2018 QBS Request for Statement of Interest (SOI) Kane County Division of Transportation (KDOT) 2019 Structure Inspections Section Number 18-00501-00-EG

The Kane County Division of Transportation is in need of professional services from a qualified engineering firm to provide engineering services as detailed in the following preliminary Scope of Services.

At this time the County anticipates starting this work in early 2019 with approximately 12 months to complete the work. At the County's discretion, the County may pursue a 2020 inspection contract and a 2021 inspection contract, without again advertising the work.

The Statement of Interest document shall be limited to 2 pages, and shall be submitted electronically via **KDOT QBS** no later than 4:30 pm on September 6, 2018 and should be addressed to Nils Jordahl, Project Manager.

If you plan to enter into a joint venture with another firm for this project please note this on your Statement of Interest, including the name of the firm you are entering into a joint venture with for this project.

The short-listed firms will be posted on our Consultant Selection Summary Table website at: <u>http://www.co.kane.il.us/dot/SOISummary.aspx</u>.

Any new firm interested in doing work with the Kane County in regards to this or any other future professional service, must first submit their Prequalification Document to Kane County by following the instructions found at http://kdot.countyofkane.org/Shared%20Documents/QBS.pdf. The prequalification document will be reviewed and approved in a short period of time if the appropriate documentation was provided. Each firm must first be prequalified in order to electronically submit their Statement of Interest via KDOT QBS no later than the deadline described above. More information regarding the Qualifications Based Selection process may be found at http://www.co.kane.il.us/dot/consultant.aspx.

Firms interested in providing services to Kane County are hereby notified of the Kane County's Ethics Ordinance No. 10-206, in particular, Section 10, page 15. Only the firm that is ultimately selected for these professional services will be required to provide the Ethics Ordinance information directly to the Kane County Division of Transportation as part of the consultant services agreement. The complete Ethics Ordinance No. 10-206 document is available online at the following link <u>Kane County Ethics Ordinance</u>.

A Statement of Interest (SOI) received after the above noted deadline will not be used as part of our consultant selection process.

SCOPE OF SERVICES 2019 STRUCTURE SAFETY INSPECTIONS

A. History

The Surface Transportation Assistance Act of 1978 required that all public bridges over 20 feet in length be inspected and inventoried in accordance with the National Bridge Inspection Standards by December 31, 1980. In October 1988, NBIS was modified to require special inspections of fracture critical details and underwater features of bridges. Since then, changes have been made to address increased focus on scour inspections, timely reporting, inspector qualification, and other issues. Qualified personnel must typically inspect bridges every two years and have their findings submitted to IDOT for inclusion in the state and national databases.

B. Qualifications

The Consultant shall provide personnel qualified to perform the bridge inspections. An IDOT approved Team Leader shall supervise, in the field, all inspection activities.

The Consultant shall also supply an IDOT approved staff member to act as Program Manager for all inspections.

The Consultant shall also provide, on an as needed basis, a structural engineer registered in the State of Illinois to review any bridges that are determined to be structurally deficient.

The Consultant shall provide the specific qualifications for all members on the inspection team, and for all related work.

C. Scope of Work

The Consultant shall inspect all structures necessary, in accordance with the National Bridge Inspection Standards and the Illinois Department of Transportation Structure Information and Procedure Manual.

The Consultant shall submit the following to the County for each major structure inspected:

- 1) IDOT Bridge Inspection Report, BBS-BIR
- 2) Bridge Inspection Report Narrative (in a format similar to the attached).

In addition, the consultant will provide a detailed cost estimate for each structure that includes the costs of all repairs mentioned in the report's recommendations.

The Consultant shall also provide a plan of access for inspection of the bridges. It is anticipated that specialized equipment such as snooper trucks or rigging may be required to allow the inspector to closely

examine the bridge elements. Generally, the inspector must be within arm's reach to adequately inspect a bridge element. Safety of the inspector is of extreme importance to the County and is the responsibility of the Consultant. The Consultant is solely responsible for the safety practices and methods used to perform any and all inspections.

The consultant shall coordinate all inspections over railroads with the appropriate railroad. Any costs for licenses and permits shall be included in this work.

The consultant will provide on-call design services, review services, and inspection services to be billed against the on-call item in the contract.

The consultant will assist in any other needs as defined by the county to comply with the requirements of NBIS.

D. Schedule

The notice to proceed will be issued on approximately January 1, 2019.

The project shall be complete by December 31, 2019.

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Bridge Inspection Report



STRUCTURE NO. 045-3166

Stearns Road / FAP 361 over Fox River

Prepared For

Kane County Division of Transportation

INSPECTION TYPE: In-Depth NBI DATE: May 11, 2016



Hampton, Lenzini and Renwick, Inc. Civil Engineering • Structural Engineering • Environmental Services • Land Surveying www.hlrengineering.com

ADMINISTRATIVE DATA:

Region / District:	1/1
County:	Kane
Township:	Virgil
Feature Carried:	Stearns Road / F.A.P. 361
Feature Crossed:	Fox River

Weather: 70° F; Cloudy

I. <u>ROADWAY/STRUCTURE DATA:</u>

6750 (2014 – IDOT Master Structure Report)
743 – 11% (2014 – IDOT Master Structure Report)
1.00 (36) (IDOT Master Structure Report); 0.89 (HLR-2011)
1.36 (48) (IDOT Master Structure Report); 1.32 (HLR-2011)
60' – 0"
56' – 0"
56' – 0"

CONSTRUCTION / RECONSTRUCTION / REPAIR HISTORY:

Year Constructed:	2010 (HL - 93 Loading)
Year/s Reconstructed:	NA

STRUCTURE DESCRIPTION:

Туре:	Continuous, Composite Cast-in-Place Concrete Deck on 55"
	(Span 1) & 76" (Spans 2 – 5) Steel Plate Girders
Span Arrangement:	Five Spans (180' – 0", 3 @ 210' – 0", 163' – 5")
Length & Width:	908' - 5" back-to-back of abutments; 63' - 0" out-out deck; 56'
_	- 0" face-to-face curbs
Abutments:	Pile Supported Stub Abutments
Piers:	Reinforced Concrete Cap & Columns on HP Pile Supported
	Footings (Piers 1 & 4) & and Drilled Shafts (Piers 2 & 3)
Skew:	0°

II. INSPECTION HISTORY (NBIS RATINGS):

<u>Year</u> :	Deck:	Super:	<u>Sub</u> :
2010	8	9	9
2012	8	8	8
2016	7	8	8

III. STRUCTURE CONDITION FINDINGS:

Approach Roadway: (See Photo Nos. 50 - 51 & 62 - 63).

The concrete approach roadway is in good condition.

• Diagonal cracking up to 1/8" with dispersed transverse and longitudinal cracking throughout in the approach slabs

Bridge Rail / Curb: (See Photo Nos. 55 & 56).

The curb mounted steel bridge rails are in good condition.

- Localized areas of paint coating failure with light surface corrosion over the full length of the railing.
- Vertical hairline cracks along the concrete curbs at ± 3' intervals.
- There is a 2' x 1' impending spall on the southeast approach slab curb.

DECK

The concrete bridge deck is **good condition**.

Joints: (See Photo Nos. 52 – 53 & 64 – 65).

- The modular expansion joints at the east and west abutments are in good condition.
 - There is a light buildup of debris at both expansion joints.
 - The east and west abutment joints measured 8.5" and 5", respectively, at 70°F.

Top of Deck: (See Photo Nos. 57 - 61).

- There are full depth transverse cracks < 0.06" at 5' intervals over the majority of the deck.
- There are hairline vertical cracks on both side of the concrete median.
- Several of the deck drains have accumulated a minor amount of debris but seem to still be operational.

Bottom of Deck: (See Photo Nos. 2, 8, 17 – 19, 26, 35, & 39).

• The full depth transverse cracks at 5' intervals are leaching underside of the deck.

SUPERSTRUCTURE

Overall the welded plate girders and diaphragms are in very good condition.

Beams / Diaphragms: (See Photo Nos. 3, 5, 7, 13 – 14, 31 – 32, 36, 38, 40, 44, & 68).

- There are a few small localized areas of paint coating flaking on the interior girders with no surface corrosion of note.
- The deck drainage system collects in two 12" diameter pipes suspended in the north and south superstructure bays.
- The south drain is leaking approximately 10 feet from the east abutment. (See Photo No. 72).

Bearings:

All bearings are in good condition.

- Type II elastomeric expansion bearings at the west abutment. (See Photo Nos. 45 & 46).
- Type III elastomeric expansion bearings at the east abutment. (See Photo No. 69).
- Fixed bearings at Pier 2. (See Photo No. 2).
- Guided expansion bearings at Piers 1, 3, & 4. (See Photo Nos. 6, 12, 34,

SUBSTRUCTURE

Overall the substructure is in very good condition.

Abutments / Wingwalls: (See Photo Nos. 1, 41 – 42, 44, 47, & 71).

• The concrete abutments / wingwalls are smooth with no noticeable defects.

Piers: (See Photo Nos. 4, 10, 22, & 27).

• The concrete piers with a decorative form liner have no noticeable defects.

SLOPE / CHANNEL (See Photo Nos. 11 & 15).

The channel is in very good condition.

Protection: (See Photo Nos. 1 & 41 - 42).

- Riprap is in place at both abutment embankment slopes and is in stable condition.
- The channel is stable with no noticeable scour.

TRAFFIC SAFETY:

Pavement lane markings are present and mostly visible

Guardrail: (See Photo Nos. 49 & 66).

Overall the structure's guardrail is in good condition.

- The steel plate approach guardrail, transition sections and terminal sections meet current IDOT policy.
- The steel plate bridge rail exhibits localized paint coating failure with light surface corrosion.

Signage:

• Chevron warning signs mark all guardrail ends.

UTILITIES: (See Photo Nos. 2 & 8).

- The 1st and 2nd bays from each fascia carry conduits for the full length of the structure.
- Electrical utility lines parallel the railroad track beneath span 1. (See Photo No. 28).
- Decorative light standards are mounted north and south of the bridge on the parapet fascia above each pier location.

STRUCTURE RATING:

The structure rating is INV - 089, OPR - 1.32.

IV. CONCLUSIONS AND RECOMMENDATIONS:

CONCLUSIONS:

• The transverse cracking in the deck should be sealed to limit the infiltration of water and deicing salts through the bridge deck.

RECOMMENDATIONS:

Short Term (1 to 3 years):

- Clean out the debris in deck drains for proper drainage.
- Clean debris from expansion joints.
- Fix drain pipe connection in Bay 6 to stop leakage

Long Term (4 to 12 years):

• Seal cracks in the bridge deck.

ATTACHMENTS:

Attachment A.	Routine Inspection Report
Attachment B.	IDOT Master Structure Report
Attachment C.	Structure Sketches
Attachment D.	Structure Photos
Attachment E.	Cost Estimate
Attachment F.	Structure Rating

Illinois Department of Transportation

Routine Inspection Report

SN: 045-3166 Distri	ct:	1 Spans:	5	Appr	. Spans:	0	Skev	v: 0	ADT:	675	T 0	ruck P	ct:	11
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Facility Carried: STEARNS F	RD				Featur	e Cro	ossed:	FOX RIV	ER	-				
Location 0.6 MI W OF IL25			Munio	cipalit	y:			Tean	n/Sub: /		Ir	nsp/Rte	:	
Bridge Name:				Materi	al & Type	: STI	EEL CO	ONTINUOU	S/STRIN	GER/M	ULŤI-	BEAM/	GIRE	DER
Insp. Intervals Routine: 2	4 F I	racture Criti	cal:	0	Underwa	ter:	0	Special:	N/A		Elem	ent Lev	el:	24
90 - Inspection Date: 5/1	1/2016	6 90C -	- Tem	p. (⁰F)	70	90	B1 – In-	-Depth		X]			
Is Delinquent: Reason:													5	
90A – Agency Program Manag	ger:				904	13 - 0	Consulta	ant Program	n Manag	er: H	LR - M	A. CIMA	1	
90A1 - Team Leader: HLR	- A. CH	ARLESWO	RTH	90	A2 – Insp	ector	HLR	R - M. BRIN	K					
90B – Inspection Remarks:														
Previous Inspection														
					Resource	es								
Time to Inspect (H:M): 8:0	6:0	Traffic Co	ntrol:	2	2 Boa	at:	N	Waders:	N	Snoo	per:	S S	S	
Ladder: N Manlift:	N	Bucket Truc	k:	N	Other:									
				Inspe	ctor's Ap	prais	als							
58 – Deck Condition:	Prev 8	7					awa ana ka		Comments	3				
59 – Superstructure Cond:	8	8												
60 – Substructure Cond:	8	8												
62 – Culvert Condition:	Ν	N												
61 – Channel Condition:	9	9											1	
71 – Waterway Adequacy:	9	9									142			
72 – Approach Rdw Align:	8	8												
111 – Pier Navig Protection:	N	N												
			9	0B – II	nspector	Rema	arks:							
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WITH LEACHING UNDERSIE	DE OF	THE DECK.	j.											
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BBS-BIR (Rev. 03/04/14) Sheet 1 of 2

									Ro	utine Inspec	tion F	Report
									Str	ucture Number	: (453166
				Additic	nal In	spection	Data					
		Prev	New									
36A – Bridge Railing A	dequacy:	3	3	Rail Types:	CUR	B MOUN	TED STEEL	RAIL	Nou			New
Approach Guardra	il Adequacy	/: 36B	– Tra	ansitions: 3	3	36C -	Guardrail:	3	3	36D - Ends:		3 3
		Prev	New	7								
108A – Wearing Surfa	ce Type:	A	A	If "L-Other" D	escrib	ə:	N. 1997					
108B – Type of Membr	ane:	F	F	If "E-Other" D	escrib	e:						
108C - Deck Protectio	n:	A	A	If "I-Other" De	escribe	:	where .					
108D - Total Deck Thi	ckness (in):	8.0	8.0)								
59A - Paint Date (Mo/	(r)·	09/200	Prev N	New								
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59B – Paint Type:		5		S		Color:	Fascia	:	In	ter:	Railing	·
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weight Limit Posting:	70C2 - Cc	ombina	ation ⁻	Type 3S-2 (5 o	r more	axles):		Tons				
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Inspection Team Lead Consultant Program I	ler: Nanager:			And	The	Signa	ture				515	Date 5 1 ¹ 201 2 31 ¹ 2016

BBS-BIR (Rev. 03/04/14) Sheet 2 of 2

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				Kind	ute Under Data ***	opecial oystems.	Special Statemer	Designated Truck Rte:	Future AADT Yr/Cnt-	Bypass Length:	One Or Two Way:	Number Of Lanes:	Est Truck Percentage:	Curr AADT Yr/Count:	Inventory Direction:	Natl. Hwy System:	Linked:	Segment:	Station:	te Under Data	RR Vertical Underclear:	RR Lateral Underclear:	Crossing 1 Nbr:	Crossing 1 Nbr:	011 ***Railroad Crossing	Rate Method: D ASSIGNE	55 Culvert Cell Widt	2,662,000 Culvert Cell Heig	ility: 0 Culvert Opening	Number Culvert (Culvert Fill Depth	No Navigation Vert Cle	No Navigation Horiz Cl	Navigation Control	Sidewalk Width Left:	Sidewalk Width Righ	None Deck Width:	7/05/2012 Appr Roadway Width	Bridge Roadway Wid	Length of Long Spar	No AASHTO Bridge Leng	100.0 Structure Length:	I		
Attachment B				Number					<u> </u>				%	/							20 Ft 00 In	35.1			y Info***	D RATING BAS	t h: 0.00	ht: 0.00	Area: 0.0	Cells: 0	h : 0.0	9 ar: 0	lear: 0	0 No	0.0	1t: 0.0	63.0	h: 56.0	3th: 56.0	n: 210.0	gth: 99.9	980.4			

Date: 10/24/2016 Page 1

Illinois Department of Transportation Structures Information Management System Master Structure Report (S-107)

Attachment B						Kellidi KS.	
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	Bridge Cost:					Type of Work:	
ollars ***	*** Costs in D			Length:	e Year:	Cost Estimate	
		vement	Proposed Impro				
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						HPP 152701500	Fed Aid Pr #:
						63075	Contract Nbr:
						06-00214-20-BR	Section Nbr:
			Sta:		Sta: 571+42.96	FAP0361	Route:
			structed	Recon		2010 Original	Year:
				formation	Construction Inf		
lecorded: No	Microfilm Data F	BBEL	sis By: JOC/CI	Analy		: 01/09/2008	Analysis Date
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E CRACKS <0.06" @ 5'	5" DECK - TRANSVERS	t Openings: W ABUT5", E ABUT8.	Joint		Meets Standards	J Appraisal: 3	Bridge Railing
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S SHP ZINC&FLD ACRYL		k Membrane: F NONE	ITERIA Decl	ENT DESIRABLE CR	EQUAL TO PRESE	aluation: 8	Structural Eva
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tion Type 3S-1: Tons	ER Combina	ties Attached: 5 STORM WAT	LEMS NOTED Utili	DITION - NO PROB	VERY GOOD CON	e : 8	Superstructur
nit Vehicles: Tons	Single U	by (Name): HLR - M. BRINK	ROBLEMS Insp	V - SOME MINOR PH	GOOD CONDITION	7	Deck:
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		Information	Inspection/Appraisal	a	[[
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				 →	66 District	umber: 045-316	Structure N
- - 		Report (S-107)	Master Structure				
Parre 2		Management System	uctures Information	St			
Date: 10/24/2016		of Transportation	Illinois Department				



SHEET LOF II

— Crack











LEGE	END
CO = Crack - Open CC = Crack - Closed CO = Corrosion	SS = Shear Stirrup EF = Efflorescence WL = Water Leakage
DL = Delamination LE = Leaching DD = Duagand Drain Hala	Delamination
PS = Prestressed Strand PS = Prestressed Strand PE = Beinforcement	Spalls
RP = Repair RS = Rust Staining	Repairs
SP = Spall	Crack

Note: Damage area dimensions are shown as length along beam x width across beam.

Spalls

Repairs

Crack

(East Side)

<u>END VIEW</u> (South Side)

KANE COU	NTY DIVISION OF TRANS	PORTATION									
STE	ARNS ROAD / F.A.P. 361 FOX RIVER KANE COUNTY STRUCTURE NUMBER: 045-31 PIER 4	OVER 66									
DRAWN BY: RDH HAMPTON, LENZINI AND RENWICK, INC. DATE: CHECKED BY: AMC STORMON OWNE, SUITE 201 SPENDERIDA LUNIONS STORMON OWNE 2010 MAY II, 2016											
PROJECT NO. 16.0118	ILLINOIS PROFESSIONAL DESIGN FIRM	SHEET II OF II									

Photo 1 - East Abutment

Photo 2 - Underside of Deck - North Bay of Span 5

Photo 3 - Span 5 - Typical Diaphragm

Photo 4 - East Face of Pier 4

Kane County SN 045-3166 May 2016

Photo 5 - Span 5 - Splice

Photo 6 - Typical Bearing at Pier 4

Photo 7 - North Fascia Looking Southwest

Photo 8 - Underside of Deck in Span 4

Photo 9 - Pedestrian Bridge Looking Southwest

Photo 10 - East Face of Pier 3

Kane County SN 045-3166 May 2016

Photo 11 - Looking South Downstream from Bridge

Photo 12 - Bearings at Pier 3

Photo 13 - Typical Beam Span 4

Photo 14 - Span 4 - Paint Failure on Splice Plate

Photo 15 - Looking North Upstream from Bridge

Photo 16 - Outside of North Parapet and Lightpole Looking Southeast

Photo 17 - Underside of Deck in Span 3 Looking West

Photo 18 - Underside of Deck in Span 3

Photo 19 - Underside of Deck - Span 3 Bay 3 Rust Staining

Photo 20 - Bearing at Pier 2 Beam 4

Photo 21 - Pedestrian Bridge Looking East From Pier 2

Photo 22 - Pier 2 Column at the Waterline

Photo 23 - Span 2 Looking West

Photo 24 - Pedestrian Bridge Looking East From Pier 2

Photo 25 - Looking Southeast Across Bridge

Photo 26 - Underside of Deck in Span 2

Photo 27 - East Face of Pier 1

Photo 28 - Track and Power Lines in Span 2

Photo 29 - Looking North along West Shoreline

Photo 30 - Looking South Along West Shoreline

Photo 31 - Span 1 - Change in Beam Section

Photo 32 - Span 2 - Change in Flange near Pier 1

Photo 33 - Span 2 From Pier 1

Photo 34 - Bearing at Pier 1

Photo 35 - Underside Deck in Span 2 from Pier 1

Photo 36 - North Fascia - Span 1

Photo 37 - Railroad Construction in Progress Under Span 1 Looking Southwest

Photo 38 - Span 1 - Beam 4 at Change of Section

Photo 39 - Underside of Span 1 from Pier 1

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Photo 40 - Splice Plate of Beam 4

Photo 41 - Slopewall and Retaining Wall of the West Abutment

Photo 42 - Slopewall and Retaining Wall of the West Abutment

Photo 43 - Drain Detail Underside Bay 1 - Span 1

Photo 44 - Diaphragm and West Abutment Backwall - Bay 1

Photo 45 - Bearing at the West Abutment

Photo 46 - Bearing at the West Abutment

Photo 47 - West Abutment

Photo 48 - Underside Joint at West Abutment Looking South

Photo 49 - Looking East Across Bridge

Photo 50 - West Approach - Westbound Lanes

Photo 51 - West Approach - Eastbound Lanes

Photo 52 - Deck Joint at West Abutment in the Eastbound Lane Looking South

Photo 53 - Deck Joint at West Abutment in the Westbound Lane Looking South

Photo 54 - Name Plate

Photo 55 - Light Pole - North Rail near West Abutment Looking Northeast

Photo 56 - Light Pole - North Rail near West Abutment Looking North

Photo 57 - Top of Deck Drain - Span 1

Photo 58 - Top of Deck - Typical Cracking in Span 1

Photo 59 - Top of Deck - Typical Cracking in Span 2

Photo 60 - Top of Deck - Typical Cracking in Span 3

Photo 61 - Top of Deck in Span 4 - Cold Joint

Photo 62 - East Approach - Westbound Lanes

Photo 63 - East Approach - Eastbound Lanes

Photo 64 - Deck Joint at East Abutment in the Eastbound Lane Looking South

Photo 65 - Deck Joint at East Abutment in the Westbound Lane Looking South

Photo 66 - Looking West Across Bridge From East Approach

Photo 67 - North Elevation Looking Southwest

Photo 68 - 3rd Bay Diaghram at the East Abutment

Photo 69 - Bearing at East Abutment

Photo 70 - Underside Joint at East Abutment

Photo 71 - East Abutment Cap and Backwall

Photo 72 - Drainage Pipe in Bay 6 near the East Abutment

Photo 73 - South Elevation Looking Northwest

KANE COUNTY DIVISION OF TRANSPORTATION

ST CHARLESTOWNSHIP STRUCTURE 045-3166 STEARNS ROAD OVER FOX RIVER

ESTIMATE OF COST - SHORT TERM RECOMENDATIONS

55'-5" bk-bk abutments; 27'-0" face-to-face rails

ITEM NO.	ITEMS	UNIT	QUANTITY	ا P	JNIT RICE	TOTAL
1.	Concrete Sealer	Sq Ft	61803	\$	2.00	\$ 123,606
2.	Traffic Control and Protection	L Sum	1	\$ 20	0,000.00	\$ 20,000
3.	Mobilization	L Sum	1	\$ 10	0,000.00	\$ 10,000
	SUBTOTAL					\$143,606
	10% CONTINGENCY					 \$14,361
	TOTAL ESTIMATE OF COST					\$157,967
	Mada by: AMC 6/14/2016					

Made by: AMC 6/14/2016 Checked by: SWM 9/15/2016

Rating Summary - SN: 045-3166 DWT 01/05/2011

	+S1	+S2	+S3	+S4	+S5	-P1	-P2	-P3	-P4	Shear	Controlling	
HS 20 - INV	0.890	1.575	1.488	1.592	1.505	1.467	1.324	1.274	1.487	1.467	0.890	RF
HS 20 - OPR	1.486	2.630	2.484	2.657	2.512	2.448	2.210	2.127	2.483	1.324	1.324	RF
KC1 - OPR	0.855	1.430	1.402	1.442	1.451	2.108	2.410	2.400	2.342	0.000	0.000	RF
	145,200	243,000	238,200	245,100	246,500	358,200	409,700	407,900	398,100	0	0	Pounds
KC2 - OPR	0.862	1.444	1.415	1.456	1.463	2.157	2.470	2.459	2.399	0.000	0.000	RF
	142,300	238,200	233,400	240,100	241,400	355,900	407,500	405,700	395,800	0	0	Pounds
KC3 - OPR	0.946	1.592	1.558	1.602	1.599	2.481	2.851	2.834	2.765	0.000	0.000	RF
	132,300	222,900	218,100	224,200	223,800	347,300	399,100	396,700	387,100	0	0	Pounds
KC4 - OPR	1.130	1.902	1.860	1.912	1.913	3.009	3.459	3.439	3.356	0.000	0.000	RF
	129,900	218,700	213,900	219,900	219,900	346,000	397,700	395,400	385,800	0	0	Pounds
Single Unit	2.612	4.419	4.305	4.421	4.406	7.685	8.857	8.797	8.583	0.000	0.000	RF
	57.4	97.2	94.7	97.2	96.9	169.0	194.8	193.5	188.8	0.0	0.0	Tons
-												
Combinations	2.046	3.449	3.364	3.456	3.461	5.816	6.699	6.654	6.492	0.000	0.000	RF
3 or 4 Axles	59.8	100.8	98.3	101.0	101.2	170.1	195.9	194.6	189.8	0.0	0.0	Tons
-												
Combinations	1.690	2.851	2.783	2.859	2.855	4.415	5.058	5.031	4.909	0.000	0.000	RF
5 or More Axles	60.8	102.6	100.1	102.9	102.7	158.9	182.0	181.1	176.7	0.0	0.0	Tons
80k Legal Load	1.720	2.886	2.825	2.906	2.913	4.415	5.058	5.031	4.909	0.000	0.000	RF
	68.8	115.4	113.0	116.2	116.5	158.9	182.1	181.1	176.7	0.0	0.0	Tons

ating Sheet - LFR					
POSITIVE MOMENT - SPA	<u>N 1</u>			Job Number	16.0118
pads:				County	Kane
D = Dead Load	3067.28	k-ft		Structure No.	045-3166
D = Dead Load (shear)	95.83	k		Ву	DWT
				Date	01/05/2011
apacity:					
C = Capacity	9048.85	k-ft	_		
C = Capacity (shear)	579.28	k	@ W Abut		
ive Load Eactors:					
Impact	1 164	1			
DE	0.864	Per Lane o	r Avlo		
ы	0.004		ANC		
RF = (C - A1*D)					
(A2*I*(1+I))					
A1 =	13	1	1		
A2 =	2 17	Inventory	1		
Δ2 -	1 2	Operating			
/ \L -	1.0	operating	1		
ating Factor					
HS 20 Loading	2606.33	k-ft	Per Lane or Ax	le	
HS 20 Loading (shear)	76.21	k	Per Lane or Ax	le	
• • • •					
	С	D	L	RF	Tons
M Inventory	9048.85	3067.28	2619.92	0.890	17.81
M Operating	9048.85	3067.28	2619.92	1.486	29.72
V Inventory	579.28	95.83	76.61	2.735	54.70
V Operating	579.28	95.83	76.61	4.565	91.31
ating Factor HS 20:			1		
ating Factor HS 20:	RF	HS]	** If Operating RF	is below 0.75 (H
ating Factor HS 20:	RF 0.890	HS 17.81		** If Operating RF the structure sh	is below 0.75 (H ould be posted.
ating Factor HS 20: Inventory Operating	RF 0.890 1.486	HS 17.81 29.72		** If Operating RF the structure sh	is below 0.75 (H ould be posted.
ating Factor HS 20: Inventory Operating	RF 0.890 1.486	HS 17.81 29.72		** If Operating RF the structure sh	is below 0.75 (H ould be posted.
ating Factor HS 20: Inventory Operating	RF 0.890 1.486	HS 17.81 29.72		** If Operating RF the structure sh	is below 0.75 (H ould be posted.
ating Factor HS 20: Inventory Operating erviceability Check:	RF 0.890 1.486	HS 17.81 29.72		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check:	RF 0.890 1.486	HS 17.81 29.72		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp	RF 0.890 1.486	HS 17.81 29.72 C or NC kei		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl	RF 0.890 1.486 C 50.00	HS 17.81 29.72 C or NC ksi k.ft		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Medl	RF 0.890 1.486 C 50.00 2748.17	HS 17.81 29.72 C or NC ksi k-ft		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Scil	RF 0.890 1.486 C 50.00 2748.17 319.11 1050.4	HS 17.81 29.72 C or NC ksi k-ft k-ft		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Sdl Sdl Sdl	RF 0.890 1.486 C 50.00 2748.17 319.11 1950.4 2222.0	HS 17.81 29.72 C or NC ksi k-ft k-ft in3 in2		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Sdl Sdl Sdl Su	RF 0.890 1.486 C 50.00 2748.17 319.11 1950.4 2236.0 2446.0	HS 17.81 29.72 C or NC ksi k-ft k-ft in3 in3		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Sdl Sdl Ssdl Sll	RF 0.890 1.486 C 50.00 2748.17 319.11 1950.4 2236.0 2416.9	HS 17.81 29.72 C or NC ksi k-ft k-ft in3 in3 in3		** If Operating RF the structure sh	is below 0.75 (H
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Sdl Sdl Ssdl Ssdl Sll	RF 0.890 1.486 C 50.00 2748.17 319.11 1950.4 2236.0 2416.9	HS 17.81 29.72 C or NC ksi k-ft in3 in3 in3		** If Operating RF the structure sh	is below 0.75 (H ould be posted.
ating Factor HS 20: Inventory Operating erviceability Check: Comp or Non-Comp Fy Mdl Msdl Sdl Sdl Ssdl Sll	RF 0.890 1.486 C 50.00 2748.17 319.11 1950.4 2236.0 2416.9 C 0566.03	HS 17.81 29.72 C or NC ksi k-ft k-ft in3 in3 in3	L 2619.92	** If Operating RF the structure sh	Tons

Illinois Posting Vehicles:

045-3166 POSITIVE MOMENT - SPAN 1

TYPE 2 Loading	1054.27	k-ft	Per Lane or Axle		
TYPE 2 Loading (shear)	30.22	k	Per Lane or Axle		
			· · ·		-
*Type 2 - 15.75 Tons	C	D		RF	Tons
M Inventory	9048.848	3067.28	1059.77	2.201	34.66
	9048.848	3067.28	1059.77	3.674	57.86
V Inventory	579.280	95.83	30.38	6.897	108.63
v Operating	579.280	95.83	30.38	11.513	181.33
TYPE 3 Loading	1482.61	k-ft	Per Lane or Axle		
TYPE 3 Loading (shear)	42.11	k	Per Lane or Axle		
	-		-		
*Type 3 - 22 Tons	C	D	L		1 ons
M Operating	9048.848	3067.28	1490.34	1.505	57.43
M Operating	9048.848	3067.28	1490.34	2.612	57.47
V Inventory	579.280	95.83	42.33	4.951	108.91
V Operating	579.280	95.83	42.33	8.264	181.80
TYPE 3-S1 Loading	1892.68	k-ft	Per Lane or Axle		
TYPE 3-S1 Loading (shear)	53.05	k	Per Lane or Axle		
*Type 3-S1 - 29.25 Tons	С	D	L	RF	Tons
M Inventory	9048.848	3067.28	1902.56	1.226	35.86
M Operating	9048.848	3067.28	1902.56	2.046	59.86
V Inventory	579.280	95.83	53.33	3.929	114.93
V Operating	579.280	95.83	53.33	6.559	191.84
TVDE 2 82 Loading	2202.44	1, 4			
TYPE 3-32 Loading	2292.44	K-IL	Per Lane or Axie		
TYPE 3-52 Loading (shear)	10.00	К	Per Lane or Axle		
*Type 3-S2 - 36 Tons	С	D	LI	RF	Tons
M Inventory	9048.848	3067.28	2304.40	1.012	36.44
M Operating	9048.848	3067.28	2304.40	1.690	60.82
V Inventory	579.280	95.83	65.95	3.177	114.38
V Operating	579.280	95.83	65.95	5.304	190.93
80k IDOT Legal Loading	2252.49	k-ft	Per Lane or Axle		
80k IDOT Legal Loading (she	66.21	k	Per Lane or Axle		
*80k IDOT Legal - 40 Tons	C	D		RF	Tons
M Inventory	9048.848	3067.28	2264.24	1.030	41.20
M Operating	9048.848	3067.28	2264.24	1.720	68.78
V Inventory	579,280	95.83	66.56	3.148	125.93
V Operating	579.280	95.83	66.56	5.255	210.21
M Operating V Inventory V Operating	9048.848 9048.848 579.280 579.280	3067.28 3067.28 95.83 95.83	2264.24 2264.24 66.56 66.56	1.720 3.148 5.255	68.78 125.93 210.21
* Data obtained from CONSYS			Pog		
stings: (Operating Level)	RF	Tone	R Pri		
stings: (Operating Level)	RF	Tons	NO		
stings: (Operating Level) Single Unit	RF 2.612	Tons 57.4	NO		
stings: (Operating Level) Single Unit 3 or 4 Axles	RF 2.612 2.046	Tons 57.4 59.8	NO NO		

Kane County Posting Vehicles:

KC-3

KC-4

045-3166

POSITIVE MOMENT - SPAN 1

KC-2	0.862	71.2	142,300			
KC-1	0.855	72.6	145,200			
	RF	Tons	Pounds			
stings: (Operating Level)						
	somputer program					
* Data obtained from ConSVS		90.00	100.20	5.400	200.00	J
V Operating	579.20	95.05	100.20	2.030	200.55	
	570.22	05.83	100.28	2 090	120 15	1
M Operating	9040.00	3067.20	3444.94	1 1 2 0	50.95 64.08	1
*KC-4	C 0048.95	D 2067.29	L 3444.04	RF	1 ons	-
						-
V Operating	579.28	95.83	122.10	2.865	200.52	
V Inventory	579.28	95.83	122.10	1.716	120.13	
M Operating	9048.85	3067.28	4117.08	0.946	66.20	
M Inventory	9048.85	3067.28	4117.08	0.567	39.66	
*KC-3	С	D	L	RF	Tons]
					•	1
V Operating	579.28	95.83	134.75	2.596	214.14	1
V Inventory	579.28	95.83	134.75	1.555	128.29	1
M Operating	9048.85	3067.28	4514.36	0.862	71.15	1
M Inventory	9048.85	3067.28	4514.36	0.517	42.63	1
*KC-2	С	D	L	RF	Tons	1
	519.20	90.00	130.90	2.372	210.03	J
V Operating	570.20	90.00	135.90	2 572	218 62	1
	570.22	05.83	135.02	1.600	12.00	1
M Operating	9040.00	3067.20	4555 27	0.512	72.65	1
	00/18 85	3067.28	L 1555-27	0.512	13.52	1
*1/0 1			1	DE	Topo	1
KC-4 Shear	99.76	k				
KC-4 Moment	3427.06	k-ft				
KC-3 Shear	121.47	k				
KC-3 Moment	4095.71	k-ft				
KC-2 Shear	134.05	k		KC-4	57.50	-
KC-2 Moment	4490.93	k-ft		KC-3	70.00	-
KC-1 Shear	135.28	k		KC-2	82.50	-
KC-1 Moment	4531.64	K-IL		NG-1	05.00	

0.946

1.130

66.2

65.0

132,300

129,900