	
11	pimpleback	<i>Quadrula pustulosa</i>	1	recent	

C) Description of activities that could result in the taking of a threatened or endangered species:

Blanding's Turtle

The Longmeadow Parkway project may directly and indirectly impact the Blanding's turtle in association with the following construction activities: trenching, boring, excavating, machinery operation, and vehicle traffic along Sleepy Hollow Road in spite of minimization efforts.

Indirect impacts would result from habitat fragmentation, creation of a barrier to movement, hydrology changes and vegetative community changes to adjacent wetlands and suitable habitat that support the listed turtles.

Mussels

Potential adverse effects to mussels could include improper removal or relocation, sediment burial, and physical destruction of living mussels due to construction activities.

Stream impacts could occur as a result of temporary stream impacts during construction, including cofferdams or causeways for dewatering. Permanent in-stream work will include pier construction.

D) Explanation of the anticipated adverse effects on the listed species:

Blanding's Turtle

It is anticipated that Blanding's turtles may be potentially directly and indirectly impacted as a result of the new alignment. Indirect impacts to suitable habitat for these species will occur. Indirect impacts would result from habitat fragmentation, creation of a barrier to movement, hydrology changes and

vegetative community changes to adjacent wetlands and suitable habitat that support the listed turtles.

Mussels

It is unknown at this time whether there will be adverse effects on listed species. Potential adverse effects to mussels could include improper removal or relocation, sediment burial, and physical destruction of living mussels due to construction activities.

Stream impacts could occur as a result of temporary stream impacts during construction, including cofferdams or causeways for dewatering. In-stream work could include the following: pier replacement/rehabilitation/addition, bridge abutments, and addition of scour protection.

2) Measures to minimize and mitigate impacts and funding available to undertake these measures

A) Plans to minimize affected area, estimated number of individuals that will be taken and amount of habitat affected:

Blanding's Turtle

KCDOT will commit to educate and inform construction crews and all on-site personnel about Blanding's turtles before work begins. Photos will be distributed of both adult and juvenile examples of the species and a discussion of the site management plan will take place during a training session and at the pre-construction site meeting which will detail the response procedure for encounters. The response plan is provided in Section 2B of this plan (below).

The Illinois Department of Natural Resources (IDNR) determined that the project area may contain the route to a nesting site. Therefore, potential harm to transiting turtles is a concern. The IDNR has recommended limiting work (when possible) to between late October and late March, when this species is hibernating, to prevent construction activities from crushing or injuring juvenile or adult turtles. If construction cannot occur within the recommended time frame, exclusionary fencing will be installed and in place from the end of March through October to prevent turtles from entering the construction areas. Daily inspections should occur for the first two weeks and then be maintained weekly throughout the construction period to ensure the exclusionary fencing has been properly installed (dug into the ground) and to check if any turtles are present on either side of the fence.

Before starting each work day, trenches and excavations should be routinely inspected to ensure no turtles (or other amphibians and reptiles) have become trapped within. Trenches should be covered at the end of each work day.

Mountable curbs will be used near the intersection of Sleepy Hollow Road and Highmeadow Parkway (where the Blanding's turtle was last observed in 2006) to

facilitate ease of movement for turtles, and to decrease the likelihood turtles will become trapped in a roadway.

KCDOT will commit to requiring contractor training sessions for all workers prior to the commencement of construction activities. These training sessions will alert all workers of the protocols to be outlined in Section 2B, to be taken if one of these turtle species is encountered during construction.

Since the area where the last observed Blanding's turtle was observed is highly developed and has not been observed since 2006, it is anticipated that no Blanding's turtles will be taken.

Mussels

The area of the in-stream work zone has been minimized to reduce the impact to the mussel habitat. The total project area within the Fox River is minimized so the amount of habitat affected is equal to the area required to complete the in-stream portion of the work.

Prior to construction or in-stream work, mussel surveys and relocations will be completed at the Fox River crossing where threatened or endangered mussels are within the project footprint and immediately adjacent to the footprint. All collected mussels will be relocated to suitable habitats outside of the influences of construction.

Where avoidance is not possible, the area of disturbance will be minimized. In addition, strict adherence to erosion and sediment control will be enforced. Erosion and sediment control will protect the waterway from receiving sediment runoff. Erosion control measures will adhere to those presented in IDOT's Bureau of Construction and Design and Environment Policy and Procedure Memorandum dated 2011. During construction, all solid waste will be disposed of properly off-site to avoid accidental deposition of material in habitat areas. The resident engineer will provide day-to-day enforcement of mitigation measures.

Specific stream impacts are not known at this time. Therefore, the number of individual mussels and amount of habitat to be taken is unknown.

B) Plans for management of the affected area that will enable continued use by the listed species:

Blanding's Turtles

Contractor training will occur to describe and illustrate Blanding's turtle, and the steps to take if any type of turtle is encountered on-site during construction.

If a turtle is encountered along the interior or exterior of the construction fence of the project site, crews will be informed to:

1. Immediately cease construction in the surrounding area and contact the Resident Engineer (RE).
2. The RE will arrive and capture the turtle and safely and humanely place it in ventilated cardboard box (or other suitable container of appropriate size) in a cool environment away from direct sunlight to prevent injury and immediately photograph the turtle if identification is in doubt.
3. The RE will notify the Corridor Manager and contact Dan Kirk, Heritage Biologist, (630-553-1372), and Jim Novak (708-557-4828) to report the encounter within 24-48 hours.
4. Any collected turtles will be released within one (1) hour of capture in similar or suitable habitat outside of urbanized sections of the Fox River.
5. Document the encounter by providing photos, a map, and GPS coordinates of relocated animals including the Blanding's turtle.
6. It will be emphasized that any person who violates any provision of this Act shall be guilty of a Class A Misdemeanor. Civil penalties also apply if violations of the Illinois Natural Areas Preservation Act occur.

All information as above and other pertinent details will be presented as a poster to be displayed at the construction site and provided as handouts to individual contractors.

Mussels

All work within the streams will be temporary except for the installation of one pier in the Fox River. After construction is completed, the causeway and cofferdams will be removed and the stream bottom will be restored to its approximate original condition and flow pattern, allowing for re-colonization of biota through natural geo-fluvial processes.

C) Description of all measures to be implemented to minimize or mitigate the effects of the proposed action on listed species:

Blanding's Turtles

Construction limits have been minimized to protect the maximum habitat for protected turtle species. A training program in recognition and disposition of the Blanding's turtle will be undertaken so contractors can recognize the species and use minimal time to detain the turtle, report the sighting and have a relocation of the specimen undertaken to remove the turtles from potential harm during construction activities.

Mussels

1. Implementation and maintenance of the soil, erosion, and sedimentation control plan will prevent sediment runoff from entering the river and streams.

2. Collection of all threatened and endangered mussels from proposed construction limits will be accomplished. All mussels will be individually planted in the proper position with siphons pointing in an appropriate direction (usually upstream but current dependent). Mussels will be hand dug into appropriate substrates similar to the substrates from which they were removed. Mussels must be hand buried to avoid having them use excess energy to rebury themselves, which could deplete the stored lipid reserves the mussels will use during the winter season. Threatened and endangered mussels will be located, aged, sexed, measured, and marked by GPS coordinates.

D) Plans for monitoring the effects of measures implemented:

Blanding's Turtles

A qualified environmental specialist will conduct post construction follow-up at locations adjacent to previous construction and at translocation areas to inspect for the presence of translocated and non-translocated protected turtle species. Approximately one (1) year and three (3) years after construction is complete, a report of findings will be provided to the IDNR.

Mussels

KCDOT plans to have mussel surveys conducted two (2) and four (4) years after construction is completed under the new bridge and at the relocation site. Reports will be forwarded to the IDNR and U.S. Fish and Wildlife Service (USFWS).

E) Adaptive management practices used to deal with changed or unforeseen circumstances affecting the effectiveness of measures instituted:

Blanding's Turtles

In the event that protected turtles are encountered during construction, the procedure of detaining, reporting and relocating turtles will be followed. Turtles may appear at any time except during the winter.

Mussels

1. Siltation during all phases of construction will be minimized through use of erosion control devices such as silt fences to prevent sediment runoff from entering the river and affecting threatened or endangered mussel habitat. A designated inspector will inspect and maintain silt fences/erosion structures and repair or rebuild them as needed by whatever means needed to adapt to specific conditions.
2. Threatened and endangered mussels from the project will be relocated to an appropriate location outside of the influence of project area using approved methods for handling mussels with minimal stress. Relocated mussel habitat may be monitored to insure that environmental conditions remain stable by whatever means necessary. Mussel relocation is dependent on the flow and

volume of water in the river at that time. If the flow is swift and/or the water levels are high the relocation will not take place. Mussel relocation will occur only when water levels are low and current conditions are moderate or low.

Potential mussel relocation beds will be carefully screened to assure that habitat is suitable for transplanted mussels and that risks of external threats to the relocation beds (siltation, chemical spills) are minimized. The relocation will be done according to accepted standards to minimize mussel mortality.

F) Verification of adequate funding to support and implement all activities described in the conservation plan:

KCDOT will provide for any mitigation and follow up studies required for this project. Adequate funding for mitigation and monitoring will be available to meet the goals of this conservation plan.

G) Description of alternative actions the applicant considered that would not result in take and reasons these courses of action were not selected:

The Longmeadow Parkway project, previously referred to as the “Bolz Road Corridor”, was studied in detail as part of the Fox River Bridges Final Environmental Impact Statement and Record of Decision in 2002. Originally over 20 bridge crossings were proposed to alleviate traffic concerns due to rapid growth in Kane County. Of these, nine crossings were determined to be effective to address regional traffic needs and were further evaluated. Of the nine crossings, five crossings passed the fatal flaw screening:

- **Bolz Road Corridor**
- **Chicago Central & Pacific (CC&P)/Stearns Road Corridor**
- **Illinois Route 56/Oak Street Corridor**
- Red Gate Road Corridor¹
- Chicago & Northwestern/Dean Street Corridor

An alternatives analysis was proposed for each of the five crossings that included (1) a no-build alternative which was defined as normal road maintenance, minor improvements to existing infrastructure, and the execution of previously committed minor projects, (2) a congestion-management system (CMS) alternative which was intended to primarily address regional traffic concerns by reducing the numbers of vehicles on the roadway system (the CMS alternative was later determined to not adequately address the purpose and need for the project and was discarded), and (3) a build alternative. The Record of Decision (2002) resulted in build approval of three crossings (bolded): the Bolz Road Corridor, the Chicago Central & Pacific (CC&P)/Stearns Road Corridor, and the Illinois Route 56/Oak Street

¹ The Red Gate Road Corridor was later locally picked up and constructed in 2013.

Corridor. The Bolz Corridor was largely approved because it is cost effective, has a habitat saving design that minimizes in-stream habitat loss, and at the time avoided impacts to threatened and endangered species.

For the Longmeadow Parkway project (Bolz Road Corridor, “Corridor”), several methods of bridge construction were considered during the design phase.

2A) Construction of the proposed bridge from the banks.

2B) Two bridge spans (one spanning the Fox River and one spanning Sandbloom Road) and mechanically stabilized earth (MSE) retaining walls to support Longmeadow Parkway over the Fox River. This option includes the installation of one pier in the channel (near the west bank) and allows for each span to be constructed each construction season.

2C) One bridge span with MSE retaining walls with piers proposed along the banks of the Fox River (not within).

Option 2A was disregarded as the river is too wide for the approach to be feasible. Option 2C was disregarded as the required elevation of the bridge and approach would not meet roadway and bridge standards. Therefore, Option 2B was selected.

H) Data to indicate that the proposed taking will not reduce the likelihood of the survival of the listed species, in the wild, within Illinois:

Blanding’s Turtle

It is unknown whether the Blanding’s turtle is currently present within the Corridor. However it may be present as it was observed in 2006 occurrence south of the Corridor. Given that this species is active on land and is wide ranging, the species may be present. If encountered, immediate capture, identification, reporting and relocation within the day of capture will ensure that the turtles are not harmed during construction activities. Therefore the likelihood that the species will be reduced in association with the project is not anticipated.

Mussels

Suitable habitat exists for mussels both upstream and downstream of the Fox River Bridge crossing. Prior to work being completed in the streams listed with potential threatened or endangered mussel species, a mussel survey will be conducted. If threatened or endangered mussels are found, they will be moved to an appropriate location out of the project limits. These surveys and translocations will be coordinated with the IDNR. Therefore the likelihood that the species will be reduced in association with the project is not anticipated.

I) Implementing agreement which includes:

A) Names and Signatures of all participants:

KCDOT is the owner of this project.

B) Obligations and Responsibilities of each participant with schedules and deadlines for completion and preparation of progress reports submitted to the department:

KCDOT is responsible for activities related to the development of the Longmeadow Parkway project in Kane County, Illinois.

Construction is scheduled to begin in fall 2015 and is anticipated to be completed in 2018. Each of the four contract sections will be let separately with a portion of Section A to be completed first.

Once the project is completed, a summary report will be submitted to the IDNR summarizing all activities that occurred prior to the commencement of monitoring.

C) Certification that each participant has the legal authority to carry out their respective obligations and responsibilities:

See Final Clause of this Plan.

D) Assurance of compliance with all other federal, state, and local regulations pertinent to the proposed action and to the execution of the conservation plan:

Coordination with the U.S. Army Corps of Engineers (COE), Chicago District and Illinois Environmental Protection Agency (IEPA) is ongoing for the project, but not specifically for the protected turtle and mussel species. Coordination has occurred with the following agencies:

- U.S. Army Corps of Engineers
- U.S. Fish & Wildlife Service
- U.S. Environmental Protection Agency
- Illinois Environmental Protection Agency
- Illinois Department of Natural Resources
- Illinois Historic Preservation Agency
- Illinois Department of Transportation

E) Copies of any federal authorizations of a taking already issued:

Coordination with the COE, Chicago District and the IEPA is ongoing. Copies of the applicable permits have will be forwarded to the IDNR as part of the Individual Permit (IP) process.

3) Attachments

Location Map(s)

Wetland Location Map(s)

INHS Natural Heritage Database Map

Certification:

KCDOT certifies that their agency has the authority to complete the project and to address the issues proposed in the Conservation Plan in the event that federal or state listed threatened or endangered species are encountered. KCDOT will assure that all applicable laws will be adhered to during the completion of the project.

Name:
Carl Schoedel, P.E.
County Engineer

Date:

Works Consulted

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